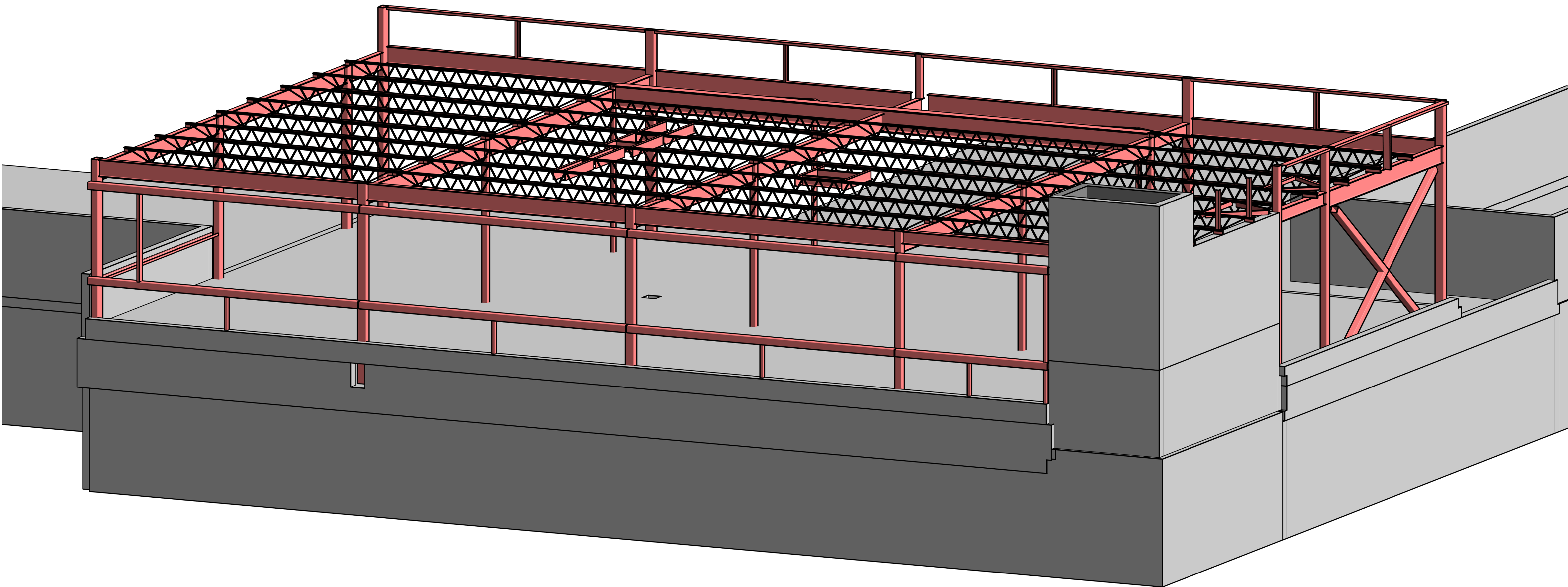


VPCH OFFICE EXPANSION



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PROJECT #: 50596-200
ISSUE: RE-ISSUED FOR TENDER
DATE: 06 SEPTEMBER 2023

STRUCTURAL DRAWING LIST	
Sheet Number	Sheet Name
S0.01	COVER SHEET
S0.02	GENERAL NOTES
S0.03	GENERAL NOTES (CONTD)
S1.01	EXISTING PARTIAL LEVEL 1 FRAMING PLAN
S1.02	EXISTING PARTIAL PARKING ROOF PLAN
S1.03	PROPOSED ELEVATED 2ND FLOOR FRAMING PLAN
S1.04	PROPOSED ROOF FRAMING PLAN
S2.01	STEEL ELEVATIONS
S3.01	BASEPLATE DETAILS
S4.01	2ND FLOOR SECTIONS
S4.02	2ND FLOOR SECTIONS CONTD
S4.03	ROOF SECTIONS
S5.01	COLUMN SCHEDULE

NOTE TO CONTRACTOR:

DO NOT SCALE DRAWINGS. CONTRACTORS MUST CHECK AND VERIFY ALL DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

ALL DRAWINGS REMAIN THE PROPERTY OF THE ENGINEER AND SHALL NOT BE REPRODUCED OR REUSED WITHOUT THE ENGINEER'S WRITTEN PERMISSION.

THE OWNER/ARCHITECT/CONTRACTOR IS ADVISED THAT M.T.E. CONSULTANTS INC. CANNOT CERTIFY ANY COMPONENT OF THE SITE WORKS NOT INSPECTED DURING CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO NOTIFY M.T.E. CONSULTANTS INC. PRIOR TO COMMENCEMENT OF CONSTRUCTION TO ARRANGE FOR INSPECTION.

Re-Issued For Tender	3	06 Sept 2023
Issued for Tender	2	23 Jun 2023
Issued for Building Permit	1	21 Apr 2023
ISSUANCE		ID DATE



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CLIENT
VICTORIA PARK

PROJECT
VPCH OFFICE EXPANSION

155 QUEEN STREET NORTH HAMILTON, ON
DRAWING

COVER SHEET

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	S0.01
Scale:			

N01	GENERAL NOTES
<p>1. CONFORM TO THE REQUIREMENTS OF THE LATEST ONTARIO BUILDING CODE (OBC), INCLUDING ALL THE LATEST STANDARDS REFERENCED THEREIN, AND ANY APPLICABLE ACTS OF AUTHORITY HAVING JURISDICTION, THE LATEST VERSION OF ALL STANDARDS AND CODES LISTED BELOW SHALL BE USED.</p> <p>2. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH ALL OTHER SPECIFICATIONS AND CONTRACT DOCUMENTS.</p> <p>3. WHERE DISCREPANCIES EXIST BETWEEN CONTRACT DOCUMENTS, INCLUDING DRAWINGS AND APPLICABLE CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN. CONTRACTOR SHALL CHECK ALL DIMENSIONS ON WORKING DRAWINGS AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.</p> <p>4. THESE DESIGN DOCUMENTS ARE PREPARED SOLELY FOR THE USE BY THE PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS ENTERED INTO A CONTRACT AND THERE ARE NO REPRESENTATIONS OF ANY KIND MADE BY THE DESIGN PROFESSIONAL TO ANY PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS NOT ENTERED INTO A CONTRACT.</p> <p>5. THE USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISION COLUMN, DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED 'ISSUED FOR CONSTRUCTION' BY MTE CONSULTANTS.</p> <p>6. UNDER NO CIRCUMSTANCES ARE THESE DRAWINGS TO BE SCALED, INCLUDING FOR PREPARATION OF SHOP DRAWINGS, CONSTRUCTION LAYOUT, OR BIDDING PURPOSES. ERRORS MADE BY PERSONS SCALING THESE DRAWINGS SHALL NOT BE THE RESPONSIBILITY OF MTE CONSULTANTS.</p> <p>7. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND SIZES OF PITS, BASES, HOUSE KEEPING PADS, SUMPS, TRENCHES, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS.</p> <p>8. BEFORE PROCEEDING WITH WORK, THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIARIZED WITH ALL CHARACTERISTICS AFFECTING NEW AND EXISTING CONSTRUCTION, ANY CHANGES, ALTERATIONS OR REVISIONS MUST BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.</p> <p>9. SUBSTITUTIONS FROM SPECIFIED PRODUCTS AND MATERIALS MUST BE APPROVED IN WRITING BY THE ENGINEER PRIOR TO ORDERING OF MATERIALS. THE CONTRACTOR SHALL REIMBURSE ALL CONSULTANTS FOR ADDITIONAL COSTS INCURRED AS A RESULT OF REVIEWING ANY CHANGES MADE TO THE CONTRACT DOCUMENTS.</p> <p>10. ALL WORK IS TO BE PERFORMED IN ACCORDANCE WITH THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS - O. REG. 213/91.</p> <p>11. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN ALL SHORING AND TEMPORARY BRACING AS PER O. REG 213/91 AND THE CONTRACTOR SHALL RETAIN AN ENGINEER AS REQUIRED.</p> <p>12. THE CONTRACTOR SHALL RETAIN AN INDEPENDENT INSPECTION AND TESTING COMPANY TO ENSURE THAT ALL WORK IS DONE IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS. REQUIRED TESTING SHALL BE AS PER THE TESTING AND INSPECTION TABLE BELOW.</p> <p>13. MTE CONSULTANTS WILL PROVIDE GENERAL REVIEW OF CONSTRUCTION IN ACCORDANCE WITH THE PERFORMANCE STANDARDS OF THE ASSOCIATION OF PROFESSIONAL ENGINEERS OF ONTARIO BY MEANS OF A RATIONAL SAMPLING PROCEDURE TO DETERMINE WHETHER THE CONSTRUCTION OF THAT WORK SHOWN ON THE MTE DRAWINGS IS IN GENERAL CONFORMITY WITH THE PLANS, SKETCHES, DRAWINGS, AND SPECIFICATIONS FORMING PART OF THE CONTRACT DOCUMENTS PREPARED BY "MTE". THE CONTRACTOR IS SOLELY RESPONSIBLE FOR QUALITY CONTROL AND THE PERFORMANCE OF THE WORK IN ACCORDANCE WITH THE CONTRACT. "MTE" SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUB-CONTRACTOR, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.</p> <p>14. IT IS THE RESPONSIBILITY OF BOTH THE OWNER AND THE CONTRACTOR TO NOTIFY THE ENGINEER OF CONSTRUCTION PROGRESS SO THE ENGINEER CAN COMPLETE GENERAL REVIEWS. THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH A CONSTRUCTION SCHEDULE PRIOR TO STARTING THE WORK. GENERALLY, REVIEWS BY THE ENGINEER WILL BE REQUIRED FOR REBAR PRIOR TO CONCRETE PLACEMENT, FOOTING AND FOUNDATIONS PRIOR TO BACKFILLING, AND ABOVE GRADE FRAMING PRIOR TO INSTALLATION OF INTERIOR FINISHES.</p>	

N02	TESTING AND INSPECTION	
THE FOLLOWING ITEMS REQUIRE TESTING OR INSPECTION BY A CERTIFIED INDEPENDENT TESTING OR INSPECTION AGENCY UNLESS NOTED OTHERWISE. THE AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE ENGINEER FOR REVIEW.		
ITEM	REQ'D	COMMENTS
SOIL BEARING CAPACITY	N/A	
SOIL COMPACTION	N/A	
REINFORCING STEEL PLACEMENT	YES	INSPECT FINAL PLACEMENT
CONC. COMPRESSIVE TESTS	YES	MIN. 2 SETS PER 100 CUBIC METRES
CONCRETE SLUMP	YES	
STRUCTURAL STEEL BOLTING	YES	
STRUCTURAL STEEL WELDING	YES	INSPECT ALL FIELD WELDS
MORTAR CUBES	N/A	

N03		REQUIRED SUBMITTALS	
THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.			
ITEM	REQ'D SUBMITTAL?	ENGINEER'S STAMP REQ'D?	NOTES
REBAR SHOP DRAWINGS	YES	NO	INCL CONC BLOCK REINF
CONCRETE MIX DESIGNS	YES	NO	
MASONRY GROUT MIX DESIGN	YES	NO	
BLOCK MILL REPORT	YES	NO	
STRUCTURAL STEEL SHOP DRAWINGS	YES	YES	FOR CONNECTIONS ONLY
MISCELLANEOUS STEEL SHOP DRAWINGS	N/A	N/A	
STEEL DECK SHOP DRAWINGS	YES	YES	
COLD FORMED STEEL FRAMING SHOP DWGS.	YES	YES	
FALL ARREST ANCHORS	N/A	N/A	

N04		PROJECT DESIGN DATA TABLE	
BUILDING IMPORTANCE CATEGORY		NORMAL	
FLOOR AND ROOF DESIGN LOADS AS NOTED ON FRAMING PLANS			
SPECIFIED WIND LOADS			
HOURLY WIND PRESSURE (1/50) DESIGN DATA		0.46 kPa	
WIND DESIGN CATAGORY		CATEGORY 2	
TERRAIN		'OPEN'	
SPECIFIED SNOW LOADS			
BASIC ROOF SNOW LOAD		S	1.28 kPa
		Ss	1.10 kPa
SNOW AND RAIN LOADING (1/50) DESIGN DATA		Sr	0.40 kPa
		24HR RAIN	108 mm
FACTORS USED FOR BASIC ROOF SNOW LOAD		Cb	0.8
		Cw	1.0
		Cs	1.0
		Ca	1.0
		ADDITIONAL SNOW ACCUMULATION AROUND OBSTRUCTIONS AND ADJACENT TO HIGHER ROOF LEVELS OR WALLS IS INDICATED ON THE DRAWINGS.	
SPECIFIED EARTHQUAKE LOADS			
SEISMIC LOADING DESIGN DATA		Sa (0.2)	0.260
		Sa (0.5)	0.128
		Sa (1.0)	0.061
		Sa (2.0)	0.028
		Sa (5.0)	0.0068
		Sa (10.0)	0.0027
		PGA	0.168
		PGV	0.101
SITE CLASS TO BE CONFIRMED BY GEOTECHNICAL ENGINEER		SITE CLASS (ASSUMED)	'D'
SEISMIC FORCE MODIFICATION FACTORS FOR SEISMIC FORCE RESISTING SYSTEM		Rd	1.5
		Ro	1.3
SEISMIC HAZARD INDEX		IeFaSa (0.2)	0.310
NO STRUCTURAL IRREGULARITIES			
Mv = 1.0			
Ie = 1.0			
NOTES:			
1. ALL LOADS AND ANALYSIS CONFROM TO THE 2012 OBC DIV 9 PART 4 (INCLUDING AMENDMENTS MADE ON JANUARY 1, 2022) AND THE USER'S GUIDE - NBC 2015 STRUCTURAL COMMENTARIES.			
2. ALL DESIGN DATA ABOVE IS FROM THE 2012 OBC SUPPLEMENTARY STANDARD SB-1 TABLE 1.2.			
3. WIND LOADING IS BASED ON THE STATIC PROCEDURE.			
4. SEISMIC LOADING IS BASED ON THE EQUIVALENT STATIC FORCE PROCEDURE.			
5. THE STRUCTURE HAS NOT BEEN DESIGNED FOR ANY FUTURE EXTENSION UNLESS NOTED.			

N07	STEEL DECK
1. DESIGN METAL DECK IN CONFORMANCE WITH THE REQUIREMENTS OF CSA S136 FOR THE LOADS INDICATED ON THE DRAWINGS.	
2. SUBMIT SHOP DRAWINGS INDICATING WELDS, MATERIALS AND FINISHES, AND BEARING THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO FOR REVIEW BY THE ENGINEER. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO PROCEEDING WITH ANY FABRICATION.	
3. UNLESS NOTED OTHERWISE,	
a. ROOF DECK SHALL BE 38 mm x 0.76 mm (1.5" x .030") VIC WEST STEEL INC. RD 938 (OR APPROVED EQUAL), MINIMUM 3 SPANS CONTINUOUS.	
b. FLOOR DECK SHALL BE 38 mm x 0.76 mm (1.5" x .030") VIC WEST STEEL INC. HB 938 (OR APPROVED EQUAL), MINIMUM 3 SPANS CONTINUOUS.	
4. METAL DECK SHALL BE LIGHT ZINC COATED STRUCTURAL STEEL SHEET FABRICATED AND ERECTED IN ACCORDANCE WITH CSSBI 10M, CAN/CSA S136, AND CSSBI 101M. THE MINIMUM ZINC COATING DESIGNATION SHALL BE Z6705 (U.N.O.).	
5. DECK SHALL OVERLAP A MINIMUM OF 50 mm (2") AT ALL END JOINTS AND HAVE A MINIMUM BEARING LENGTH OF 50 mm (2") ON ALL STRUCTURAL STEEL.	
6. DECK HAS BEEN DESIGNED FOR DIAPHRAGM ACTION AND SHALL BE FASTENED AS FOLLOWS: WELD DECK TO SUPPORTING STEEL WITH 20 mm (3/4") DIAMETER PLUG WELD AT TRANSVERSE WELD SPACING =300 mm (12") O.C. PERIMETER WELD SPACING =300 mm (12") O.C. SIDE LAP BUTTJON PUNCHING =300 mm (12") O.C. LONGITUDINAL WELD SPACING =300 mm (12") O.C	
7. DECK WELDS SHALL BE TOUCHED UP WITH APPROVED PAINT BY THE DECK ERECTOR.	
8. PROTECT ROOF AND FLOOR DECK FROM DAMAGE DURING SHIPPING STORAGE AND ERECTION. CONTRACTOR SHALL REPLACE ANY PUNCTURED, DENTED OR WELD PERFORATED DECK.	
9. STEEL DECK WORK SHALL INCLUDE THE SUPPLY AND INSTALLATION OF ALL SHEET STEEL ANGLES, COVER PLATES, CLOSURES, STIFFENERS AND ANY OTHER ACCESSORIES REQUIRED.	
10. CUT OPENINGS AND REINFORCE EDGES AS REQUIRED FOR PIPES, DUCTS, ETC. a. THE MAXIMUM SIZE OF AN UNREINFORCED OPENING IS 150 mm (6"). b. REINFORCE ALL OPENINGS LARGER THAN 150 mm (6"), BUT NOT EXCEEDING 450 mm (18") AS INDICATED BY THE METAL DECK SUPPLIER. c. FOR OPENINGS GREATER THAN 450mm (18") NOT SHOWN ON THE DRAWINGS, CONTACT ENGINEER FOR DIRECTION.	
11. HANGER WIRE FOR SUSPENDED CEILINGS SHOULD PIERCE BOTH SIDES OF THE FLUTE AND BE LOOPED AROUND AND TIED.	

