## Builder/Contractor Responsibilities

Drawing Validity- These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and guality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

Code Official Approval - It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection - The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3) (CSA/S16-09 Section 29).

Discrepancies - Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

<u>Materials by Others</u> - All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans - The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building on indicated on these drawings.

Foundation Design - The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that dequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items specinging to emotionment, bearing values, to toos and to other associated netwine embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA do Sections 3.2.2 and A3)

Shimming - In accordance with Section 6.10 of Chapter 4 Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim

Download panel installation manuals from:

Descargue los manuales de instalación del panel desde:

Building Descriptions								
Building ID	Width	Length	Height					
Building A	60'-0	120'-0	14'-0					

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# **TORO STEEL BUILDINGS**

1405 DENISON STREET, MARKHAM, ONTARIO L3R 5V2 TEL: (877) 870-8676 FAX: (877) 474-4445

DESIGN CRITERIA

Building Code	:	2015 National Building Code Of Canada
Building Importance Category:	:	Normal
Roof Dead Load Superimposed:		0.12 kPa // 2.48 psf
		0.19 kPa // 4.00 psf
(Other: 0.19 kPa // 4.00 psf)		
Roof Live Load :	:	1.00 kPa // 20.89 psf

Ground Snow Load (Ss) : 2.92 kPa // 61.03 psf : 0.40 kPa // 8.40 psf 
 Ground Snow Load (Ss)
 : 2.92 kPa // 61.03 psf

 Rain Load (St)
 : 0.40 kPa // 8.40 psf

 Basic Roof Snow Load Factor (Cb)
 : 0.80

 Importance Factor (Is)
 : 1.00

 Show Exposure Factor (Cw)
 : 1.00

 Roof Snow Load
 : 2.74 kPa // 57.22 psf

Wind Load '1/50 Wind Exposure (Ce) Building Internal Pressure Wind Importance Factor (Iw) Wind Topographic Factor

Seismic Data

: 0.48 kPa // 10.02 psf : Open Terrain : Category 2 : 1.00 : 1.00

Sa(0.2)=: 0.08 Sa(0.5)=: 0.06 Sa(1.0)=: 0.04 Sa(2.0)=: 0.02 Sa(5.0)=: 0.005 Sa(10.0)=: 0.002 Pga=: 0.05 Fa=: 1.24 Fv=: 1.55

## PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A457 of Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A651 Grade 55 with 55 ksi min. yield. For Canada, material properties ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

This project is designed using manufacturer's standard serviceability criteria. Generally this means that all deflections are within typica performance limits for normal occupancy and standard metal building products.

The metal building manufacturer has not designed the structure for snow accumulation loads at the ground level which may impose snow loads on the wall framing provided by the manufacturer.

The following criteria apply to projects in Canada. a. Erection tolerances must meet the requirements of CAN/CSA S16. b. Materials will be fabricated in a facility that has received the Canadian Welding Bureau certification CSA standard W47.1 in Division 1. c. Materials will be fabricated in a facility that has been certified to standard CSA A660 Certification of Manufactures of Steel Building Systems Systems.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

### DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length. The frame lateral drift or sidesway is based upon importance factors of 0.9 for specified show loads. The limits shown are at service loads unless indicated otherwise.

BUILDING DEFLECTION LIMITS .....: BLDG-A

Roof Limits		Rafters	Purlins	Panels
Live:	L/	180	180	60
Snow:	L/	180	180	60
Wind:	L/	180	180	60
Total Gravity:				
Total Uplift:				
Frame Limits		Sidesway		
Live:	н/	60		
Snow:				
Wind:	н/	60		
Seismic:				
Total Wind:	н/	60		
Total Gravity:				
Total Seismic:				
Wall Limits		Limit		
Total Wind Panels:	т./	60		
Total Wind Girts:				
Total Wind EW Columns:				