N08	STRUCTURAL STEEL
1. ALL STRUCTURAL STEEL AND CONNECTIONS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST CSA STANDARD S16	
2. STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA-G40.20 FOR GENERAL REQUIREMENTS, AND CAN/CSA-G40.21 FOR QUALITY a. GRADE 350W CLASS C FOR H.S.S. b. GRADE 350W FOR W SHAPES, S SHAPES, AND TEES. c. ALL OTHER MISCELLANEOUS METAL SHALL BE MINIMUM GRADE 300W (U.N.O.)	
3. BOLTED CONNECTIONS SHALL USE ASTM A325 BOLTS. ALL BOLTS, NUTS AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325 EXCEPT THAT ANCHOR BOLTS SHALL BE FABRICATED FROM STEEL ROD CONFORMING TO CSA STANDARD G40.21 OR ASTM F1554 WITH A MINIMUM YIELD STRENGTH OF 250 MPa.	
4. STEEL COATINGS - UNLESS NOTED OTHERWISE ALL STRUCTURAL STEEL SHALL BE CLEANED AND PREPARED TO A MINIMUM LEVEL OF SSPC SP-3 AND IN ACCORDANCE WITH CSA STANDARD S16: a. ALL INTERIOR STEEL THAT IS TO BE PROTECTED BY A SPRAY APPLIED CEMENTIOUS FIRE PROOFING SHALL BE CLEANED AND REMAIN UNCOATED STEEL. b. ALL OTHER INTERIOR STRUCTURAL STEEL SHALL BE SHOP PRIME PAINTED AS PER CSA CAN-S-16. SHOP PRIMER SHALL CONFORM TO CISC/CPMA 1-73A. c. ALL STEEL EXPOSED TO WEATHER IS TO BE HOT DIP GALVANIZED IN ACCORDANCE TO CAN/CSA-G164. TOUCH UP OF WELDS, CUTS OR SCRATCHES TO GALVANIZING SHALL BE DONE WITH A MINIMUM OF 3 COATS OF ZINC RICH PAINT.	
5. WELDING OF STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD W59 AND SHALL BE UNDERTAKEN BY A FABRICATOR AND ERECTOR FULLY APPROVED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF CSA STANDARD W47, DIVISION 1 AND DIVISION 2. FABRICATOR TO SUPPLY CERTIFICATION OF FUSION WELDING, AND WELDING MAY ONLY BE CARRIED OUT IN ACCORDANCE WITH OWNERS SAFETY REGULATIONS REGARDING WELDING.	
6. FABRICATOR SHALL DESIGN CONNECTIONS AND THE LIKE IN ACCORDANCE WITH THE 2012 OBC FOR THE FORCES SHOWN ON THE DRAWINGS. WHERE FORCES ARE NOT NOTED ON THE DRAWINGS, BEAM REACTIONS SHALL BE TAKEN AS ONE-HALF OF THE TOTAL UNIFORMLY DISTRIBUTED FACTORED LOADS NOTED ON THE BEAM LOAD TABLES OF PART FIVE OF CISC'S HANDBOOK OF STEEL CONSTRUCTION, LATEST EDITION. PROVIDED NO POINT LOADS ACT ON THE BEAM. ALL WELDS SHALL BE 5 mm (3/16") MIN. FILLET. ALL BOLTS SHALL BE MIN. M20 (3/4") DIAMETER AND PROVIDE MIN. (2) BOLTS PER CONNECTION.	
7. WHERE MOMENT CONNECTIONS ARE CALLED FOR BUT VALUES ARE NOT INDICATED, DESIGN CONNECTIONS FOR FULL MOMENT CAPACITY OF THE SMALLER MEMBER JOINED.	
8. SPLICES SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF THE MEMBER AT THE POINT OF THE SPLICE. MEMBERS SHALL NOT BE SPICED AT POINTS OF MAXIMUM STRESS. NO SPLICES SHALL BE MADE UNLESS SHOWN ON THE DRAWINGS OR REVIEWED AND APPROVED BY THE ENGINEER.	
9. MOMENT FRAME AND X-BRACE CONNECTIONS SHALL HAVE ASTM A325 FRICTION TYPE M20 (3/4") MINIMUM DIAMETER BOLTS (U.N.O.).	
10. SHAPE AND SIZE GUSSET PLATES TO CLEAR ARCHITECTURAL FINISHES AND MECHANICAL DUCTS AND PIPES AND ELEVATOR SHAFTS.	
11. SHOP DRAWINGS OF STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE FABRICATION.	
12. ALL BEAMS CANTILEVERED OR CONTINUOUS OVER A COLUMN OR OTHER SUPPORT, AND BEAMS SUPPORTING POINTS OF CONCENTRATED LOAD, SHALL HAVE A MIN. OF 2-10 mm (3/8") STIFFENERS EACH SIDE OF WEB UNLESS OTHERWISE NOTED.	
13. TOP OF COLUMNS WHICH ARE NOT BRACED BY JOISTS OR BEAMS SHALL BE BRACED DIAGONALLY TO THE ROOF OR FLOOR BY A MINIMUM OF 4-L76 x 76 x 6.4 mm (L3 x 3 x 1/4") ANGLES FOR INTERIOR COLUMNS. A MINIMUM 2-L76 x 76 x 6.4 mm (L3 x 3 x 1/4") ANGLES FOR EXTERIOR COLUMNS. BRACING SHALL BE BETWEEN TOP OF COLUMN AND TOP CHORD OF JOISTS.	
14. COLUMN BASE PLATES AND BEAM BEARING PLATES SHALL BE GROUTED WITH 40 mm (1.5") NON-SHRINK 40 MPa GROUT.	
15. ALL COLUMNS BUILT INTO MASONRY WALLS SHALL HAVE ADJUSTABLE ANCHORS AT MINIMUM 400 mm (16") O.C.	
16. STEEL BEAMS AND LINTELS SHALL HAVE 200 mm (8") MINIMUM END BEARING ON MASONRY AND 65 mm (2 1/2") MINIMUM BEARING ON STEEL UNLESS INDICATED OTHERWISE.	
17. FOR ALL BEAMS AND LINTELS ON STEEL BEARING PLATES. a. BEARING PLATES ARE TO BE CENTRED BELOW ALL BEAMS OR LINTELS U.N.O. ON THE DRAWINGS. b. WELD TO BEARING PLATE WITH A MINIMUM 50 mm x 5 mm (2" x 3/16") FILLET ON BOTH SIDES OF BEAM.	
18. WHERE BACK TO BACK ANGLES ARE USED AS LINTELS OR SUPPORTS, STITCH WELD TOGETHER AT A MAXIMUM SPACING OF 300mm (12") O.C.	
19. ALL ROOF OPENINGS TO BE REINFORCED BY FRAMES COMPRISED OF C130X10 (C58x7) CHANNEL MEMBERS UNLESS NOTED OTHERWISE. MAXIMUM SPAN 2250 mm (7'-6").	
20. SUPPORT AT COLUMNS AND IRREGULARITIES: a. INSTALL L76 x 76 x 6.4 mm (L3 x 3 x 1/4") ANGLE SEATS FOR STEEL DECK AT CONNECTIONS, AT COLUMNS OR OTHER IRREGULARITIES, TO PROVIDE SUPPORT TO THE RIBS OF THE DECK. b. INSTALL L102 x 102 x 7.9 mm (L4 x 4 x 5/16") ANGLE SEATS FOR PRECAST SUPPORT AT CONNECTIONS, AT COLUMNS OR OTHER IRREGULARITIES, TO PROVIDE BEARING FOR PRECAST PLANKS.	
21. NO STRUCTURAL STEEL SHALL BE CUT IN THE FIELD UNLESS REVIEWED AND APPROVED BY THE ENGINEER.	
22. MAINTAIN ERECTION BRACING UNTIL COMPLETION OF ENTIRE STRUCTURE, INCLUDING ROOF DECKS AND OTHER ELEMENTS WHICH ARE PART OF THE LATERAL LOAD RESISTING SYSTEM.	

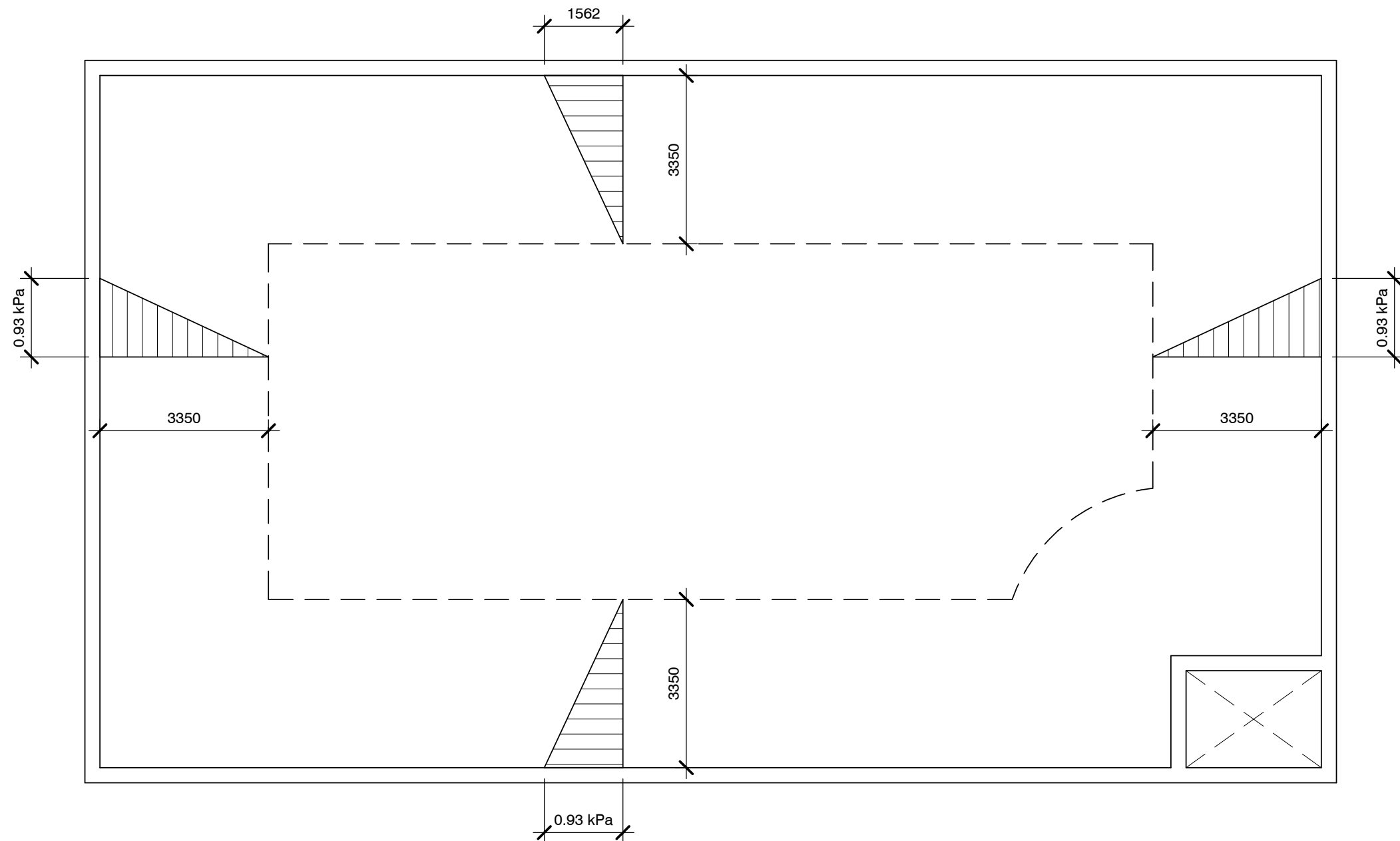
N06		CONCRETE AND REINFORCING			
1. ALL CONCRETE WORK TO CONFORM TO THE LATEST REQUIREMENTS OF CSA STANDARDS A23.1, A23.2 & A23.3.					
2. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CAN/CSA G30.18 GRADE 400W FOR REINFORCING STEEL AND BE DEFORMED HI-BOND HARD GRADE WITH MINIMUM YIELD STRENGTH OF Fy = 400 MPa.					
3. WELDED WIRE MESH AND WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS OF CAN/CSA G30.5 WITH A MINIMUM YIELD STRENGTH OF Fy = 450 MPa. ALL WELDED WIRE PRODUCTS ARE TO BE SUPPLIED AS FLAT SHEETS AND SHALL BE LAPPED A MINIMUM OF 150mm (6") AT JOINTS (U.N.O.).					
4. DETAILING AND PLACING OF ALL REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE REINFORCING STEEL INSTITUTE OF CANADA "MANUAL OF STANDARD PRACTICE".					
5. ALL REINFORCING STEEL SHALL BE SHOP FABRICATED TO INCLUDE HOOKS AND BENDS AS REQUIRED.					
6. ALL REINFORCING LAP SPLICES SHALL CONFORM TO THE LATEST CSA STANDARD A23.3 AND ALL BAR SPLICES SHALL BE CLASS 'B' TENSION SPLICES (U.N.O.). a. NO BAR SPLICES SHALL BE LESS THAN IN THE TABLE BELOW. b. INCREASE HORIZONTAL SPLICE LENGTHS IN THE TABLE BY 1.3 WHERE MORE THAN 300MM (12") OF FRESH CONCRETE IS CAST BELOW THE SPLICE.					
REBAR SIZE	CONCRETE	TENSION SPLICE			COMPRESSION SPLICE
		25 MPa	30 MPa	35 MPa	
10M		400 (16")	400 (16")	400 (16")	450 (18")
15M		600 (24")	600 (24")	600 (24")	450 (18")
20M		800 (32")	800 (32")	800 (32")	600 (24")
25M		1200 (48")	1100 (44")	1000 (40")	750 (30")
30M		1400 (56")	1300 (52")	1200 (48")	900 (36")
35M		1650 (66")	1500 (60")	1400 (56")	1050 (42")
7. ALL DOWEL EMBEDMENT SHALL MATCH THE ABOVE TENSION SPLICE LENGTH, UNLESS NOTED OTHERWISE.					
8. ALL REINFORCING STEEL FABRICATION AND PLACEMENT DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE FABRICATION.					
9. PLACE REINFORCING BARS SYMMETRICALLY OVER SUPPORTS AND SYMMETRICALLY IN SPANS, UNLESS NOTED OTHERWISE.					
10. REINFORCING BARS, DOWELS AND ANCHOR BOLTS SHALL BE SECURELY TIED IN PLACE SO AS TO MAINTAIN THEIR EXACT POSITION BEFORE AND DURING PLACEMENT OF CONCRETE. BAR SUPPORTS SHALL ONLY BE MADE OF PRECAST CONCRETE BLOCKS, PLASTIC OR WIRE.					
11. ALL OIL, GREASE, MUD AND DEBRIS SHALL BE ENTIRELY REMOVED FROM THE REINFORCING STEEL AND ANCHOR BOLTS PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE STORED ON SITE IN A MANNER TO BE KEPT CLEAN AND FREE FROM DELETERIOUS MATERIALS.					
12. WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.					
13. CONFORM TO THE CONCRETE COVER REQUIREMENTS OF CSA A23.1 AND THE FOLLOWING, UNLESS NOTED OTHERWISE: a. CONCRETE CAST AGAINST EARTH: 75 mm (3") b. PIERS AND WALL: 40 mm (1.5") c. EXPOSED TO DE-ICING CHEMICALS: 60 mm (2.5") d. INTERIOR BEAMS: 30 mm e. INTERIOR SLABS: 25 mm (1")					
14. CONCRETE PROPERTIES: a. ALL CONCRETE SHALL HAVE A 28 DAY MINIMUM COMPRESSIVE STRENGTH OF 35 MPa UNLESS OTHERWISE SPECIFIED. b. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO USE AT JOB SITE.					
15. WHEN SUPER-PLASTICIZERS ARE USED, THE SLUMP MAY BE INCREASED BEYOND THE VALUES GIVEN, BUT SHALL BE BELOW THE POINT WHERE SEGREGATION WILL OCCUR. THE COST OF SUPER-PLASTICIZERS SHALL BE INCLUDED IN THE COST OF CONCRETE.					
16. DO NOT ADD WATER TO CONCRETE UNLESS WRITTEN APPROVAL GIVEN BY THE ENGINEER. IF HIGHER SLUMP CONCRETE IS DESIRED, CONCRETE SUPPLIER SHALL DESIGN AND SUPPLY ACCORDINGLY.					
17. HOT AND COLD WEATHER CONCRETING SHALL COMPLY WITH ALL REQUIREMENTS OF CSA STANDARD A23.1. CALCIUM CHLORIDE ADDITIVES WILL NOT BE PERMITTED.					
18. ALL CONCRETE FORMWORK TOLERANCES AND SURFACE FINISHES SHALL COMPLY WITH CSA STANDARD A23.1 UNLESS NOTED OTHERWISE ON THE ARCHITECTURAL DRAWINGS.					
19. ALL CONCRETE FORMS TO BE WET THOROUGHLY BEFORE POURING CONCRETE.					
20. WATER CURING OF CONCRETE IS RECOMMENDED. CURE AND PROTECT ALL CONCRETE IN ACCORDANCE WITH CSA A23.1 SECTION 7.4.					
21. ALL CONCRETE EXCEPT SLABS ON GRADE 150mm (6") THICK OR LESS SHALL BE MECHANICALLY VIBRATED SO AS TO COMPLETELY FILL THE FORM WITHOUT CAUSING UNDUE SEGREGATION. ANY DEFECTS IN THE HARDENED CONCRETE SHALL BE SATISFACTORILY REPAIRED OR SHALL BE REPLACED.					
22. CONTROL JOINTS IN SLABS ON GRADE SHALL BE ¼ THE THICKNESS OF THE SLAB. SPACING OF CONTROL JOINTS IN CONCRETE SLABS-ON-GRADE SHALL NOT EXCEED THE GREATER OF 25 TIMES THE THICKNESS OF THE SLAB OR 3000 MM (10'-0") UNLESS NOTED ON THE DRAWINGS.					
23. WHERE STEEL BEARING PLATES ARE SHOWN ON THE DRAWINGS, THEY SHALL BE ANCHORED WITH A MINIMUM OF TWO 12 mm DIA X 450MM LONG ± 50 mm (1/2" DIA x 18" LONG ± 2") HOOKED ANCHOR RODS WELDED TO THE PLATES AND EMBEDDED INTO THE CONCRETE.					
CONCRETE MIX PROPERTIES TABLE					
CONCRETE	MIN.28 DAYS STRENGTH (MPa) U.N.O.	SLUMP mm(in)	AIR CONTENT T (%)	MAX. AGGREGATE SIZE (in)	EXPOSURE CLASS
EXTERIOR FOUNDATION WALLS, RETAINING WALLS	35	80 (±30")	4-7	3/4"	F-2
INTERIOR PIERS / WALLS/FOOTINGS	35	80 (±30")	0	3/4"	N
INT. S.O.G.	25	80 (±30")	0	3/4"	N
FREEZE THAW EXPOSURE	25	80 (±30")	4-7	3/4"	F-2
EXTERIOR SLAB (UNREINFORCED)	32	80 (±30")	5-8	3/4"	C-2
EXTERIOR SLAB (REINFORCED)	35	80 (±30")	5-8	3/4"	C-1
NON-SHRINKABLE GROUT	30	AS PER MANUF. RECOMEN	0	-	N
LEAN MIX CONCRETE	8	80 (±30")	0	-	N
EXTERIOR FOOTINGS	35	80 (±30")	4-7	3/4"	F-2
24. CHECK ALL STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, LANDSCAPE AND ALL OTHER RELEVANT DRAWINGS FOR LOCATIONS AND SIZES OF BOLTS, SLEEVES AND OPENINGS.					

N06	CONCRETE AND REINFORCING CONT.
25. SUPPLY AND SET ANCHOR BOLTS, SLEEVES, PIPE HANGERS, JOISTS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE FOR BEAMS AND COLUMNS; NO SLEEVES, DUCTS, PIPES OR OTHER OPENINGS SHALL PASS VERTICALLY OR HORIZONTALLY EXCEPT WHERE EXPLICITLY DETAILED ON STRUCTURAL DRAWINGS OR WHERE APPROVED IN ADVANCE BY ENGINEER. FOR SLABS AND WALLS, ALL SLEEVES AND OPENINGS GREATER THAN 100 mm (4") IN ANY DIMENSION OR REQUIRING THE CUTTING OF ANY REINFORCEMENT, AND NOT INDICATED ON STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE ENGINEER. FOR MULTIPLE OPENINGS OR SLEEVES, IF WITHIN 600 mm (24") OF EACH OTHER CONSULT ENGINEER FOR DIRECTION.	
26. CAST IN ANCHOR BOLTS SHALL CONFORM TO THE LATEST CSA STANDARD G40.21 OR ASTM F1554 WITH A MINIMUM YIELD STRENGTH OF 250 MPa and SHALL BE SET TRUE AS O LOCATION, ELEVATION AND PROJECTION TO THE FOLLOWING TOLERANCES: ANCHOR BOLT LOCATION = ± 3mm (1/8") ANCHOR BOLT PROJECTION = ± 6mm (1/4").	
27. CONSTRUCTION JOINTS FOR WALLS, SLABS, AND BEAMS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION. GENERALLY, JOINTS IN SLABS SHALL BE AT RIGHT ANGLES TO THE SPANS, AT MID SPAN IF POSSIBLE AND BE CLEAR OF SUPPORTS AND POINT LOADS.	
28. CONSTRUCTION JOINTS FOR WALLS, SLABS, AND BEAMS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION. GENERALLY, JOINTS IN SLABS SHALL BE AT RIGHT ANGLES TO THE SPANS, AT MID SPAN IF POSSIBLE AND BE CLEAR OF SUPPORTS AND POINT LOADS.	
29. INSERTS, FRAME-OUTS, SLEEVES, BRACKETS, CONDUITS AND FASTENING DEVICES, SHALL BE INSTALLED AS REQUIRED BY THE DRAWINGS AND SPECIFICATIONS IN A MANNER THAT SHALL NOT IMPAIR THE STRUCTURAL STRENGTH OF THE SYSTEM. BE SO INSTALLED THAT HEY SHALL NO REQUIRE THE CUTTING, BENDING, OR DISPLACEMENT OF THE REINFORCING OTHER THAN AS SHOWN ON THE TYPICAL DETAILS.	
30. ELECTRICAL CONDUITS SHALL NOT PASS THROUGH A COLUMN, SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN 1/3 SLAB THICKNESS OR WALL OR BEAM WHICH IT IS EMBEDDED, SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER UNLESS APPROVED AND HAVE A MINIMUM CONCRETE COVER OF 25 mm (1") AND UNLESS SPECIFICALLY PERMITTED OTHERWISE, SHALL NOT RUN HORIZONTALLY IN A CONCRETE WALL.	
31. TYPE 'S' CONCRETE TO BE USED FOR ALL ELEMENTS.	

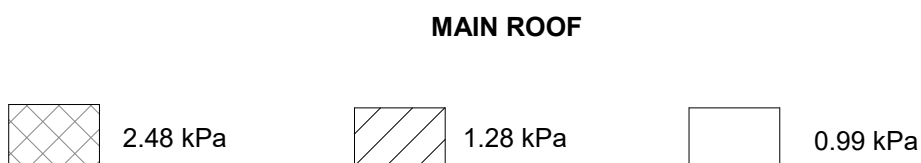
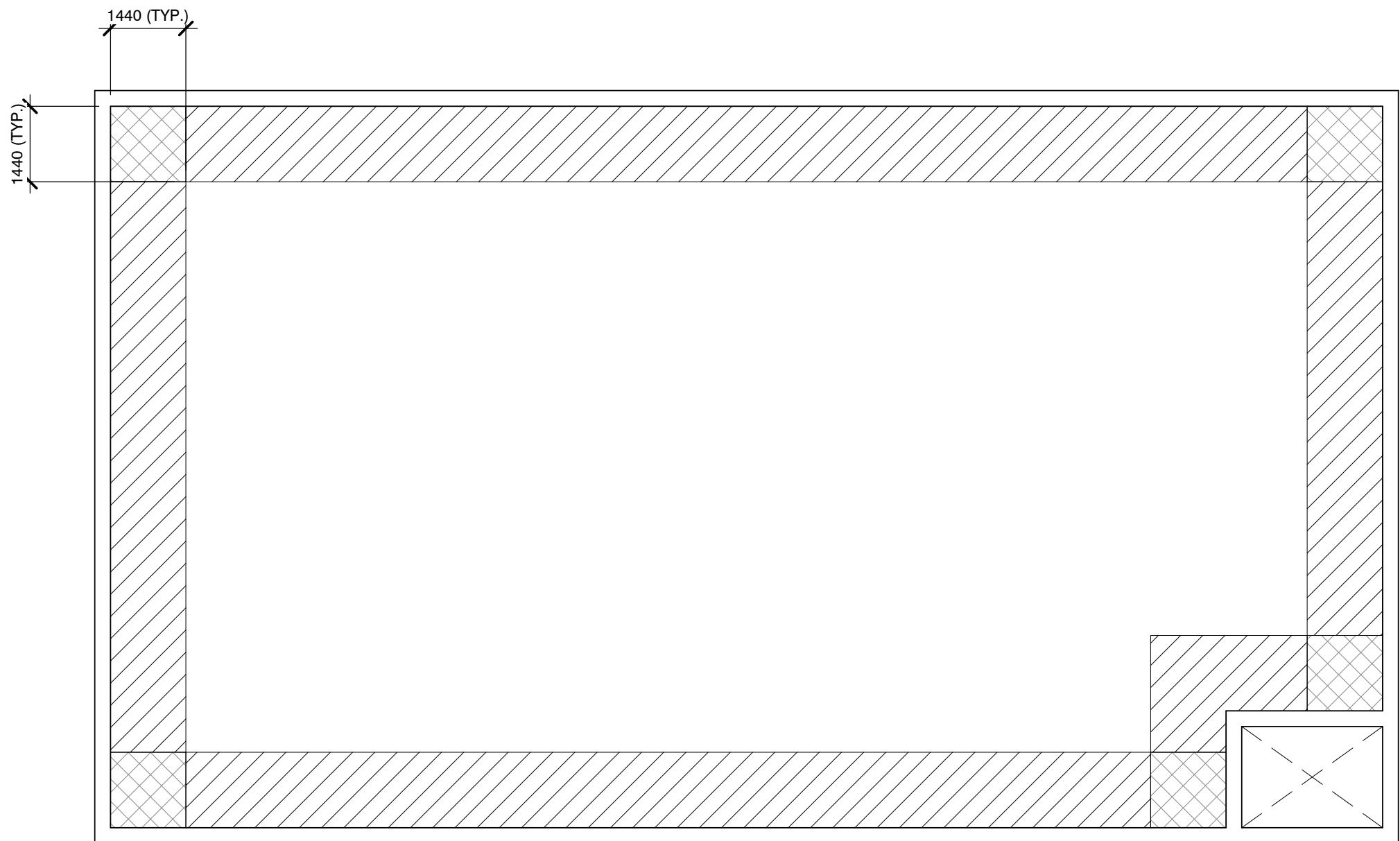
N10	SHOP DRAWING REVIEW
1. ERECTION AND FABRICATION SHOP DRAWINGS FOR ALL BUILDING COMPONENTS AS LISTED IN THE REQUIRED SUBMITTALS TABLE AND ANY RELATED WORKS ARE TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE COMMENCING WITH FABRICATION.	
2. AS PART OF THEIR FIELD SERVICES, MTE CONSULTANTS ("MTE") WILL REVIEW SHOP DRAWINGS PERTAINING TO WORK SHOWN ON MTE CONSULTANT'S DRAWINGS BY MEANS OF APPROPRIATE RATIONAL SAMPLING PROCEDURES AND COMMENT ON THE ACCURACY WITH WHICH THE CONTRACTOR PREPARED THE DRAWINGS.	
3. REVIEW OF THE SHOP DRAWINGS IS FOR THE SOLE PURPOSE OF ASCERTAINING CONFORMANCE WITH THE GENERAL DESIGN CONCEPT AND IS NOT AN APPROVAL OF THE DETAIL DESIGN INHERENT IN THE SHOP DRAWINGS. RESPONSIBILITY FOR WHICH SHALL REMAIN WITH THE CONTRACTOR SUBMITTING THEM. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITY FOR ERRORS AND OMISSIONS IN THE SHOP DRAWINGS OR FOR MEETING ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INFORMATION PERTAINING TO THE FABRICATION PROCESS TECHNIQUES OF CONSTRUCTION AND INSTALLATION AND FOR COORDINATION OF THE WORK OF ALL SUB-TRADES.	
4. THE APPROVAL OF SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR FROM THE RESPONSIBILITY OF THE FITTING OF BUILDING COMPONENTS. ANY DISCREPANCIES IN THE SHOP DRAWINGS ARE THE RESPONSIBILITY OF THE CONTRACTOR.	
5. ALL SHOP DRAWINGS MUST BEAR THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN ONTARIO UNLESS NOTED OTHERWISE IN THE SUBMITTALS TABLE BELOW. UNSEALED SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS ALTERNATIVE ARRANGEMENTS HAVE BEEN AGREED UPON.	

N15 STRUCTURAL ABBREVIATIONS			
A.B	ANCHOR BOLT	I.D.	INSIDE DIAMETER
ALT.	ALTERNATE	kN.	KILONEWTON
ALUM.	ALUMINUM	kPa	KILOPASCAL
ANCH'S	ANCHORS	∠	ANGLE
APPROX.	APPROXIMATELY	L.L.H.	LONG LEG HORIZONTAL
ARCH.	ARCHITECTURAL	L.L.V.	LONG LEG VERTICAL
B/F	BOTTOM FACE	L.P.	LOW POINT
B.PL	BASE PLATE	L.G.	LONG
BLK.	BLOCK	MAX.	MAXIMUM
BM.	BEAM	MECH	MECHANICAL
BOT.	BOTTOM	METL	METAL
BRG.	BEARING	MIN.	MINIMUM
BT.PL.	BENT PLATE	MISC.	MISCELLANEOUS
C/W	COMPLETE WITH	m	METRE
C/C	CENTRE TO CENTRE	mm	MILLIMETRE
C.J.	CONTROL JOINT	MPa	MEGAPASCAL
CLG.	CEILING	N.I.C.	NOT IN CONTRACT
COL.	COLUMN	N.T.S.	NOT TO SCALE
CONC.	CONCRETE	No.	NUMBER
CONN.	CONNECTION	O.C.	ON CENTRE
CONSTRN	CONSTRUCTION	O.D.	OUTSIDE DIAMETER
CONT.	CONTINUOUS	O.H.	OVERHEAD
DEMO.	DEMOLITION	OWSJ	OWEN WEB STEEL JOIST
DET.	DETAIL	OPNG.	OPENING
DIA.	DIAMETER	PARTN	PARTITION
DIM.	DIMENSION	PL.	PLATE
DO	DIDO	R.C.	REINFORCED CONCRETE
DP.	DEEP	R.D.	ROOF DRAIN
DWG.	DRAWING	R.O.	ROUGH OPENING
DWL.	DOWEL	REF.	REFERENCE
E.F.	EACH FACE	REINF.	REINFORCED
E.J.	EXPANSION JOINT	REQ'D	REQUIRED
ELEC.	ELECTRICAL	S.C.	SAWCUT
E.S.	EACH SIDE	SECT.	SECTION
E.W.	EACH WAY	S.L.H.	SHORT LEG HORIZONTAL
EA.	EACH	S.L.V.	SHORT LEG VERTICAL
EL.	ELEVATION	S.O.G.	SLAB ON GRADE
EQ.	EQUAL	S.S.	STAINLESS STEEL
EXIST.	EXISTING	STL.	STEEL
F.F.	FACE TO FACE	STIFF.	STIFFENER
FIN.	FINISHED	STRUCT.	STRUCTURAL
FL.	FLOOR	T/O	TOP OF
FNDR.	FOUNDATION	T.L.L.	TOP LOWER LAYER
FTG.	FOOTING	T.U.L.	TOP UPPER LAYER
Ga.	GAUGE	TYP.	TYPICAL
GALV.	GALVANIZED	U.N.O.	UNLESS NOTED
GRD.	GRADE		OTHERWISE
HORIZ.	HORIZONTAL	U/S	UNDERSIDE
H.D.	HEAVY DUTY	VERT.	VERTICAL
H.D.G.	HOT DIPPED GALVANIZED	V.E.F.	VERTICAL EACH FACE
H.E.F.	HORIZONTAL EACH FACE	V.I.F.	VERTICAL INSIDE FACE
H.P.	HIGH POINT	V.O.F.	VERTICAL OUTSIDE FACE
HSS	HOLLOW STRUCTURAL STEEL	W.P.	WORKING POINT
		W.W.M.	WELEDED WIRE MECH @ SPACED AT

N11 OPEN WEB STEEL JOIST	
1. OPEN WEB STEEL JOISTS (OWSJ'S) SHALL CONFORM TO CSA STANDARDS S16 AND CAN/CSA-S136.	
2. WELDING OF STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD W59 AND SHALL BE UNDERTAKEN BY A FABRICATOR AND ERECTOR FULLY APPROVED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF CSA STANDARD W47, DIVISION 1 AND DIVISION 2. FABRICATOR TO SUPPLY CERTIFICATION OF FUSION WELDING AND WELDING MAY ONLY BE CARRIED OUT IN ACCORDANCE WITH OWNERS SAFETY REGULATIONS REGARDING WELDING.	
3. JOISTS TO BE DESIGNED FOR THE LOADS AS SPECIFIED ON DRAWINGS AND IN ACCORDANCE WITH THE 2012 OBC. DESIGN OF JOISTS SHALL ALSO INCLUDE ALL LOADS FROM MECHANICAL EQUIPMENT SUCH AS ROOF TOP UNITS, DUCTS AND PIPING.	
4. SHOP DRAWINGS OF JOIST DETAILS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE FABRICATION. JOIST DESIGN AND DETAILS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN ONTARIO. JOIST DESIGN CALCULATIONS SHALL BE SUBMITTED FOR RECORD PURPOSES.	
5. PROVIDE SUFFICIENT CAMBER TO JOISTS TO ENSURE 10" CAMBER AFTER APPLICATION OF ALL DEAD LOADS SHOWN. ADJUST STIFFNESS AND REQUIRED CAMBER OF JOISTS ADJACENT TO MASONRY WALLS, STEEL BEAMS OF SHORTER SPAN AND THE LIKE TO PERMIT THE PROPER FASTENING OF THE STEEL DECK. AS A GUIDE, LIMIT THE DIFFERENTIAL DEFLECTION OF THE ADJACENT JOIST, UNDER ALL DEAD LOADS, TO L/120, WHERE L IS THE SPAN OF THE STEEL DECK PERPENDICULAR TO THE JOISTS.	
6. "TJ" ON PLANS DENOTES "TIE JOIST". BOTTOM CHORD TO BE FRAMED INTO COLUMNS, BEAMS OR WALLS. ALL JOISTS AT COLUMNS TO BE TIE JOISTS UNLESS OTHERWISE NOTED. TIE JOIST CONNECTIONS SHALL BE BOLTED.	
7. WHERE TIE JOISTS ARE INDICATED, DESIGN TOP AND BOTTOM CHORDS AND CONNECT TO COLUMNS TO SAFELY DEVELOP LOADS SHOWN OR A MINIMUM OF A 25 kN SPECIFIED LOAD IN TENSION OR COMPRESSION.	
8. DESIGN AND INSTALLATION OF ALL OWSJ BRIDGING SHALL BE IN ACCORDANCE WITH CSA S16. COMBINED DIAGONAL AND HORIZONTAL BRIDGING SHALL BE PROVIDED AT THE ENDS OF BRIDGING LINES AS REQUIRED. ENDS OF BRIDGING LINES SHALL BE ANCHORED TO STEEL, MASONRY OR OTHERWISE SHOWN AND BE CAPABLE OF RESISTING AN AXIAL LOAD OF AT LEAST 3 kN.	
9. BRIDGING SHOWN ON THE DRAWINGS IS INTENDED AS A GUIDELINE ONLY. DESIGN AND PROVIDE BRIDGING FOR ALL OWSJ AND TRUSSES AS PER CSA S16.	
10. OWSJ'S SHALL HAVE 100 mm (4") SHOE (U.N.O.)	
11. FOR OWSJ BEARING ON MASONRY, JOIST SUPPLIER SHALL DESIGN AND SUPPLY ALL BEARING PLATES AND BEARING PRESSURE SHALL NOT EXCEED 1.2 MPa (175 psi).	
12. ALL STEEL JOISTS SHALL BE WELDED TO STEEL BEAMS OR BEARING PLATES WITH A MINIMUM 50 mm x 5 mm (2" x 3/16") FILLET ON BOTH SIDES OF SHOES.	
13. ALL HANGERS, STUB COLUMNS, TRAPEZE BARS, ETC. THAT SUPPORT MECHANICAL, ELECTRICAL OR STRUCTURAL EQUIPMENTS, PIPES, DUCTS, CATWALKS, ETC. MUST BE CONNECTED TO AN OWSJ PANEL POINT OR WHERE THE WEB OF THE JOIST MEETS THE CHORD OF THE JOIST.	



**SPECIFIED SNOW LOAD ACCUMULATION
DIAGRAM (N.T.S.)**



GROSS WIND UPLIFT DIAGRAM (N.T.S.)

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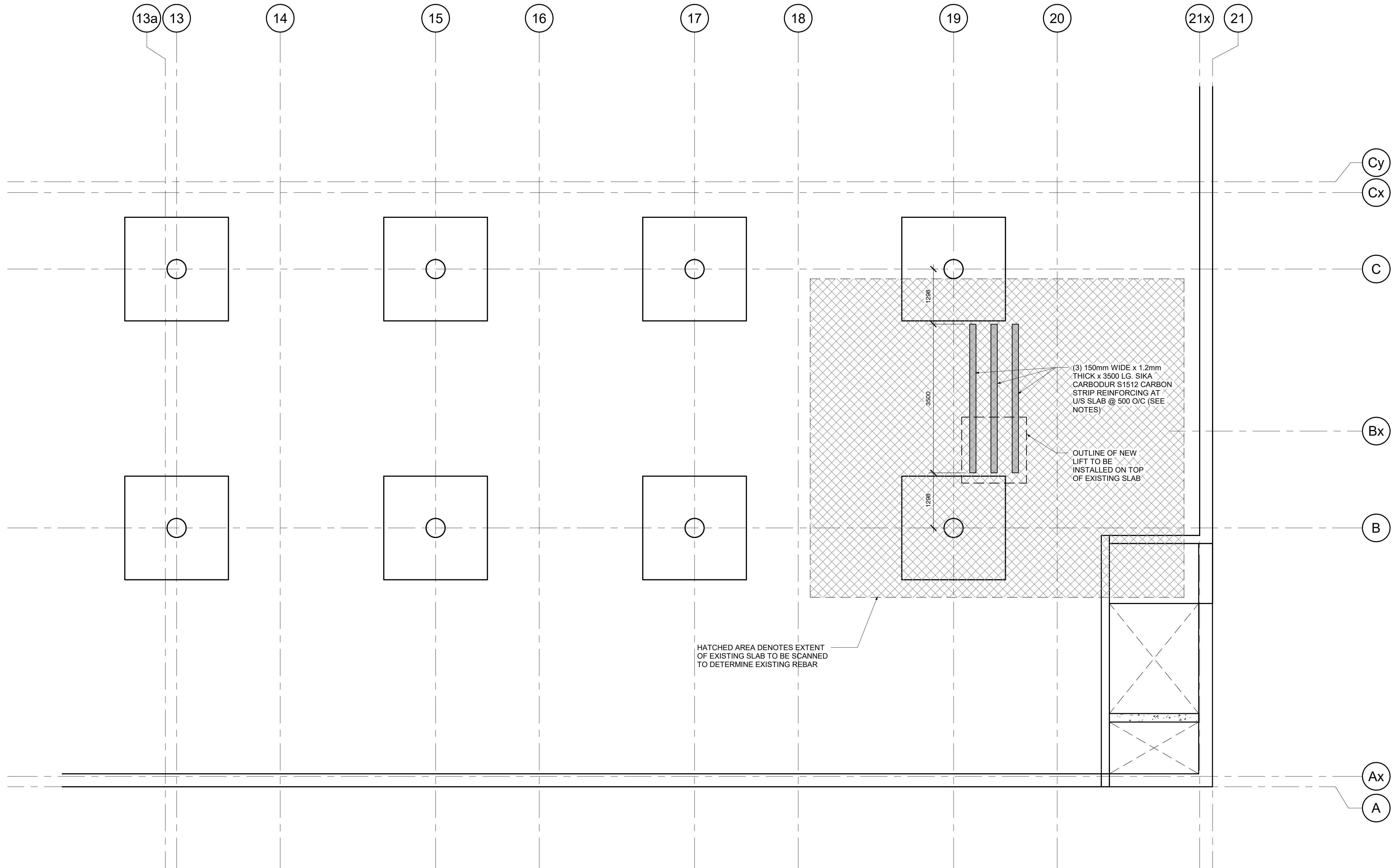
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PROJECT
VPCH OFFICE EXPANSION

155 QUEEN STREET NORTH HAMILTON, ON
DRAWING

GENERAL NOTES (CON'TD)

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	As indicated		S0.03



T/O EXTG LEVEL 01 STRUCTURE

1 : 50

NOTES:

- TOP OF EXISTING LEVEL 01 SLAB IS AT ELEVATION 0.000m (SEE ARCHITECTURAL)
- EXISTING SLAB IS 178mm THICK TWO-WAY SLAB WITH 102mm DROP PANELS 2440mm x 2440mm
- EXISTING SLAB LOADS AS FOLLOWS (r.e. EXIST STRUCTURAL DRAWINGS BY ROSCOE & STIENSTRA, JOB 7120, DATED MAY 1, 1972):
 - DEAD: SLAB O.W. - 4.40 kPa
 - LIVE LOAD: - ASSEMBLY 4.8 kPa
- SUBMIT DETAILS FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN TO STRUCTURAL CONSULTANT FOR REVIEW.
- REFER TO ALL OTHER TYPICAL NOTES & DETAILS.

CARBON STRIP REINFORCING NOTES:

GENERAL:

- FOR SLAB STRENGTHENING, USE CARBON FIBER REINFORCED PLASTIC (CFRP) SIKA CARBODUR. CARBODUR IS BONDONT ONTO THE STRUCTURE AS EXTERNAL REINFORCEMENT USING SIKADUR 30 EPOXY RESIN AS THE ADHESIVE. ANY SHEAR CRACK MUST BE PREVENTED FROM CAUSING DISPLACEMENT ON THE STRENGTHENED SURFACE AND SHEARING OF THE LAMINATE. SIKA CARBODUR S1512 IS 150mm WIDE, 1.2mm THICK. THE LEASTIC MODULUS OF SIKA CARBODUR S1512 IS 165 GPa. THE ENSILE STRENGTH OF SIKA CARBODUR S1512 IS 2.8 GPa.
- FOR CFRP FIRE PROTECTION, USE SIKACRETE-213F (40mm THICK).

SURFACE PREPARATION:

- ALL CONCRETE SURFACES SHALL BE DRY AND FREE OF SURFACE MOISTURE AND TESTED BY THE CONTRACTOR TO EVALUATE MOISTURE TRANSMISSION IN ACCORDANCE WITH ASTM D4263 "INDICATING MOISTURE IN CONCRETE BY THE PLASTIC SHEET METHOD."
- ALL CONCRETE SURFACES SHALL BE SOUND, REMOVE DETERIORATED CONCRETE, DUST, LAITANCE, GREASE, PAINT, CURING COMPOUNDS, WAXES, IMPREGNATIONS, FOREIGN PARTICLES, AND OTHER BOND INHIBITING MATERIALS FROM THE SURFACE BY BLAST CLEANING OR EQUIVALENT MECHANICAL METHODS.
- ALL CONCRETE SURFACES SHALL BE AIR BLASTED AND VACUUMED CLEAN TO A DUST-FREE CONDITION.
- CONCRETE SURFACE IRRECTULARITIES LESS THAN 25mm SHALL BE GROUND AND SMOOTHED AND / OR FILLED WITH AN APPROVED REPAIR MORTAR (E.G. SIKADUR 30) WITH THE ADDITION OF 1 PART OVEN DIRED SAND TO MAKE AN EPOXY MORTAR. SURFACE IRREGULARITIES SHALL BE LIMITED TO LESS THAN 1mm. SURFACE IRREGULARITIES GREATER THAN 25mm SHALL BE REPAIRED USING AN APPROVED CEMENTIOUS REPAIR MORTAR (e.g. SIKATOP 123)
- EXTERNAL CONCRETE CORNERS SHALL BE ROUNDED TO AT LEAST 13mm RADIUS WHEN PERPENDICULAR TO FIBER ORIENTATION AND INTERNAL CORNERS SHALL BE SMOOTHED BY TROWELLING EPOXY MORTAR INTO THE CORNERS.
- THE ADHESIVE STRENGTH OF THE CONCRETE SHALL BE VERIFIED AFTER PREPARATION BY RANDOM PULL-OFF TESTING (ACI 503R) AT THE DIRECTION OF THE ENGINEER.

MIXING PRIMER AND SATURANT

- MIX COMPONENTS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- DILUTING IS NOT PERMITTED. PRE-CONDITION MATERIALS AS INDICATED ON TECHNICAL DATA SHEET
- MIX ONLY THAT QUANTITY WHICH CAN BE USED WITHINITS POT LIFE.
- DO NOT BATCH DELIVER UNITS INTO SMALLER QUANTITIES. MIX ONLY FULL UNITS.

PRIMER APPLICATION:

- SURFACE DEPRESSIONS SHALL BE FILLED WITH EPOXY FILLER PER MANUFACTURER'S INSTRUCTIONS.