	1/2" DIA. A	A325 BOLT GRIP TABLE	
GRIP	LENGTH	BOLT LENGTH	NOTE: FULL THREAD
0 TO 9/16"	1 1/4" F.T.		ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE
Over 9/16" TO 1 1/16"	1 3/4" F.T.		END OF THE BOLT IS FLUSH
Over 1 1/16" TO 1 5/16"	2"		WITH THE FACE OF THE NUT.
Over 1 5/16" TO 1 9/16"	2 1/4"		
Over 1 9/16" TO 1 13/16"	2 1/2"		R REQUIRED ONLY WHEN SPECIFIED.
Over 1 13/16" TO 2 1/16"	2 3/4"	GRIP	R MAY BE LOCATED UNDER HEAD T, UNDER NUT, OR AT BOTH AT
LOCATIONS OF BOLTS LONGER TH NOTED ON ERECTION DRAWINGS	IAN 2 3/4"	ADD 5/3	IONS NOTED ON ERECTION DRAWINGS. 32" FOR EACH WASHER TO IAL THICKNESS TO DETERMINE GRIP.
F.T. DENOTES FULLY THREADED		MATERI	IAL THICKNESS TO DETERMINE GRIP.



ra=: 1.2. Fv=: 1.5: Soils Site Class:: D Importance Factor (Ie) : 1.00 Transverse Response Modification Rd: 1.50 Longitudinal Response Modification Rd: 1.50 Overstrength Factor Ro : 1.30