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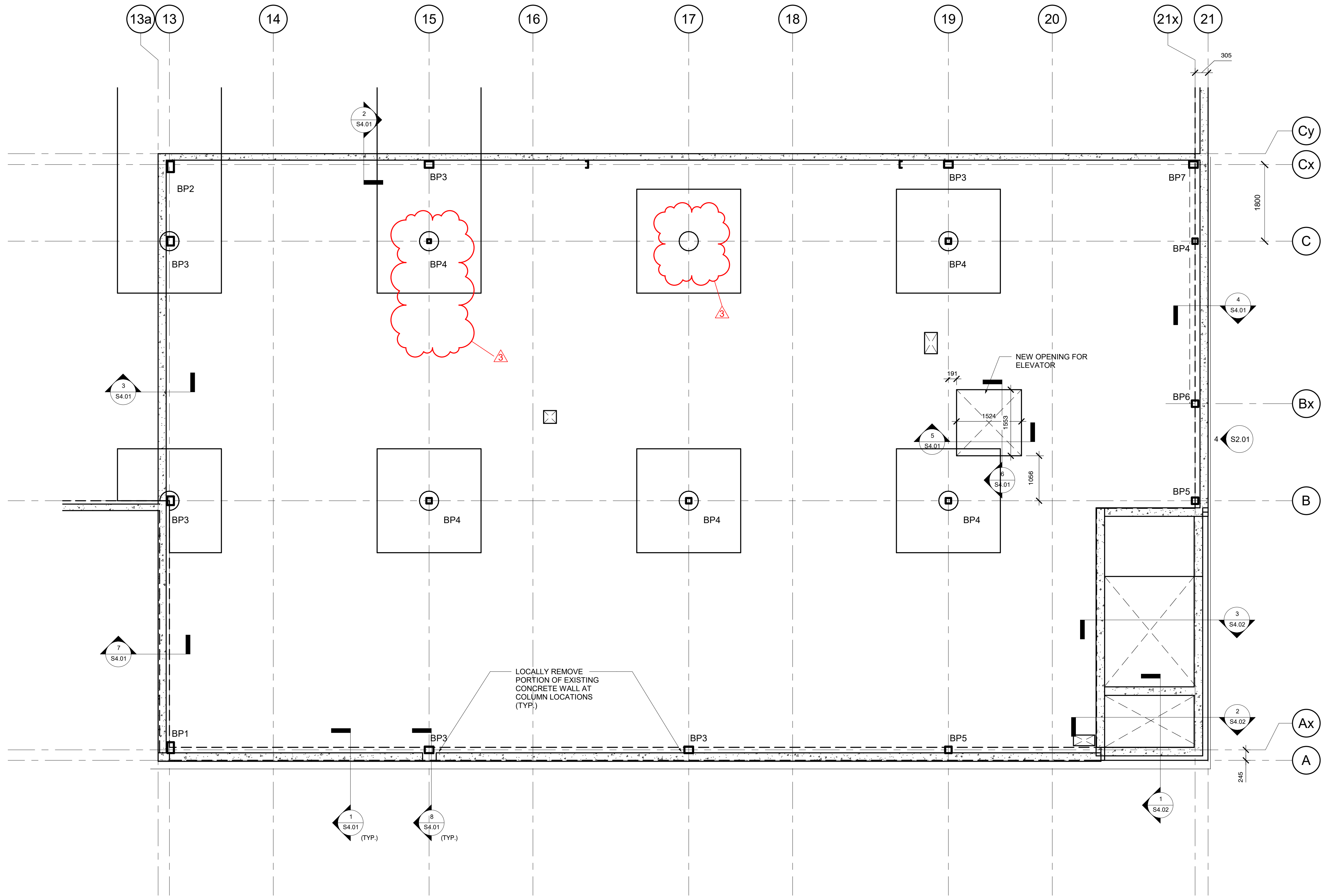
PROJECT

VPCH OFFICE EXPANSION

155 QUEEN STREET NORTH
DRAWING
HAMILTON, ON

**EXISTING PARTIAL LEVEL 1
FRAMING PLAN**

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 50		S1.01



EXIST. PARITAL PARKING ROOF FRAMING

1 : 50

NOTES:

- TOP OF EXISTING PARKING ROOF SLAB IS AT ELEVATION 2.670m (SEE ARCHITECTURAL)
- EXISTING SLAB IS 200mm THICK TWO-WAY SLAB WITH 152mm DROP PANELS 2440mm x 2440mm
- EXISTING SLAB DESIGN LOADS AS FOLLOWS (r.e. EXIST STRUCTURAL DRAWINGS BY ROSCOE & STIENSTRA, JOB 7120, DATED MAY 1, 1972)
 - DEAD: SLAB O.W. 5.40 kPa
ROOF: 0.50 kPa
 - LIVE LOAD: 15 kPa
- NEW DESIGN LOADS TO BE SLAB O.W. (DL = 5.40 kPa) PLUS THOSE SHOWN ON SHEET S1.03.
- SUBMIT DETAILS FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN TO STRUCTURAL CONSULTANT FOR REVIEW.
- REFER TO ALL OTHER TYPICAL NOTES & DETAILS.

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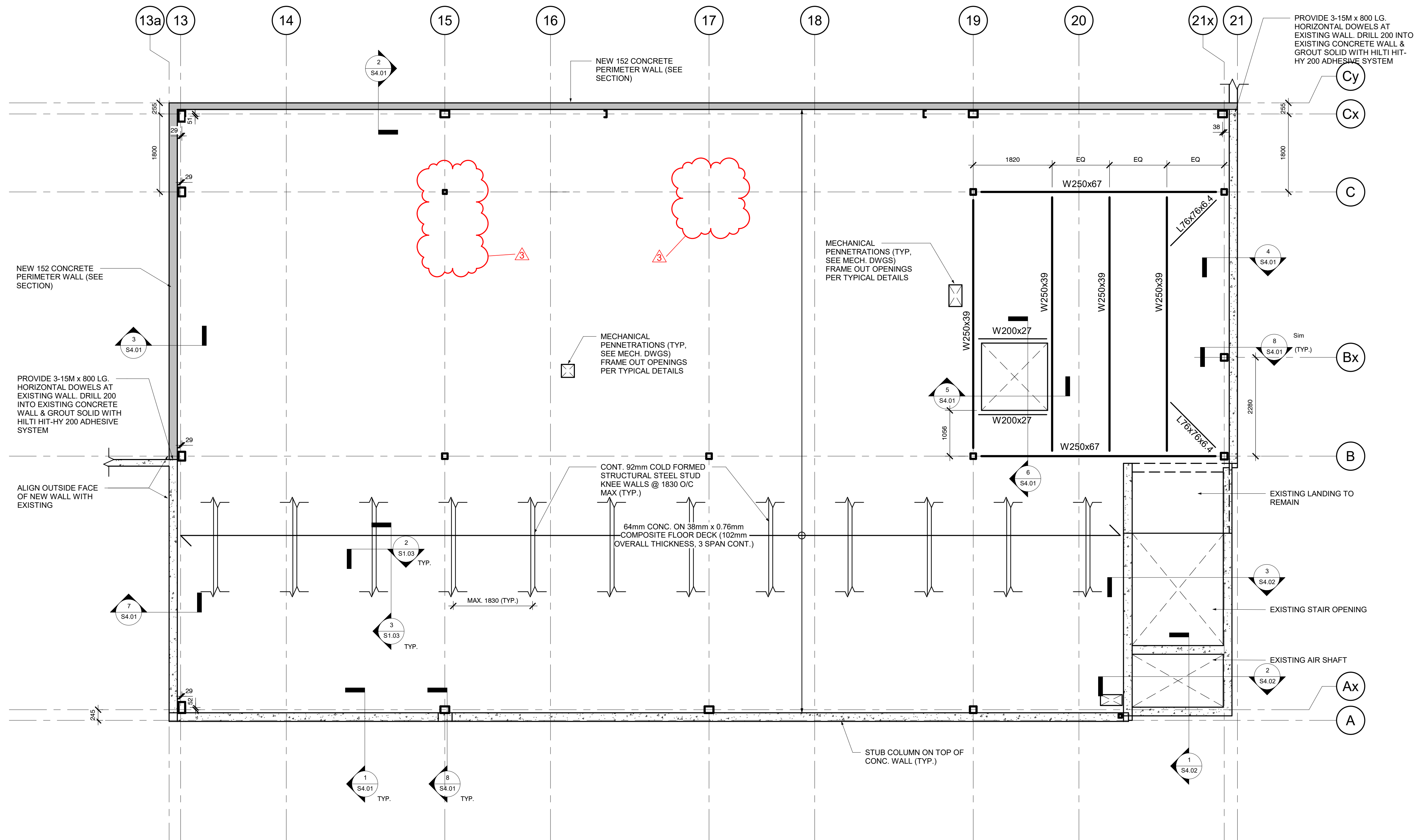
PROJECT
VPCH OFFICE EXPANSION

155 QUEEN STREET NORTH HAMILTON, ON

DRAWING

**EXISTING PARITAL
PARKING ROOF PLAN**

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 50		S1.02

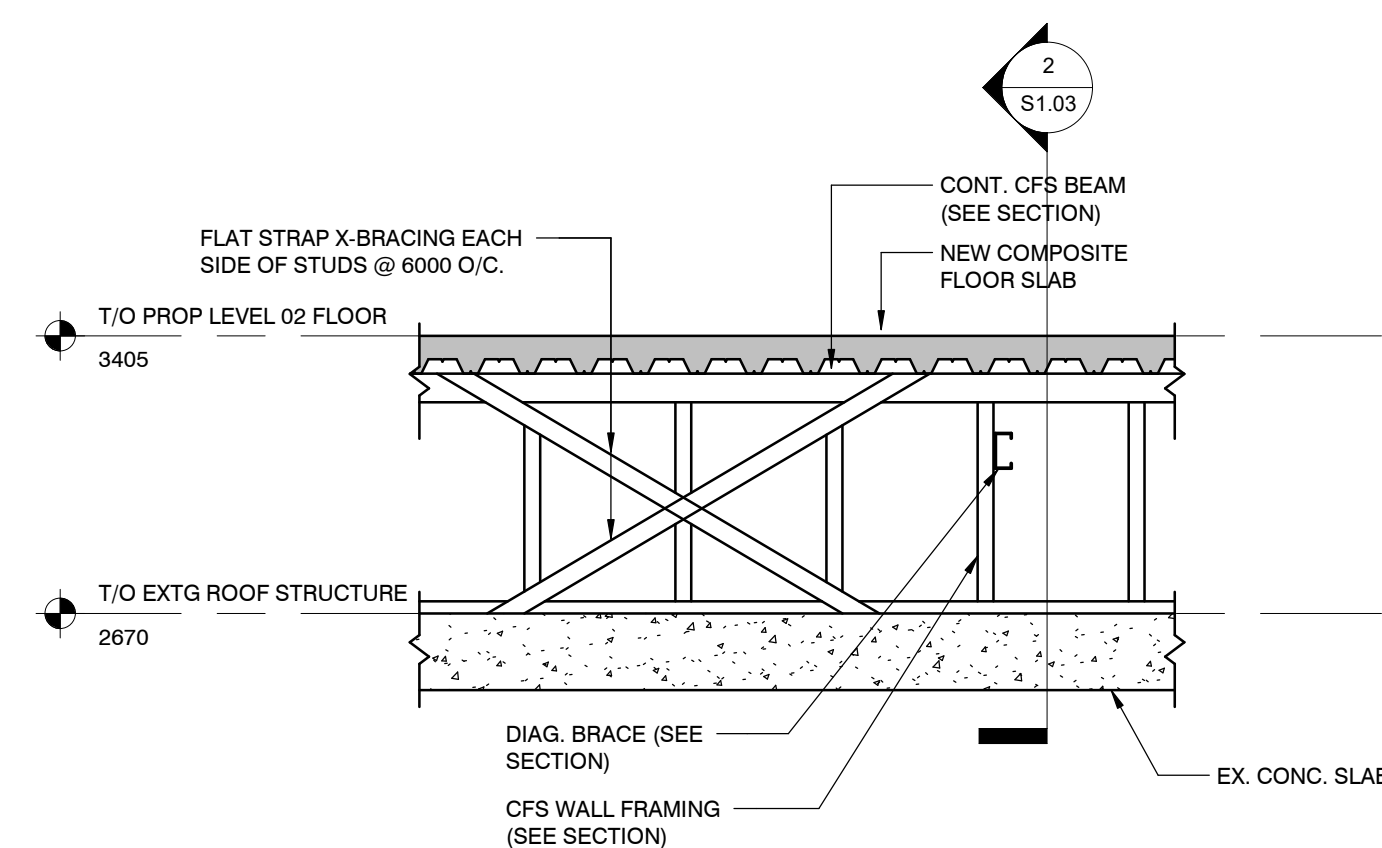


ELEVATED 2ND FLOOR FRAMING PLAN

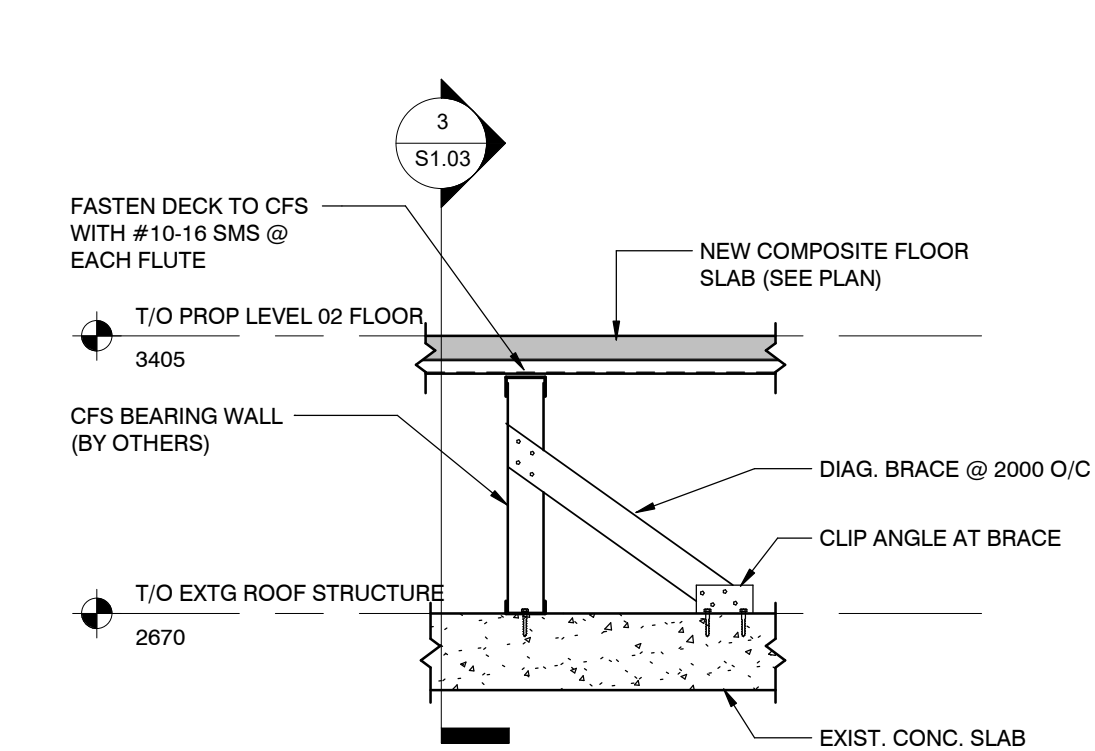
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NOTES:

1. TOP OF PROPOSED 2ND FLOOR TO BE 3405mm ABOVE DATUM (SEE ARCHITECTURAL)
2. TOP OF STEEL BEAMS SHOWN ON PLAN TO BE 100mm BELOW FINISHED FLOOR ELEVATION (SEE SECTIONS)
3. TOTAL DEAD LOAD IS 3.40 kPa, INCLUDING A PARTITION ALLOWANCE OF 1.0 kPa. SELF-WEIGHT OF 102mm THICK COMPOSITE FLOOR IS 1.90 kPa.
4. LIVE LOAD ON 2ND FLOOR IS A UNIFORM LOAD OF 4.8 kPa.
5. CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20MPa AT 28 DAYS.
6. ALL FRAME ALL OPENINGS WITH CFS WALLS AS PER TYPICAL DETAILS SHOWN ON PLAN.
7. DENOTES LOCATION OF ROUGH OPENING. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND OPENINGS NOT SHOWN. COORDINATE ALL OPENINGS WITH MECHANICAL DRAWINGS.
8. SUBMIT DETAILS FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN TO STRUCTURAL CONSULTANT FOR REVIEW.
9. REFER TO ALL OTHER TYPICAL NOTES & DETAILS.



3 TYPICAL KNEE WALL ELEVATION
1 : 20



2 TYPICAL KNEE WALL SECTION
1 : 20

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155 QUEEN STREET NORTH HAMILTON, ON
DRAWING

PROPOSED ELEVATED 2ND FLOOR FRAMING PLAN

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	As indicated		S1.03

WALL PLATE SCHED.

TYPE	PLATE SIZE THK.xLG.xWD.	ANCHORAGE
WPL 1	16x127x200	(2) 12mm Ø ROD w/300 EMBEDMENT

NOTES:

1. PLATE DIMENSION NOTED AS "LENGTH" TO BE PARALLEL WITH BEAM WEB.
2. FIELD WELD BEAM TO BEARING PLATE w/2- 6mmx40mm LG. FILLET WELDS (EACH SIDE)

"LENGTH"

EMBEDMENT

6mm TYP.

BAR/STRAP ANCHOR

50mm

TYP. ELEVATION DETAIL @ WALL PLATE (N.T.S.)

"LENGTH"

EMBEDMENT

6mm TYP.

BAR/STRAP ANCHOR

50mm

TYP. ELEVATION DETAIL @ WALL PLATE (N.T.S.)

"LENGTH"

EMBEDMENT

6mm TYP.

BAR/STRAP ANCHOR

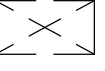
50mm

TYP. SECTION DETAIL @ CONT. WELD PLATE (N.T.S.)

ROOF FRAMING PLAN

1 : 50

NOTES:

- U/S METAL ROOF DECK 6907mm ABOVE DATUM (SEE ARCHITECTURAL DRAWINGS)
- REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, OPENINGS AND SLOPES NOT SHOWN ON THIS DRAWING
- REFER TO MECHANICAL DRAWINGS FOR ROOF TOP UNIT WEIGHTS AND LOCATIONS.
- REFER TO ELEVATION DETAILS FOR VERTICAL BRACING, MOMENT FRAMES AND GIRTS.
-  DENOTES LOCATION OF ROUGH OPENING. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND OPENINGS NOT SHOWN. COORDINATE ALL OPENINGS WITH MECHANICAL DRAWINGS.
- ROOF DEAD LOAD = 1.30 kPa
- [] DENOTES MOMENT CONNECTION VALUES AND ARE GIVEN IN kN.m (UNLESS NOTED OTHERWISE)
- REFER TO ALL OTHER TYPICAL NOTES & DETAILS.