rawing Index     p       Description     a       Anchor Rod Setting Plan     a       Anchor Rod Details     a       Reactions     a       Cover Sheet     a       Primary Steel     a       Roof Framing     a       Sidewall SWA     a       Sidewall SWC     a       Endwall EWB     a	
Description       Anchor Rod Setting Plan       Anchor Rod Details       Reactions       Cover Sheet       Primary Steel       Roof Framing       Sidewall SWA       Sidewall SWC	
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Cover Sheet       Primary Steel       Roof Framing       Sidewall SWA       Sidewall SWC	
Primary Steel Roof Framing Roof Sheeting Sidewall SWA Sidewall SWC	
Roof Framing     Sidewall       Sidewall     SWA       Sidewall     SWC	
Roof Sheeting     9       Sidewall SWA     5       Sidewall SWC     0	
Endwall EWB	
Endwall EWD	
Cross Section at Frame Line 2	
Cross Section at Frame Line 3	
Cross Section at Frame Line 4	
Cross Section at Frame Line 5	+
Cross Section at Frame Line 7	
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Checked by: DC 7/12/2 Project Engineer: JDM	33
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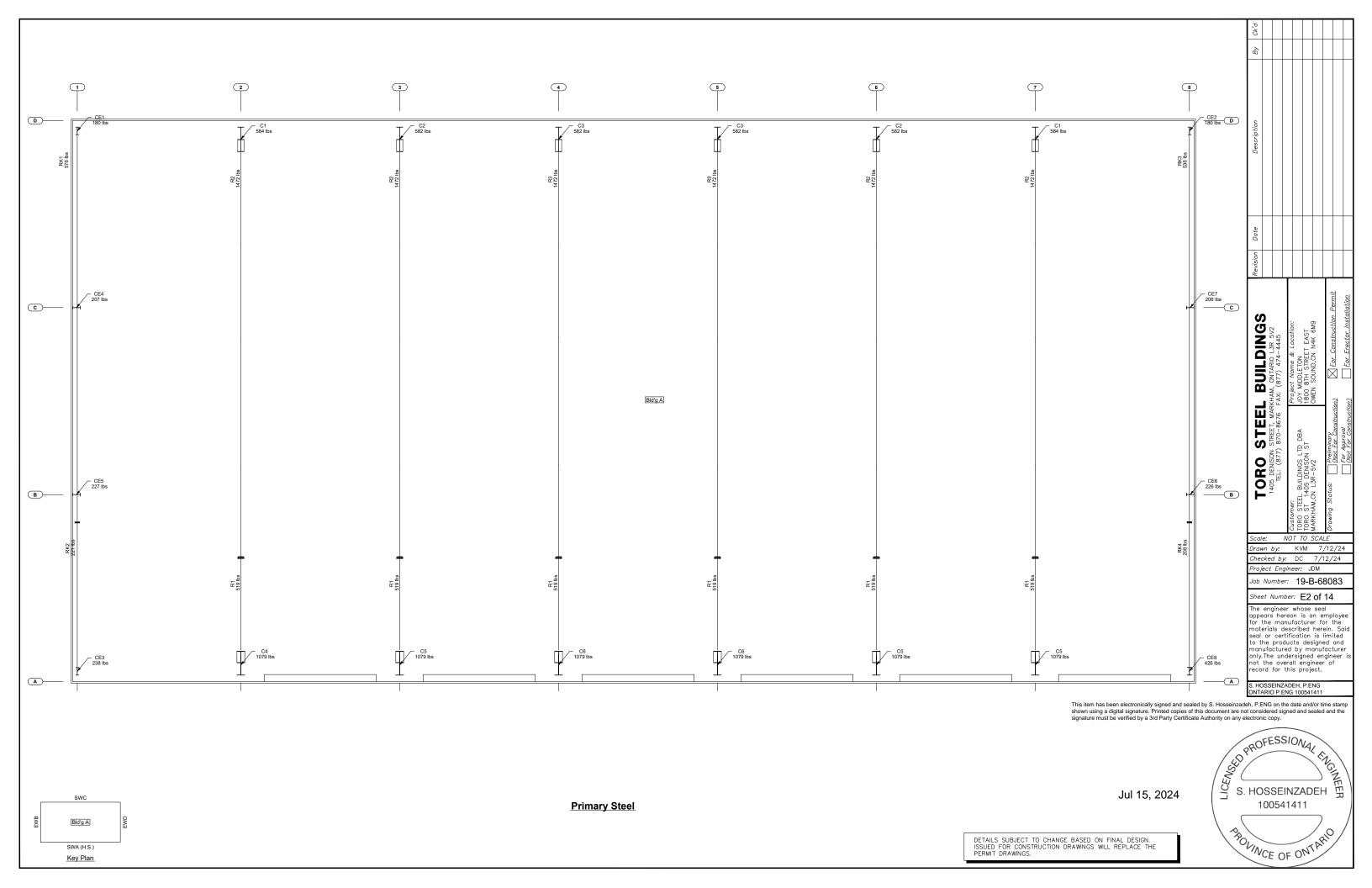
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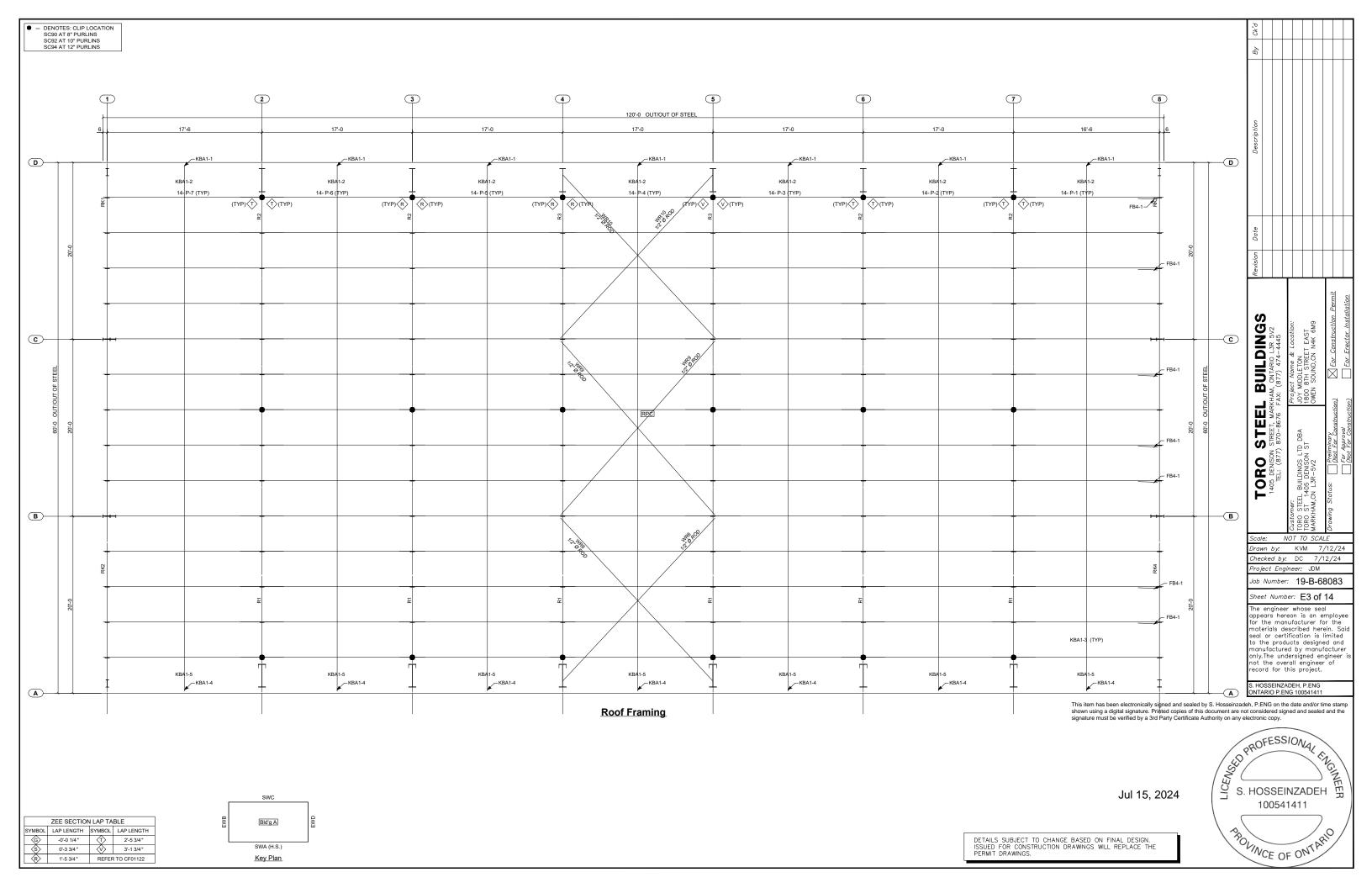
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DETAILS SUBJECT TO CHANGE BASED ON FINAL DE R CONSTRUCTION DRAWINGS WILL REPLACE

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## Non-Standard PBR Roof Panel Fasteners

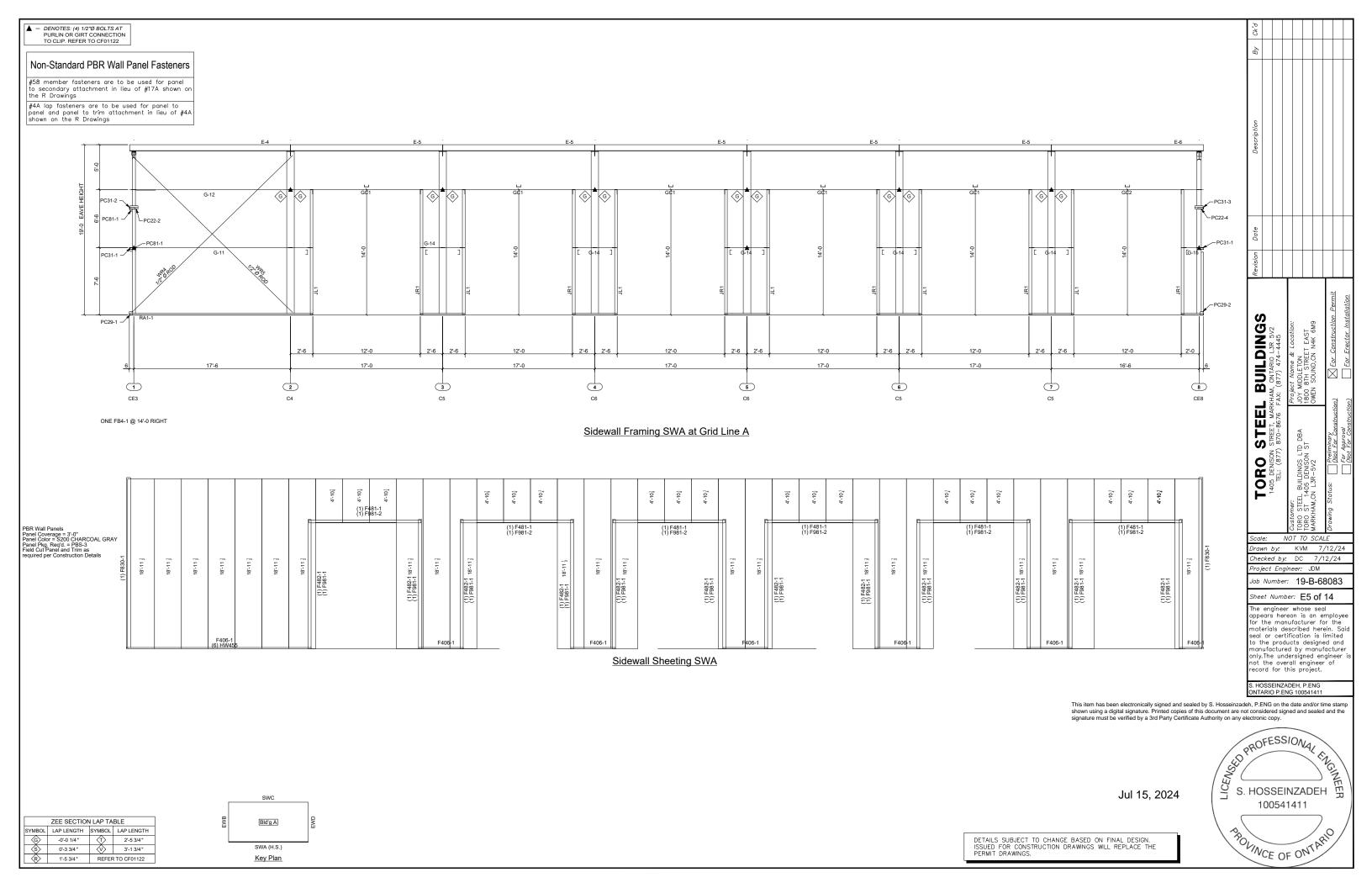
#58 member fasteners are to be used for panel to secondary attachment in lieu of #3 shown on the R Drawings

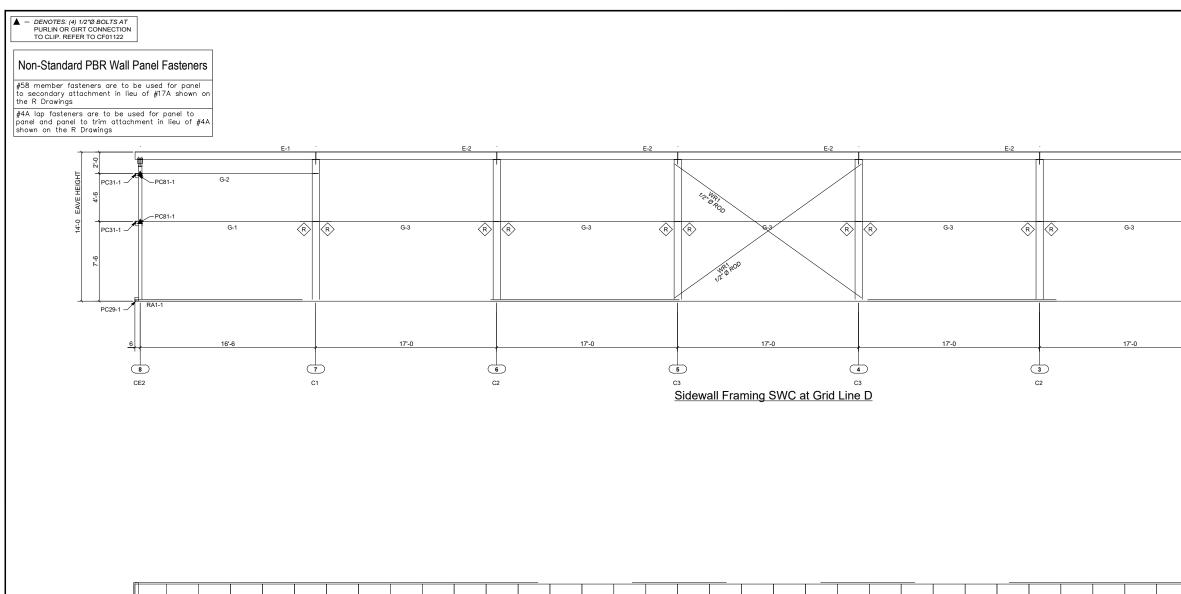
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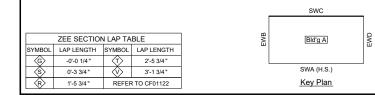
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Sidewall Sheeting SWC