ROOF DECK NOTES:

TRANSVERSE WELDS: 19mm PUDDLE WELD 36/7
SIDE LAP CONNECTION: BUTTON PUNCH @ 300mm O/C
PERIMETER WELDS: 19mm PUDDLE WELD @ 300mm O/C

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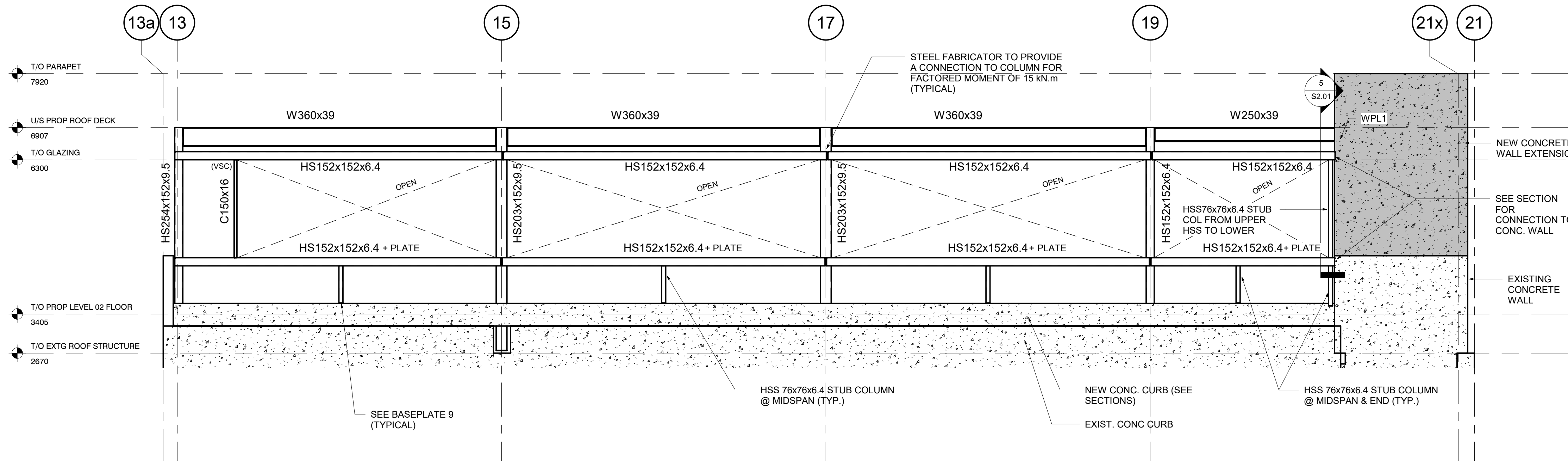
HAMILTON, ON

DRAWING

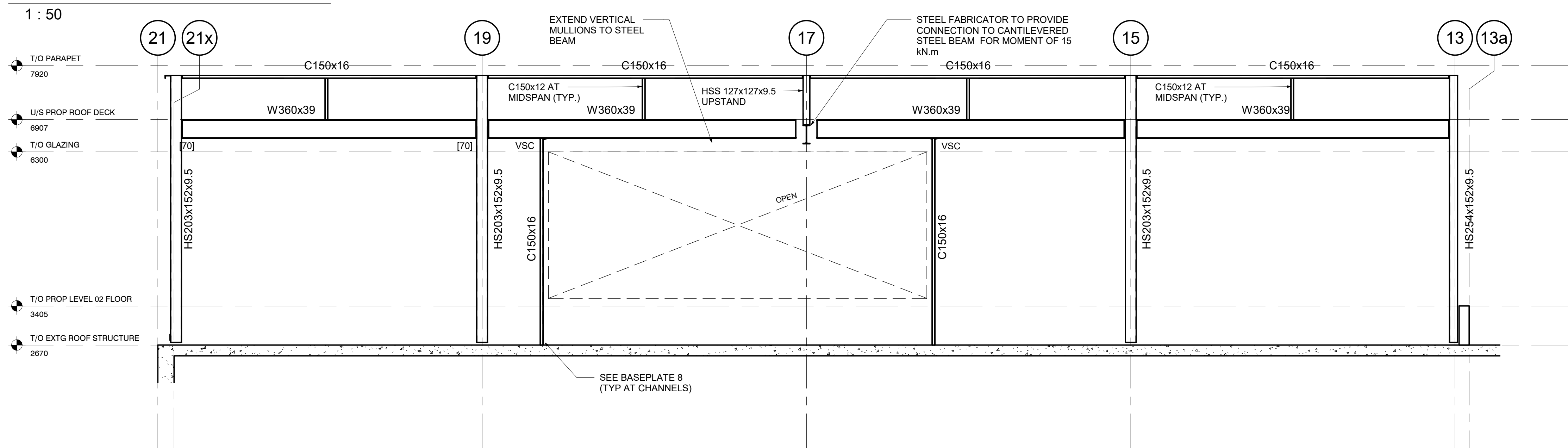
**PROPOSED ROOF
FRAMING PLAN**

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 50		

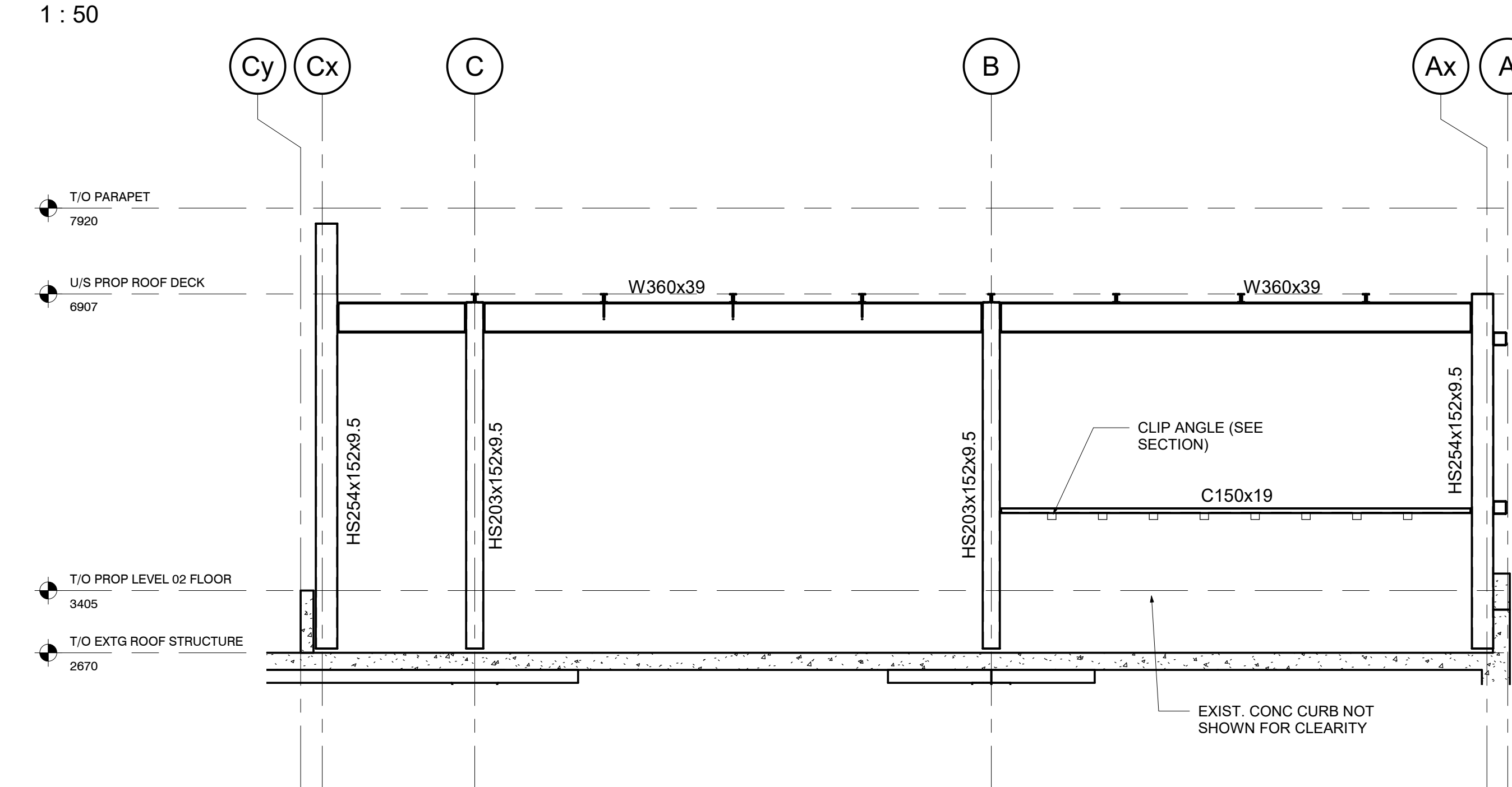
S1.04



ELEVATION GRID LINE Ax



ELEVATION GRID LINE Cx



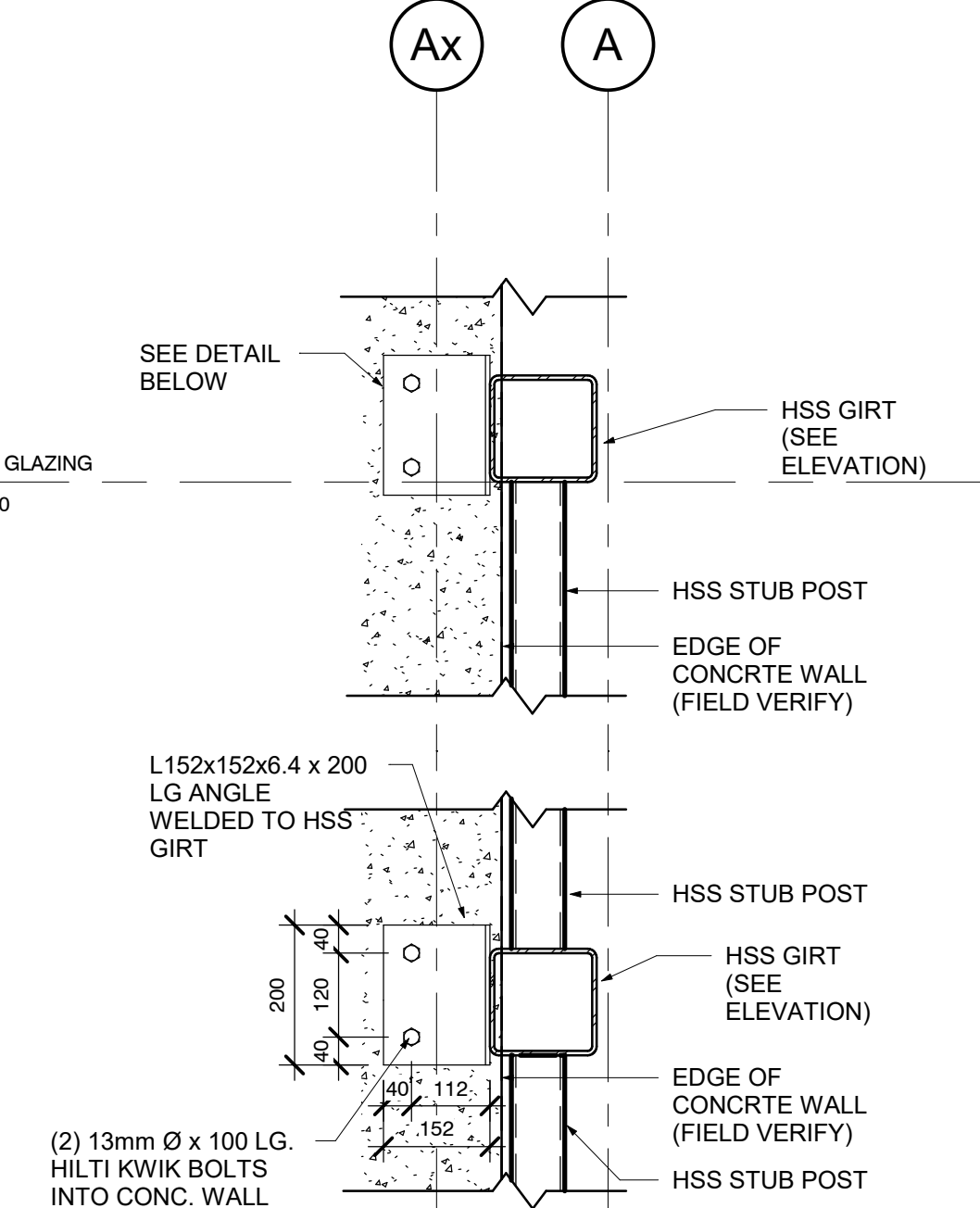
ELEVATION GRID LINE 13



ELEVATION GRID LINE 21x

NOTES:

- STEEL FABRICATOR TO DESIGN ALL STEEL CONNECTIONS. MOMENT CONNECTIONS TO BE DESIGNED FOR THE VALUE PROVIDED ON PLAN. PROVIDE STAMPED SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.
- FIELD VERIFY ALL DIMENSIONS WITH EXISTING CONDITIONS PRIOR TO FABRICATION.



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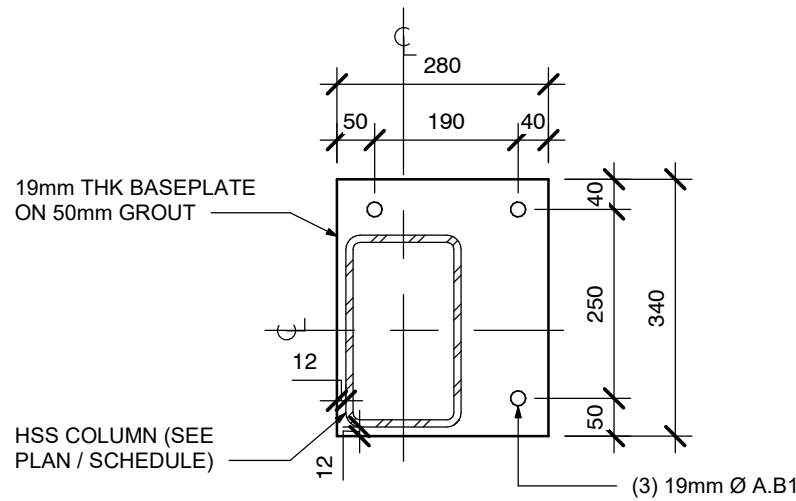
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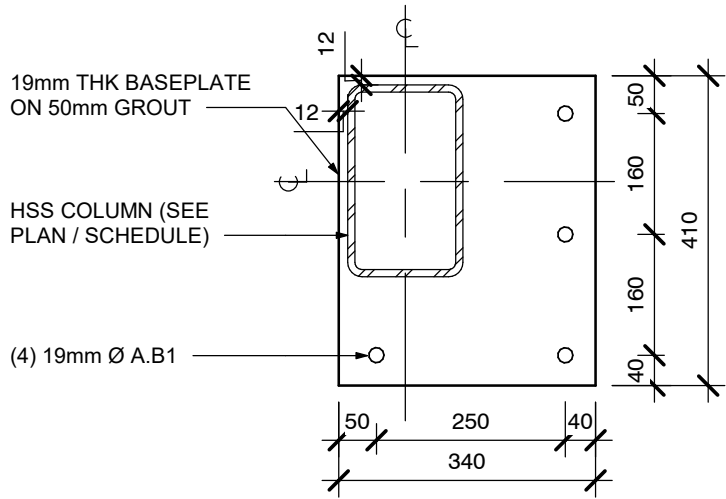
155 QUEEN STREET NORTH HAMILTON, ON

DRAWING
STEEL ELEVATIONS

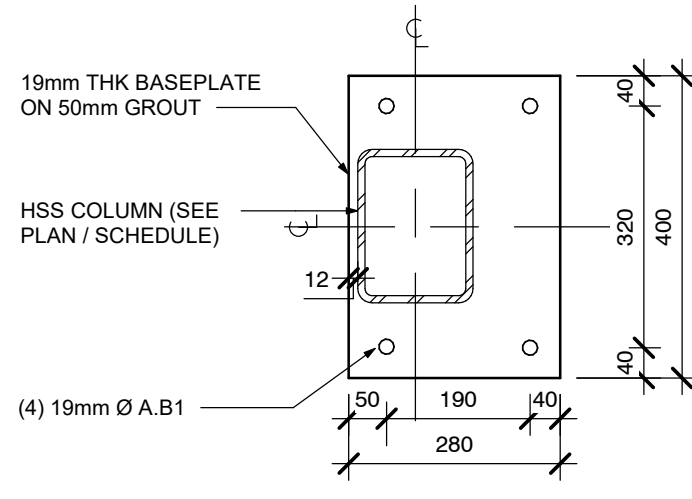
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Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	As indicated		S2.01



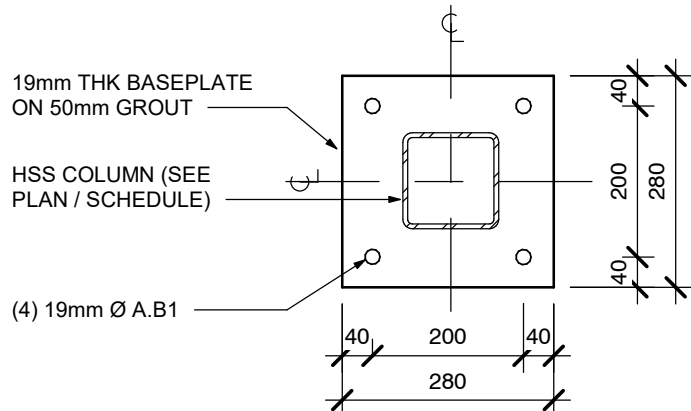
BASEPLATE 1 PLAN
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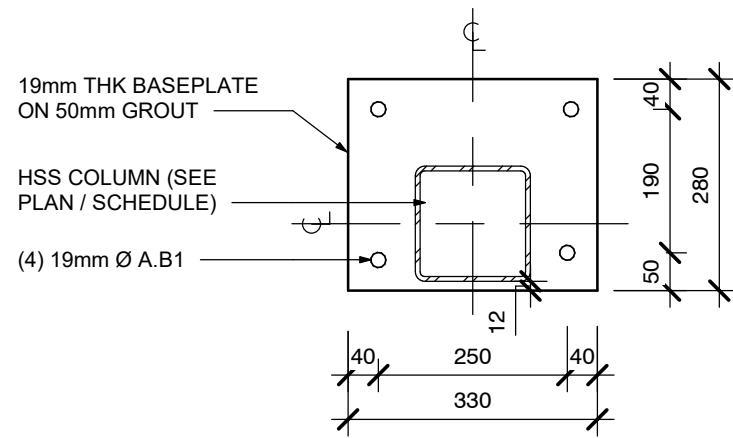
BASEPLATE 2 PLAN
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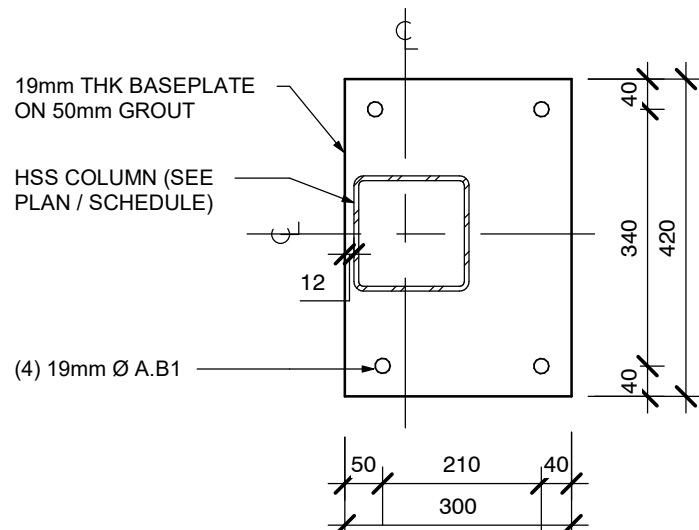
BASEPLATE 3 PLAN
(1:10)



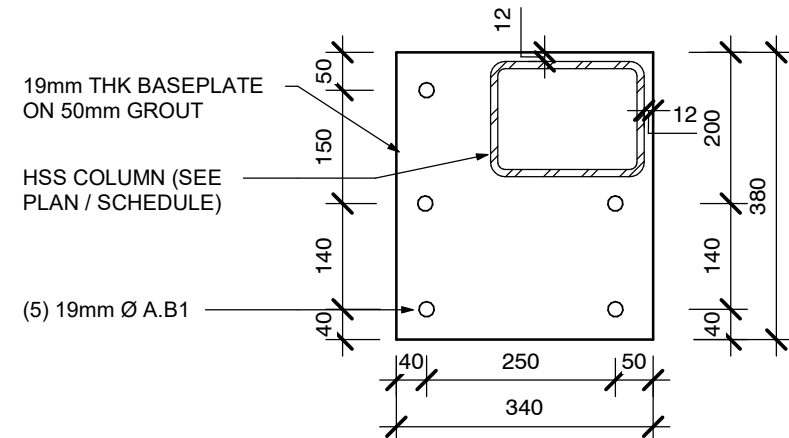
BASEPLATE 4 PLAN
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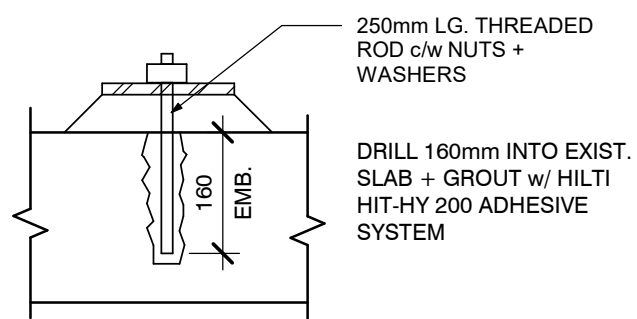
BASEPLATE 5 PLAN
(1:10)



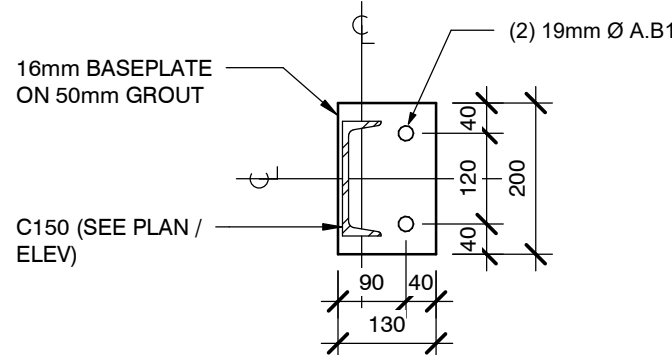
BASEPLATE 6 PLAN
(1:10)



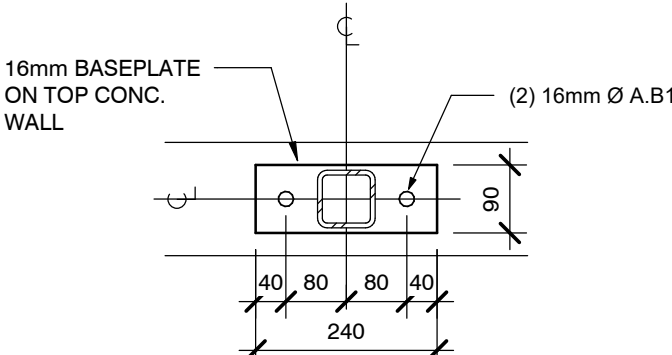
BASEPLATE 7 PLAN
(1:10)



A.B1



BASEPLATE 8 PLAN
(1:10)



BASEPLATE 9 PLAN
(1:10)

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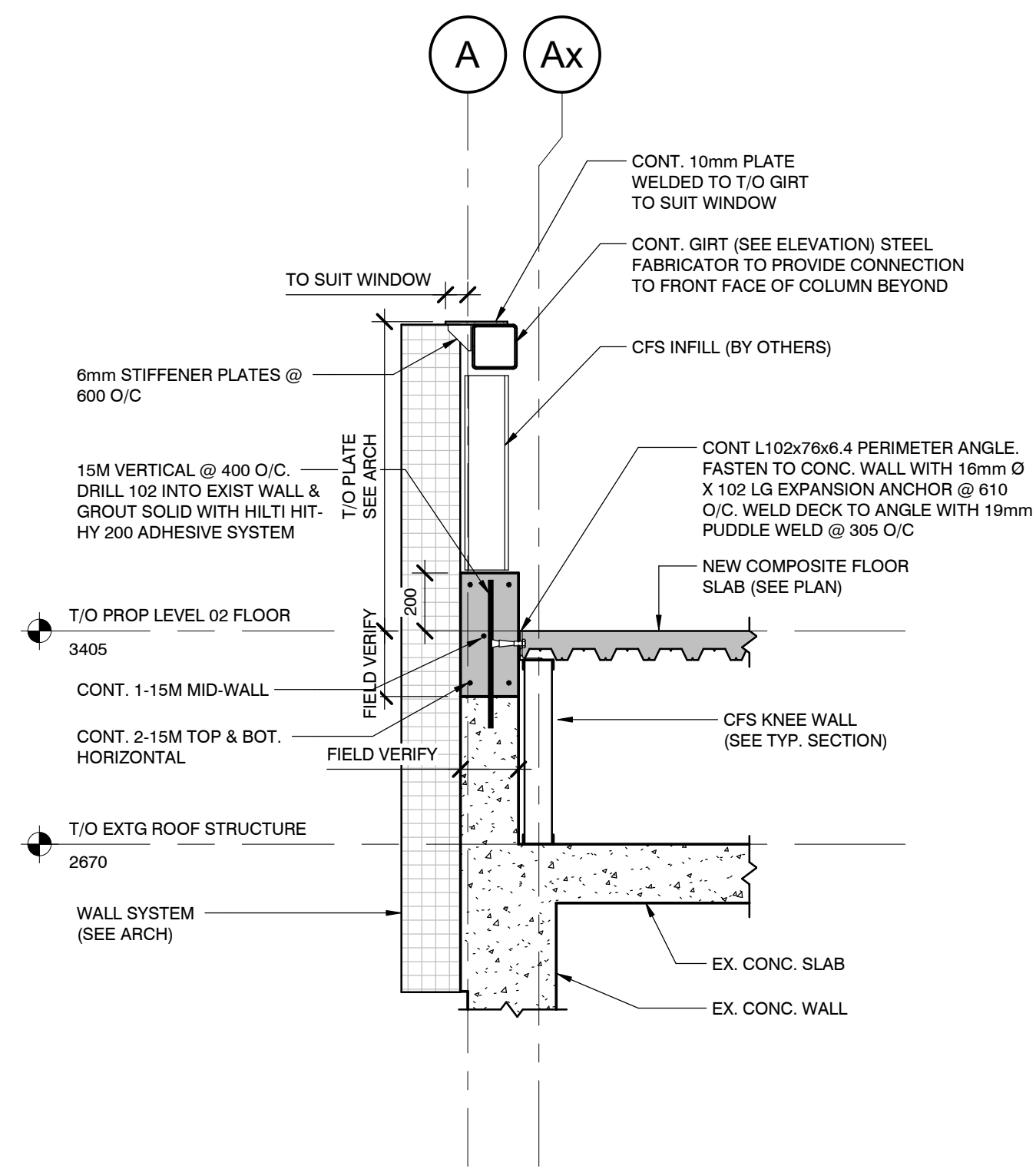
155 QUEEN STREET NORTH

HAMILTON, ON

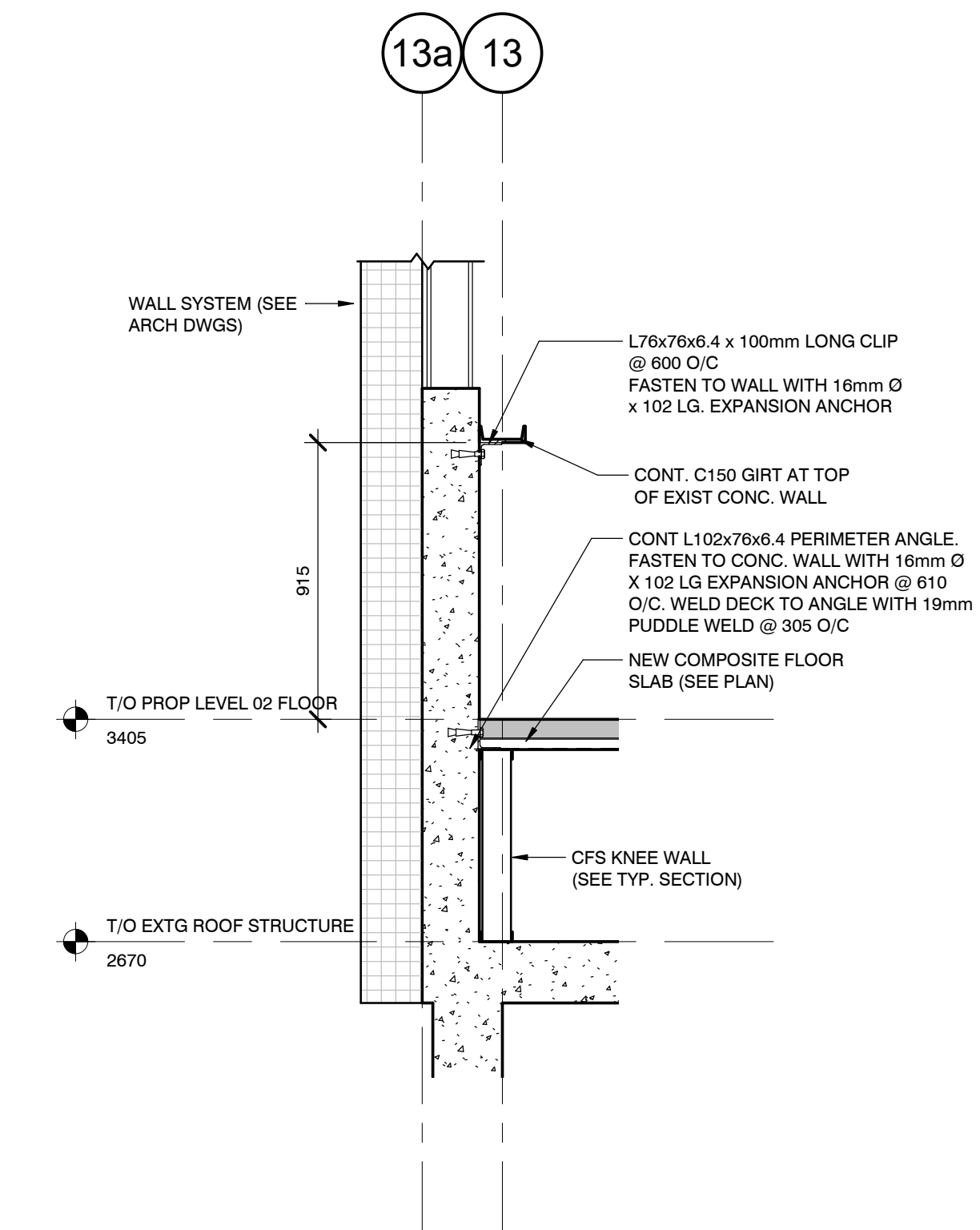
DRAWING

BASEPLATE DETAILS

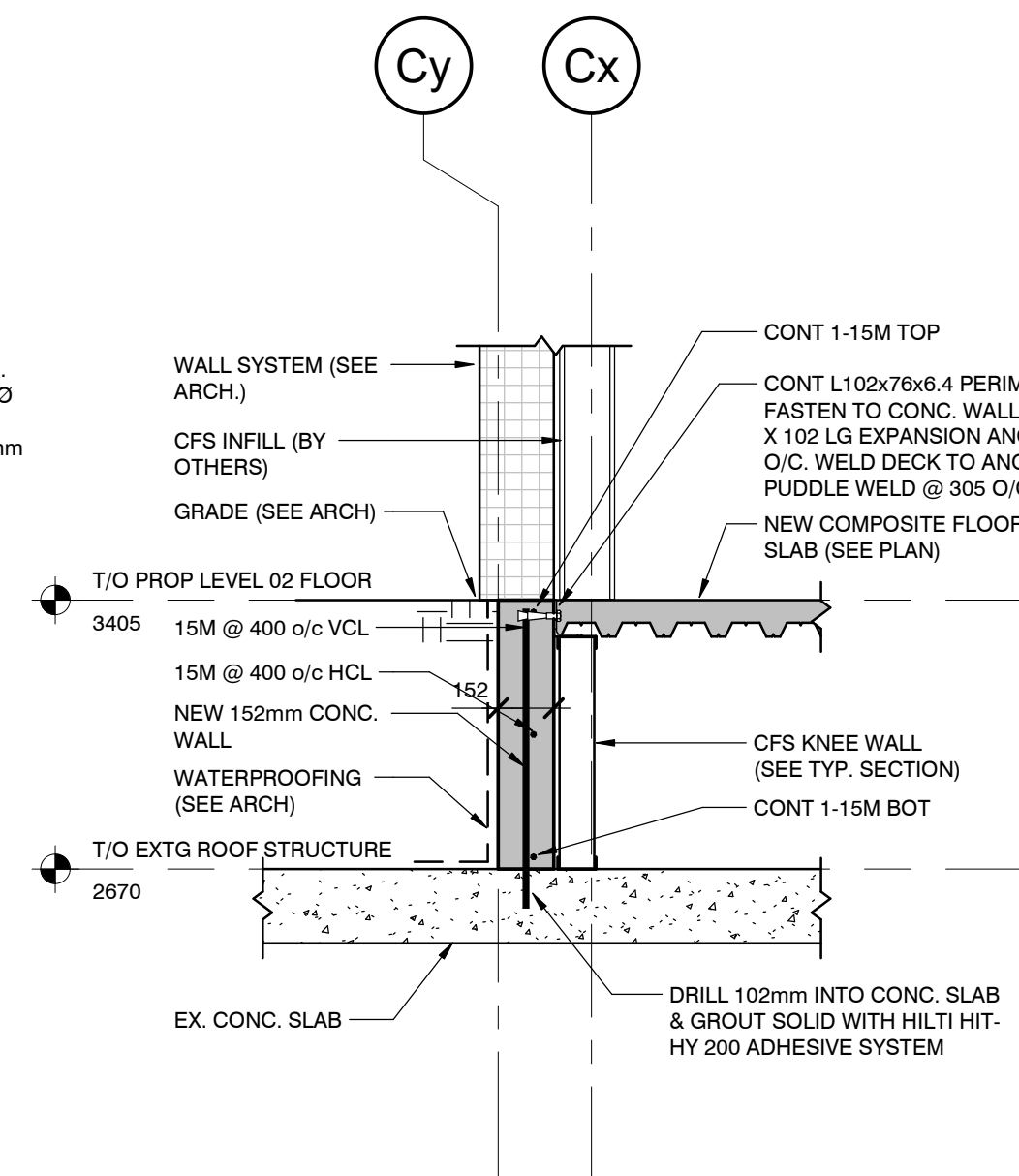
Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 10		S3.01



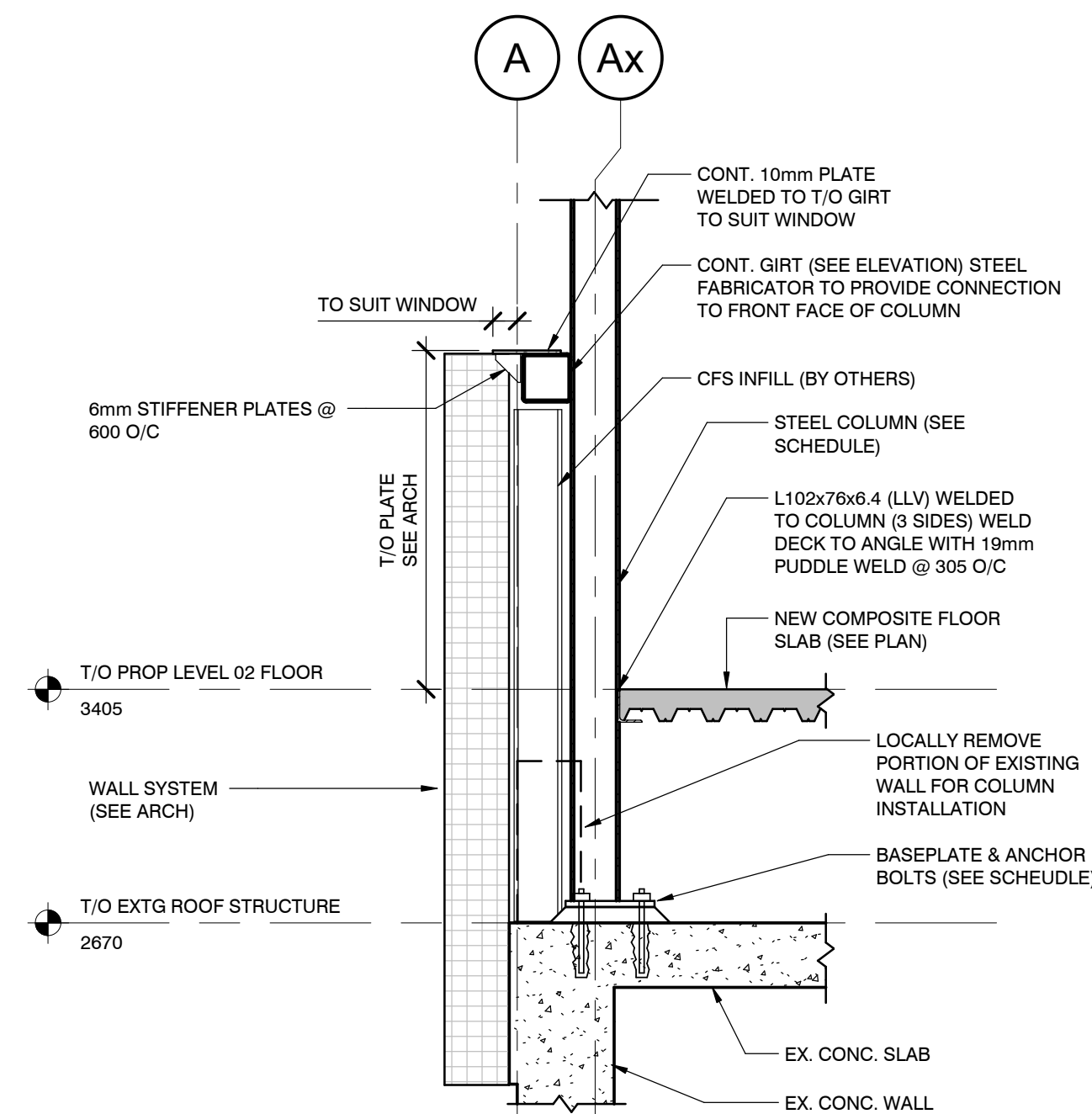
1 SECTION DETAIL
1 : 20



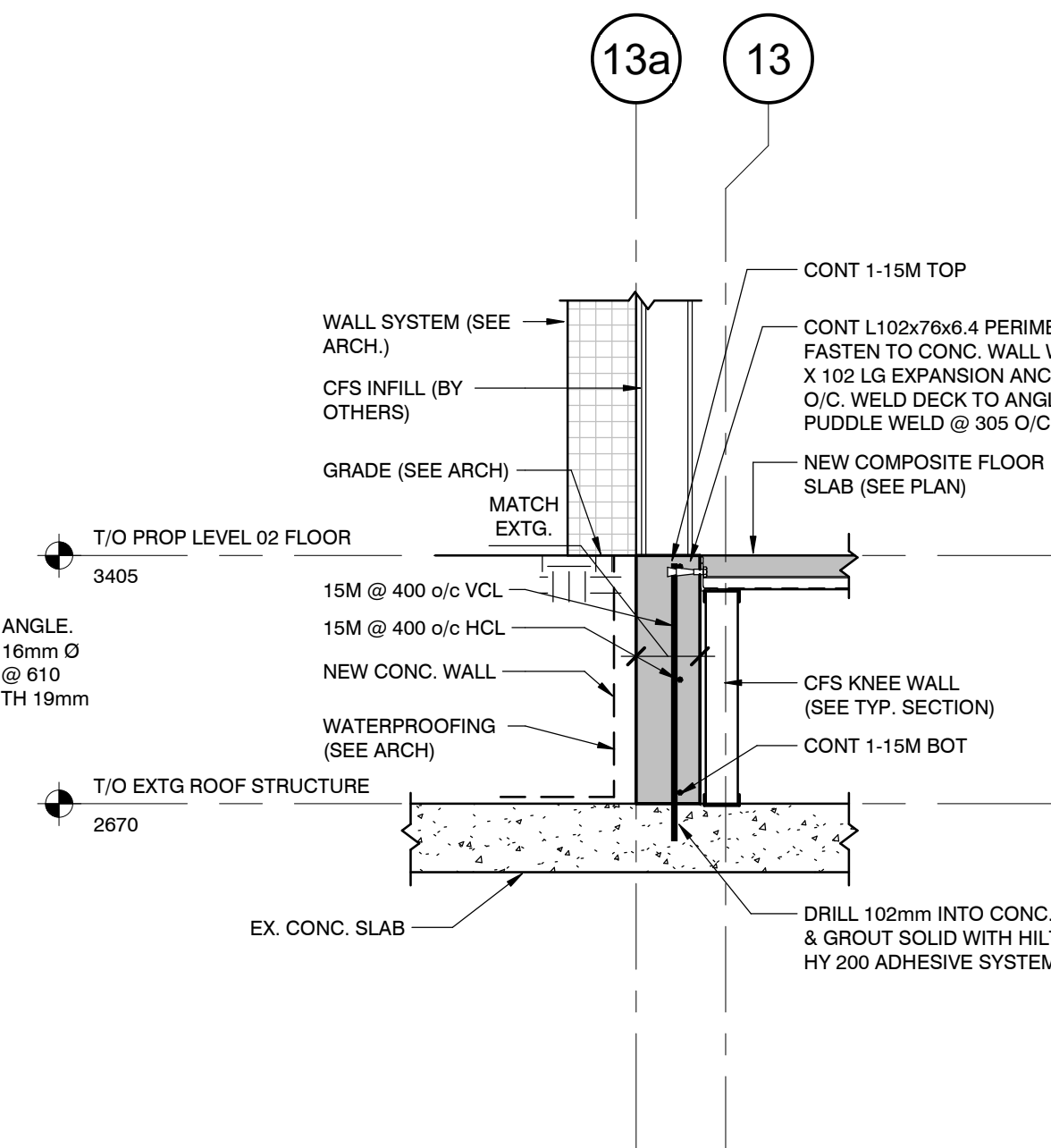
7 SECTION DETAIL
1 : 20



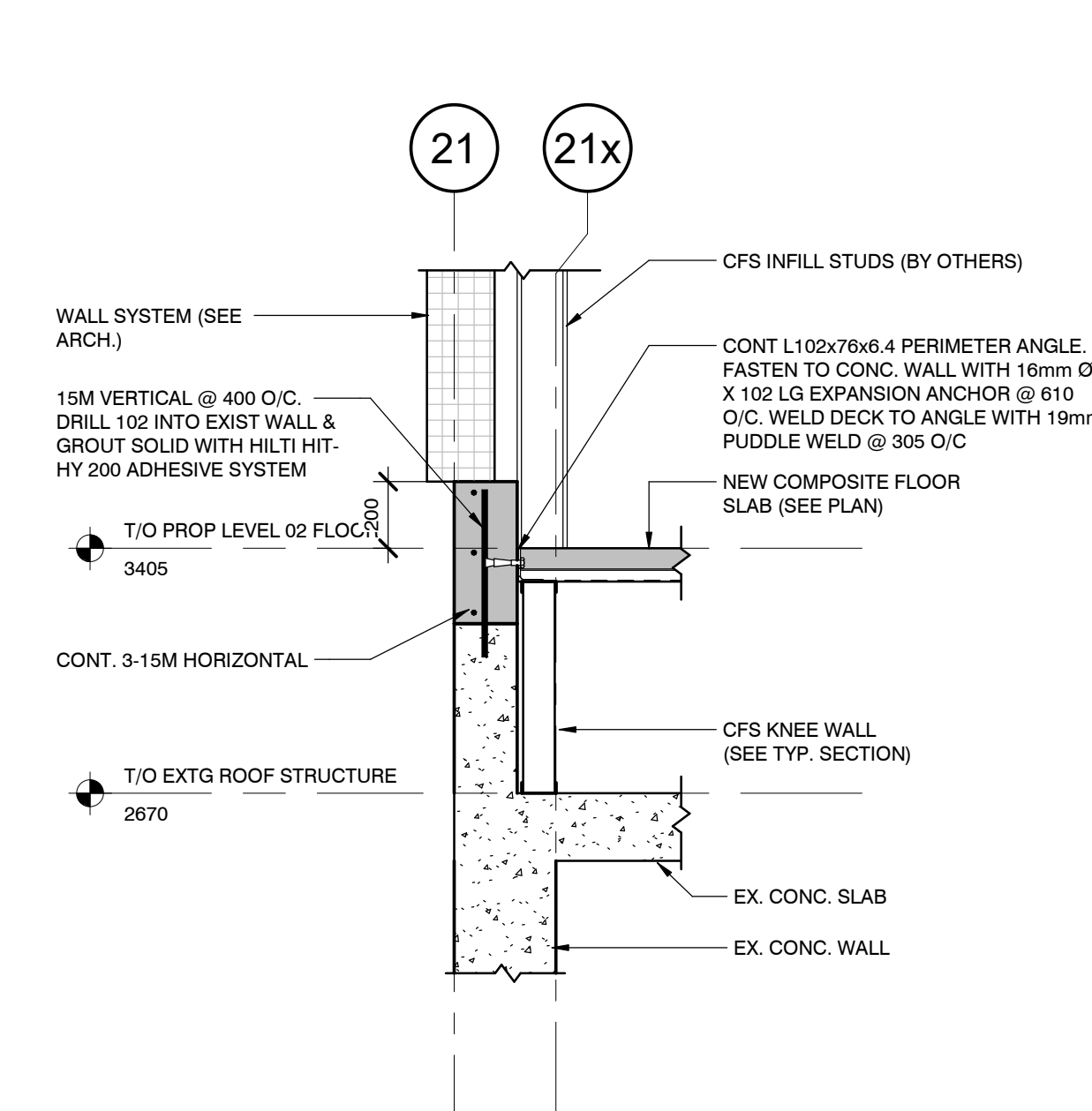
2 SECTION DETAIL
1 : 20



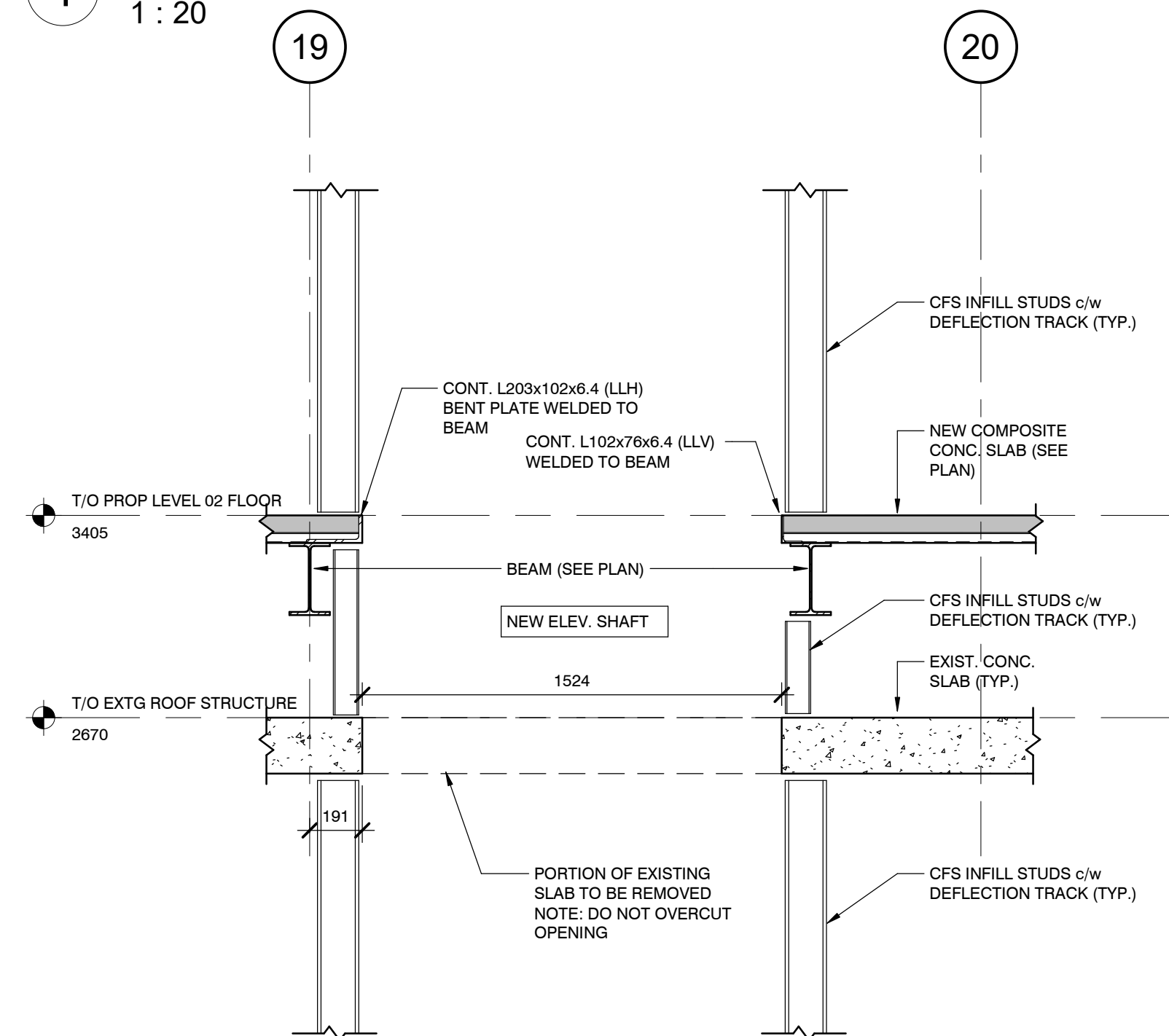
8 SECTION DETAIL
1 : 20



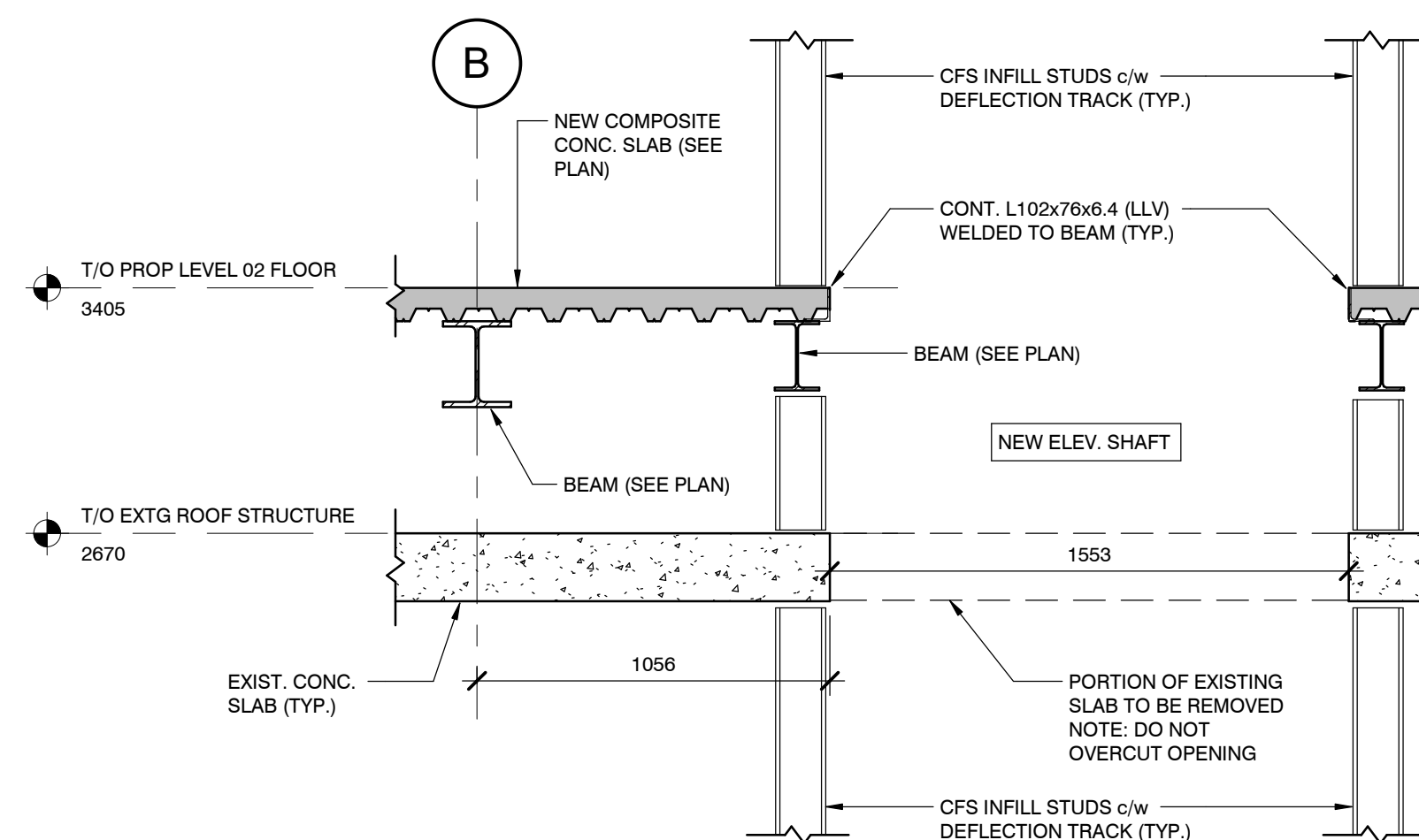
3 SECTION DETAIL
1 : 20



4 SECTION DETAIL
1 : 20



5 SECTION DETAIL
1 : 20



6 SECTION DETAIL
1 : 20

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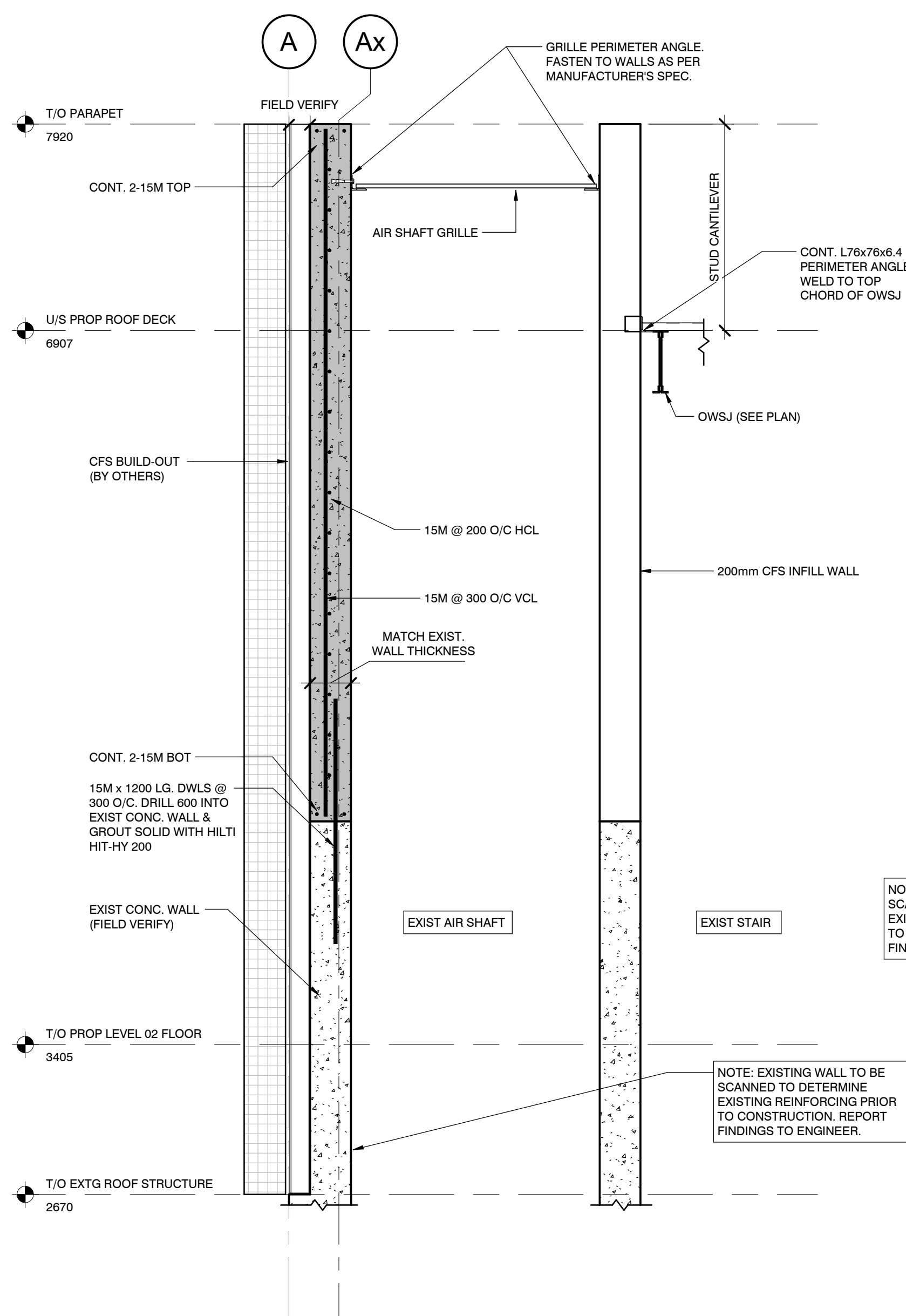
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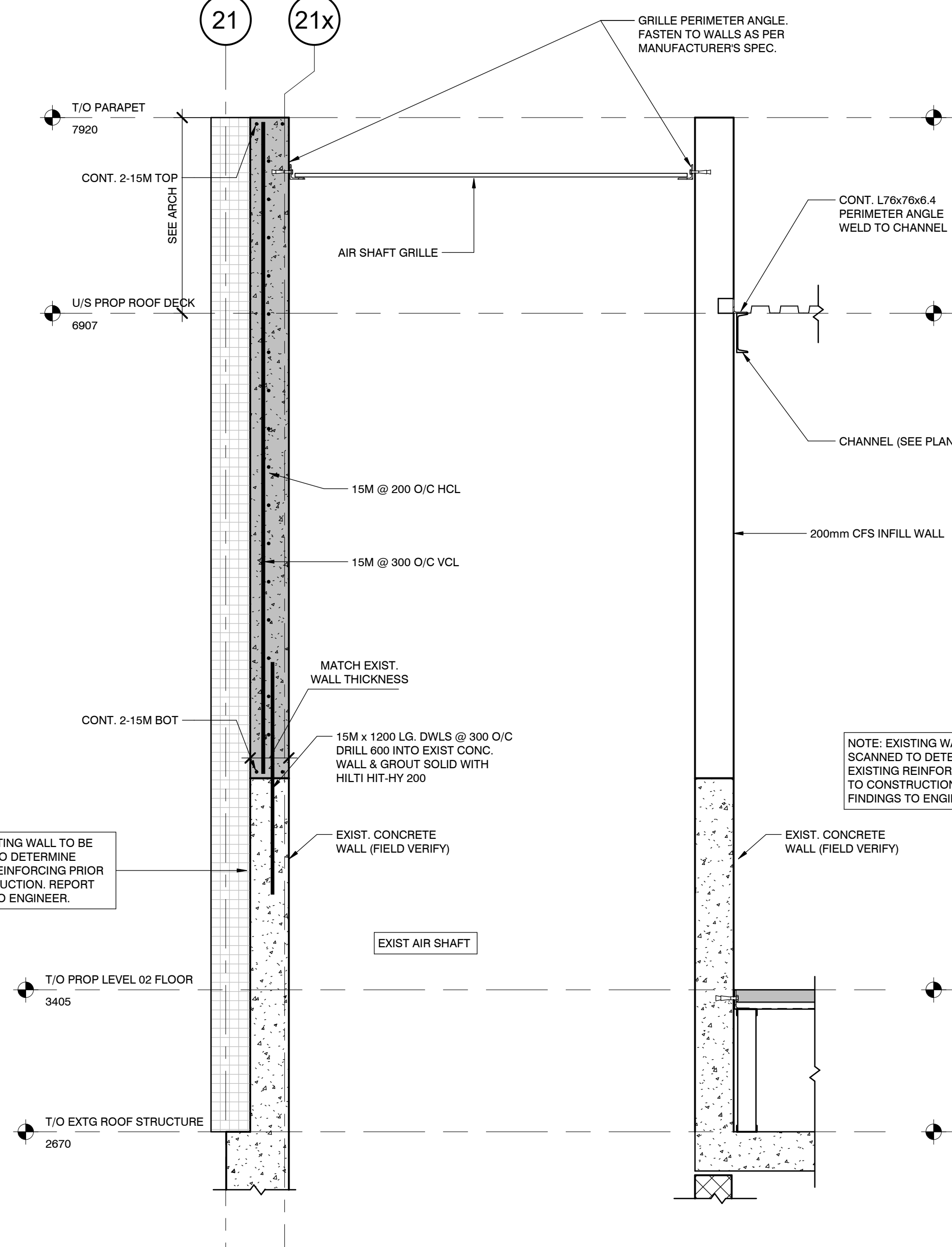
155 QUEEN STREET NORTH HAMILTON, ON
DRAWING

2ND FLOOR SECTIONS

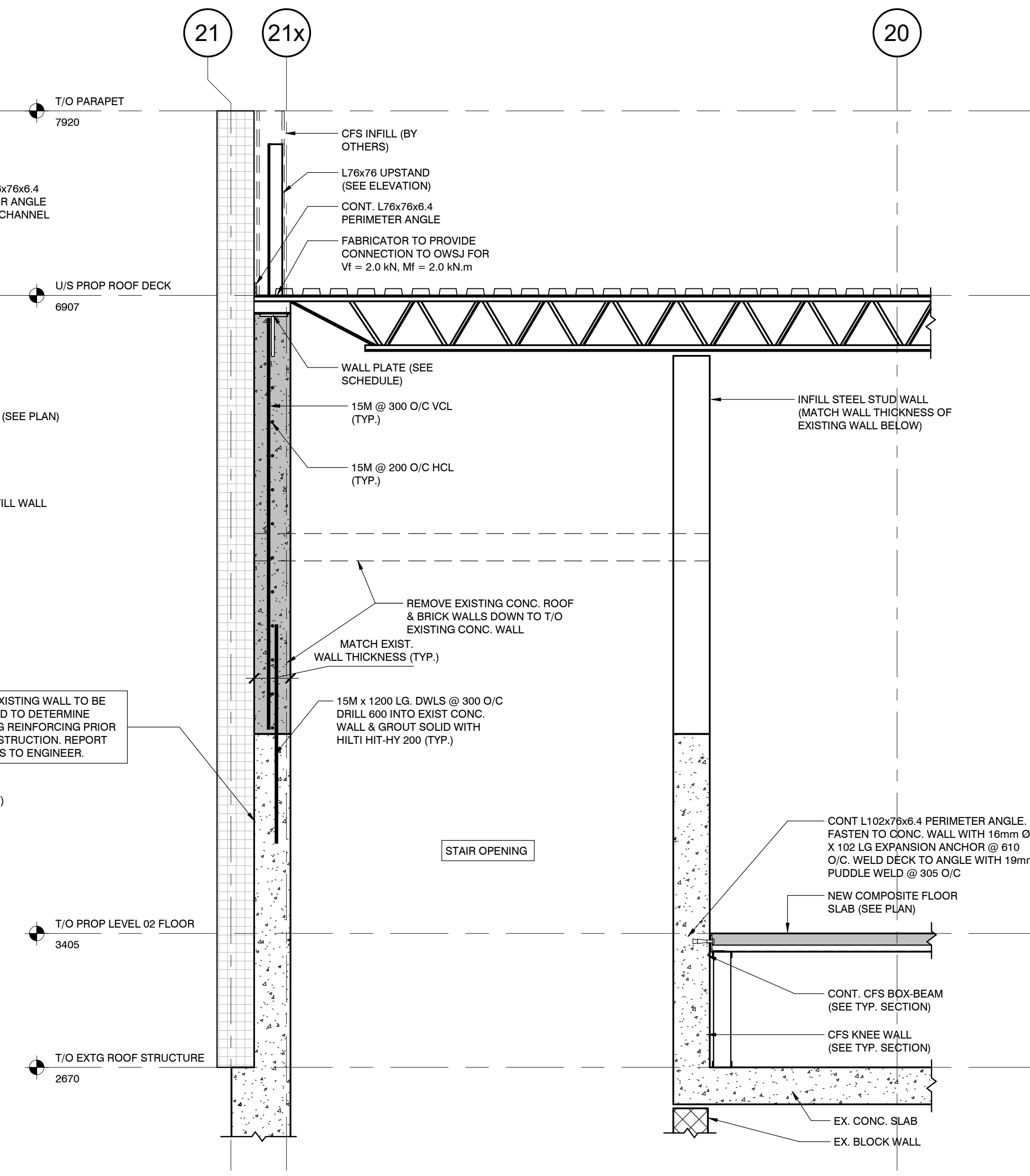
Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
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1 SECTION DETAIL
1 : 20



2 SECTION DETAIL
1 : 20



3 SECTION DETAIL
1 : 20

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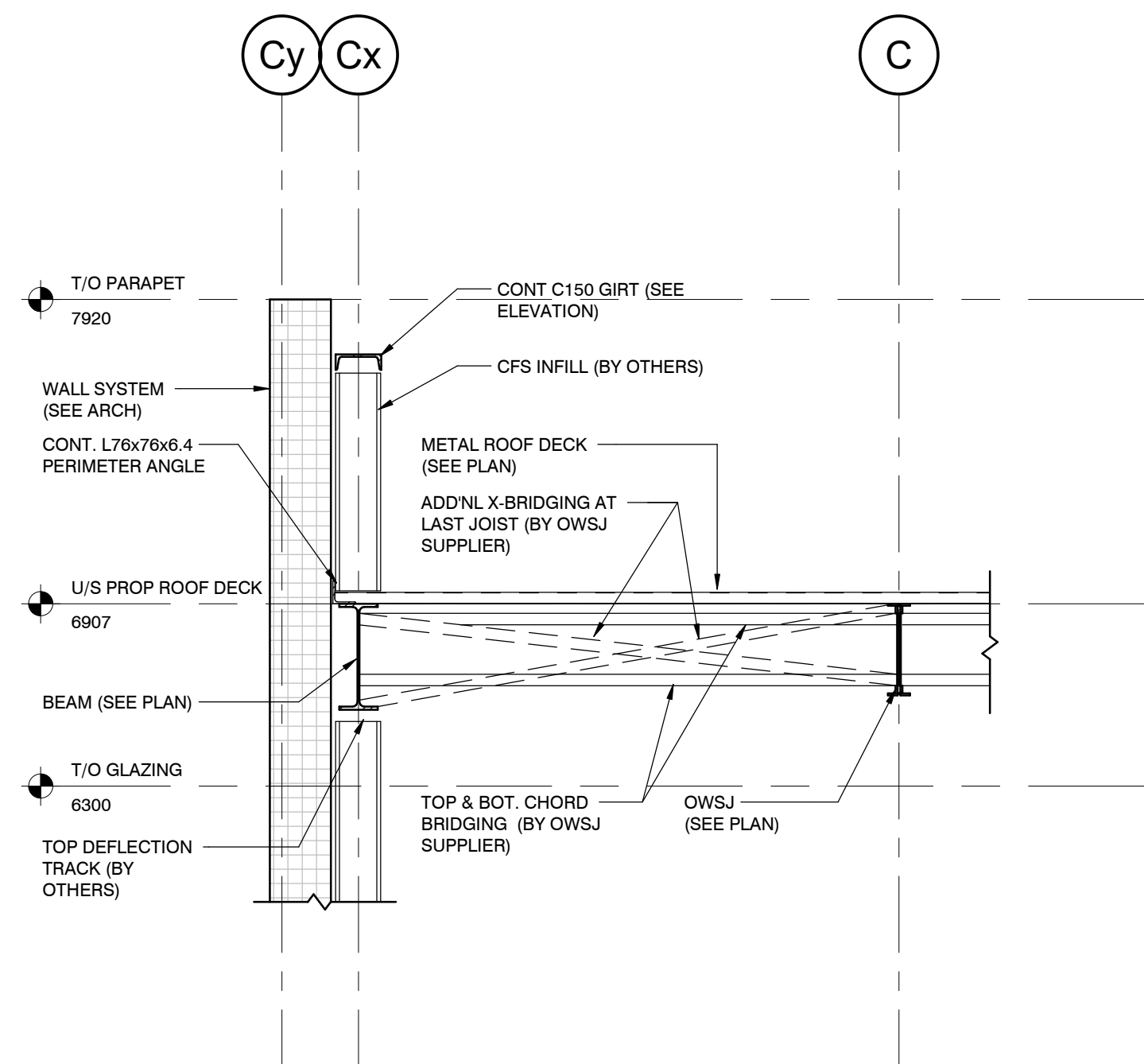
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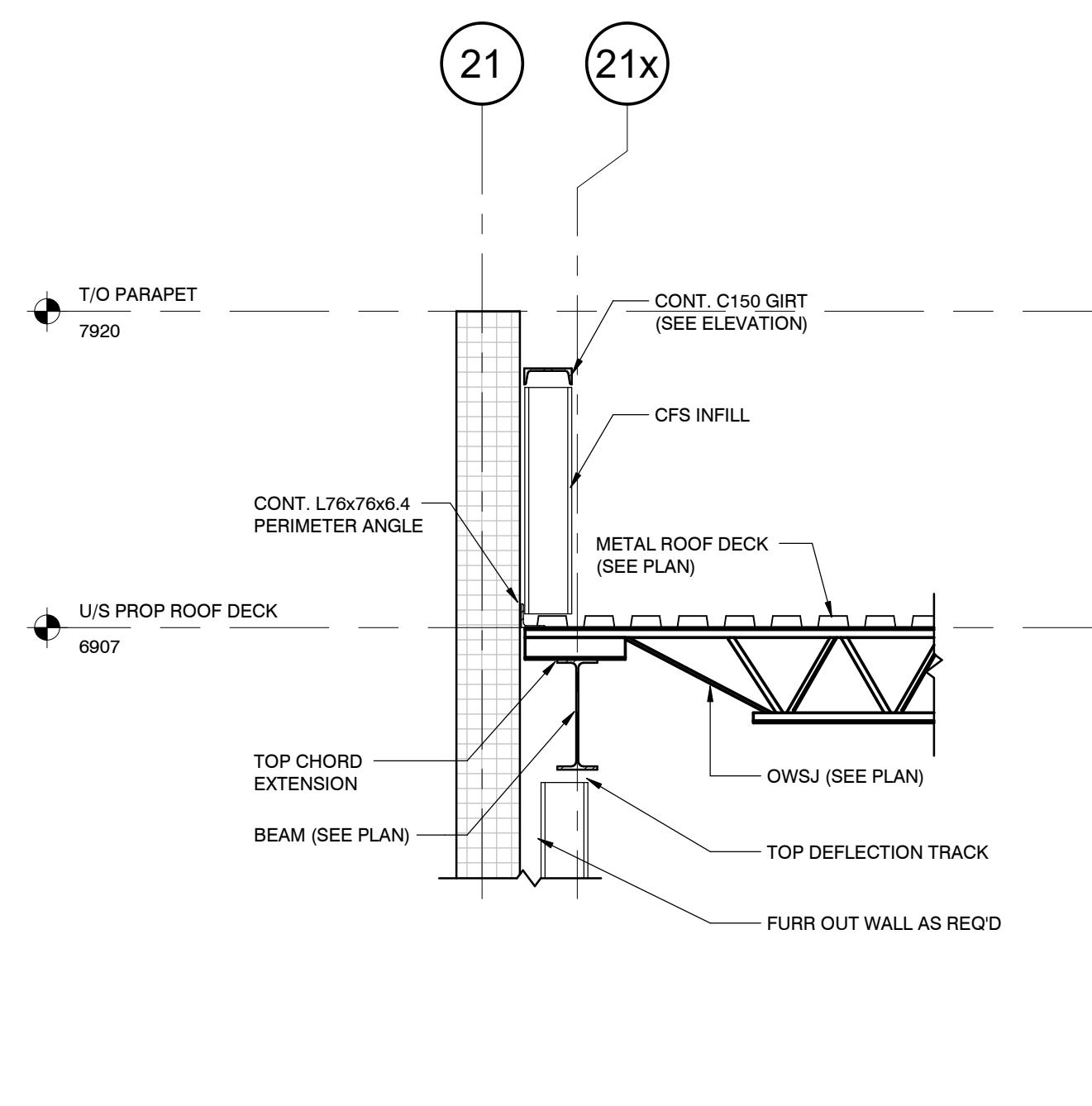
155 QUEEN STREET NORTH HAMILTON, ON
DRAWING

**2ND FLOOR SECTIONS
CONT'D**

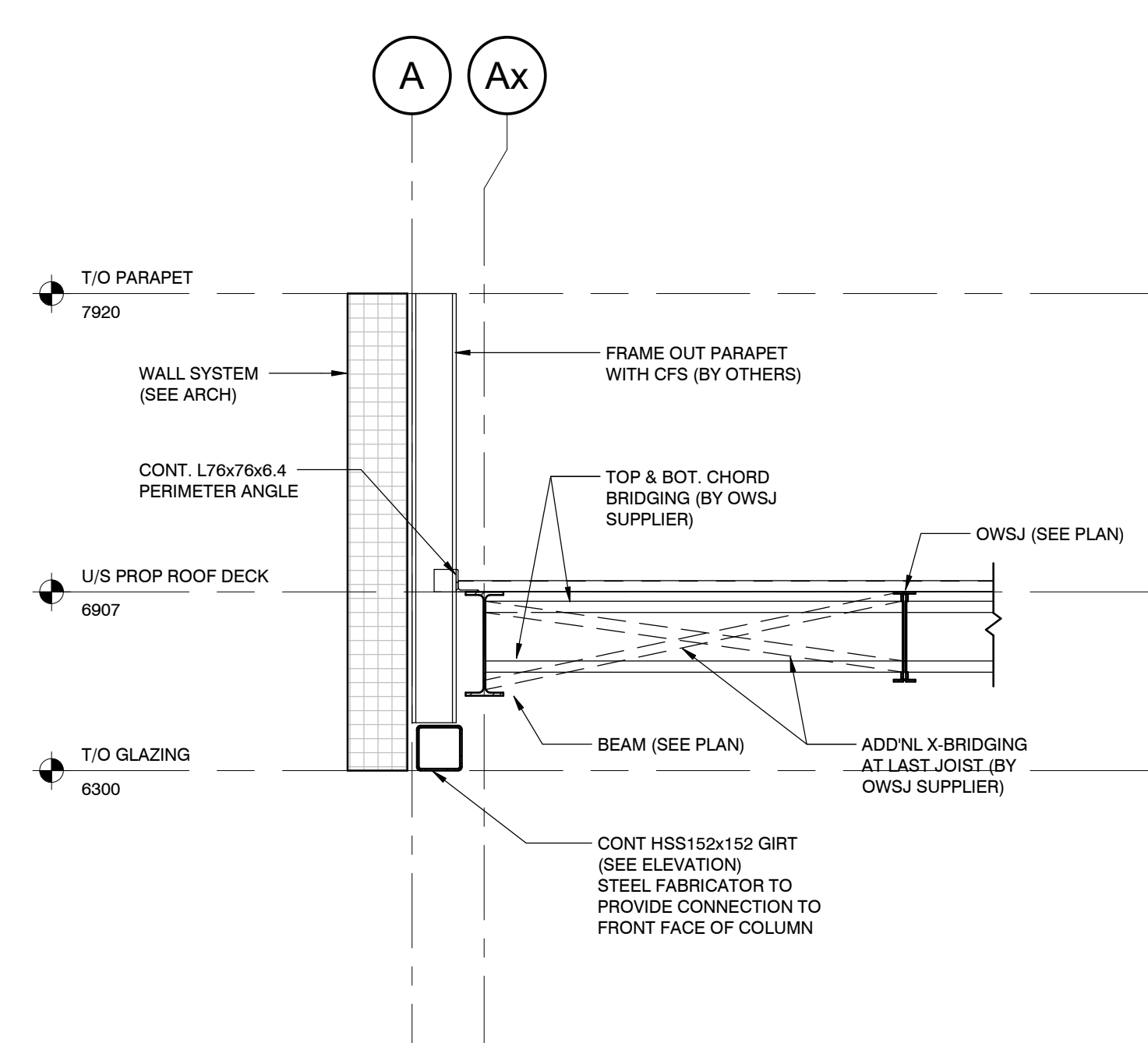
Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
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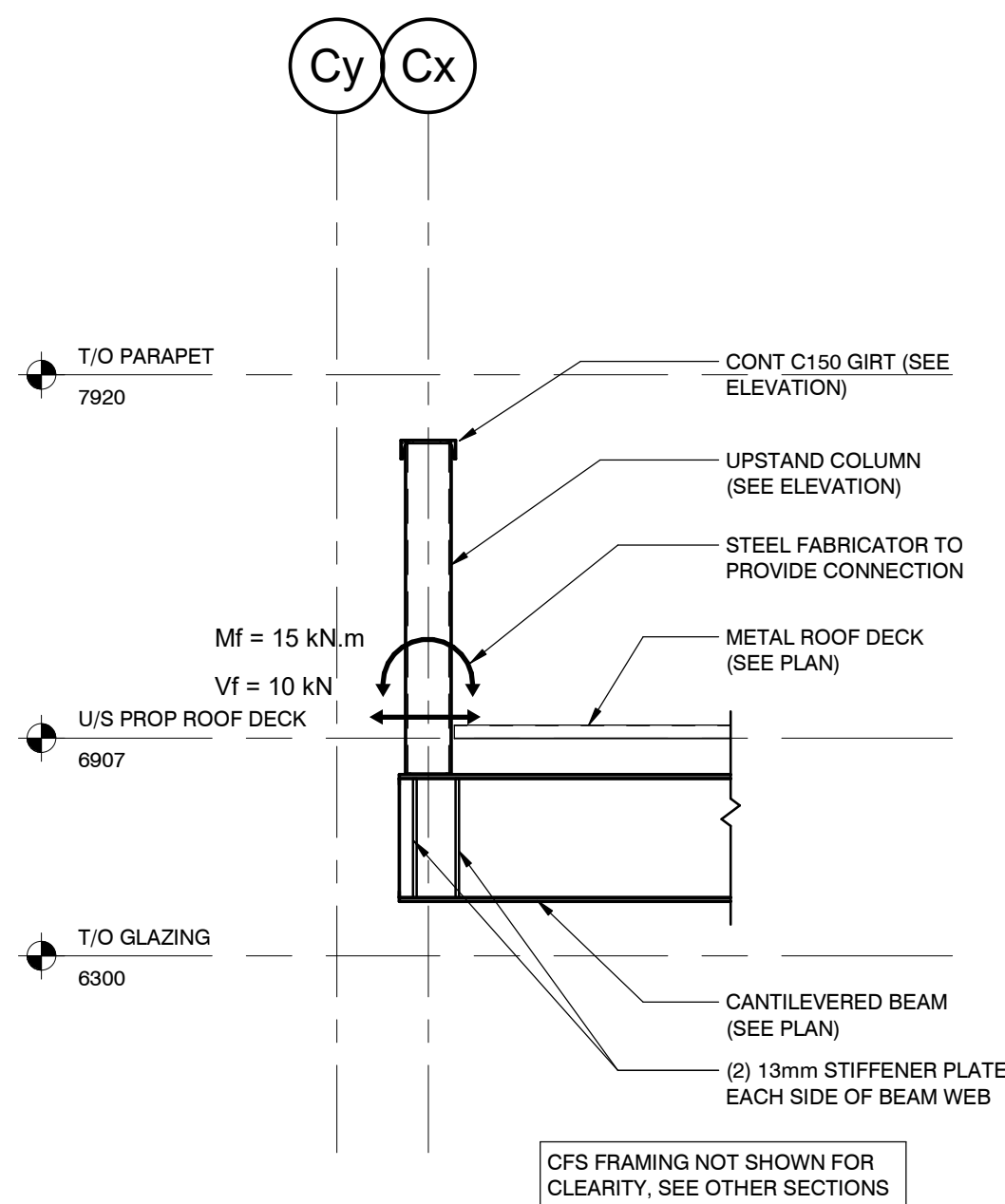
1 SECTION DETAIL
1 : 20



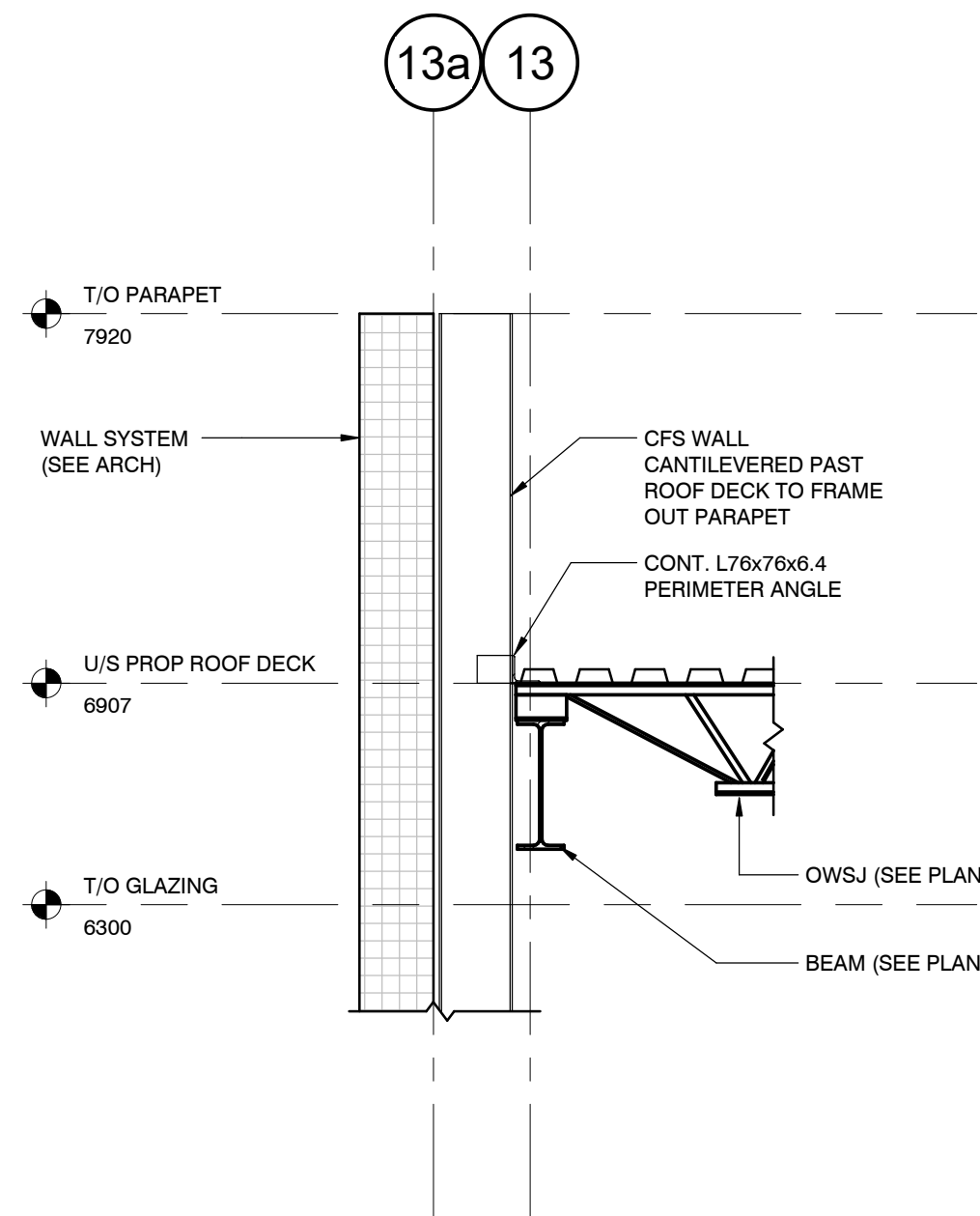
2 SECTION DETAIL
1 : 20



3 SECTION DETAIL
1 : 20



4 SECTION DETAIL
1 : 20



5 SECTION DETAIL
1 : 20

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ROOF SECTIONS

Project Manager:	DXF	Start Date:	AUG 2022
Design By:	MYB	Project No.:	50596-200
Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 20		S4.03

T/O PARAPET											T/O PARAPET
7920											7920
U/S PROP ROOF DECK											U/S PROP ROOF DECK
6907	HS203x152x9.5	HS127x127x6.4	HS127x127x6.4	HS127x127x6.4	HS152x152x6.4	HS203x152x9.5	HS89x89x9.5	HS127x127x6.4	HS254x152x9.5	HS203x152x9.5	6907
T/O PROP LEVEL 02 FLOOR											T/O PROP LEVEL 02 FLOOR
3405											3405
T/O EXTG ROOF STRUCTURE											T/O EXTG ROOF STRUCTURE
2670											2670
Column Locations	B-13	B-15	B-17	B-19	B-21x	C-13	C-15	C-19	Ax-13	Ax-15	

(TYP. ALL COL'S)

T/O PARAPET								T/O PARAPET
7920								7920
U/S PROP ROOF DECK								U/S PROP ROOF DECK
6907	HS203x152x9.5	HS152x152x6.4	HS152x152x6.4	HS254x152x9.5	HS203x152x9.5	HS203x152x9.5	HS203x152x9.5	6907
T/O PROP LEVEL 02 FLOOR								T/O PROP LEVEL 02 FLOOR
3405								3405
T/O EXTG ROOF STRUCTURE								T/O EXTG ROOF STRUCTURE
2670								2670
Column Locations	Ax-17	Ax-19	Bx-21x	Cx-13	Cx-15	Cx-19	Cx-21x	

- STEEL COLUMN SCHEDULE NOTES:**
1. VERIFY ALL ELEVATIONS WITH ARCHITECTRUAL DRAWINGS.
 2. PROVIDE 20mm THK. CAP PLATE FOR ALL COLUMNS.
 3. 6mm LEVELING PLATES ARE PERMITTED IF FABRICATOR CHOOSES SO.
 4. HDG = HOT DIPPED GALVANIZED.
 5. ALL BASE PLATES TO BEAR ON MIN. 50mm NON-SHRINK GROUT U.N.O.
 6. SCHEDULE IS FOR COLUMN SIZES ONLY, OFFSETS FROM GRIDS MUST BE VERIFIED WITH ARCHITECTUAL DRAWINGS.
 7. COORDINATE T/O COLUMN ELEVATION WITH BEAM DROPS FOR SLOPING IF INDICATED ON ROOF PLANS
 8. ELEVATIONS AT BASE COLUMNS ARE FOR U/S OF BASE PLATE

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DRAWING
COLUMN SCHEDULE

Project Manager:	DXF	Start Date:	AUG 2022
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Drawn By:	SYJ	Drawing No.:	
Scale:	1 : 50		S5.01