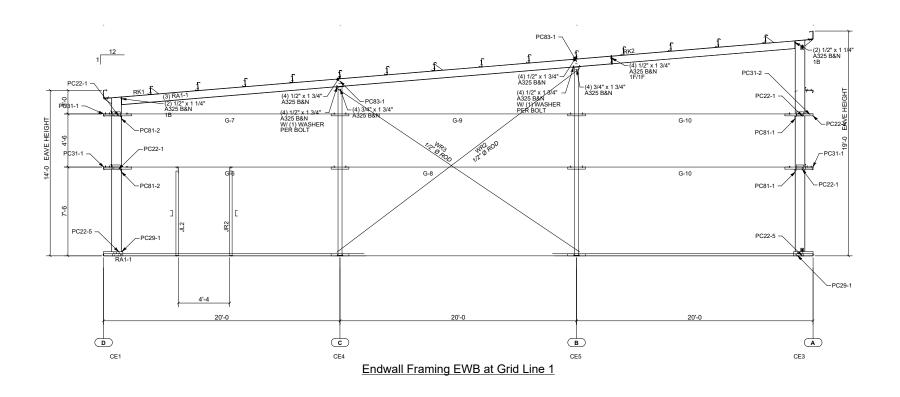


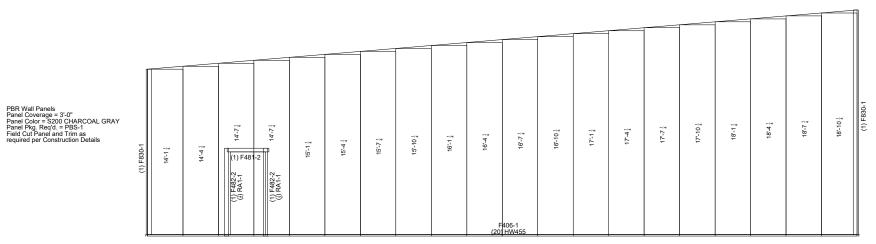
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)R	JECT TO CONSTRU INGS.	CHANGE ICTION DI	shown us signature	ing a digită must be vi	Jul 15	. Printed cc. 3rd Party ( , 2024	opies of th Certificate	is document are Authority on any	Tor the manufacturer for the materials described herein. Soid seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer of record for this project. S. HOSSEINZADEH, P.ENG ONTARIO P.ENG 100541411 S. HOSSEINZADEH, P.ENG 100541411 S. HOSSEINZADEH III S. HOSSEINZADEH IIII S. HOSSEINZADEH IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII

## Non-Standard PBR Wall Panel Fasteners

#58 member fasteners are to be used for panel to secondary attachment in lieu of #17A shown on the R Drawings

#4A lap fasteners are to be used for panel to panel and panel to trim attachment in lieu of #4A shown on the R Drawings





Endwall Sheeting EWB

SWC WB Bld'g A SWA (H.S.) Key Plan



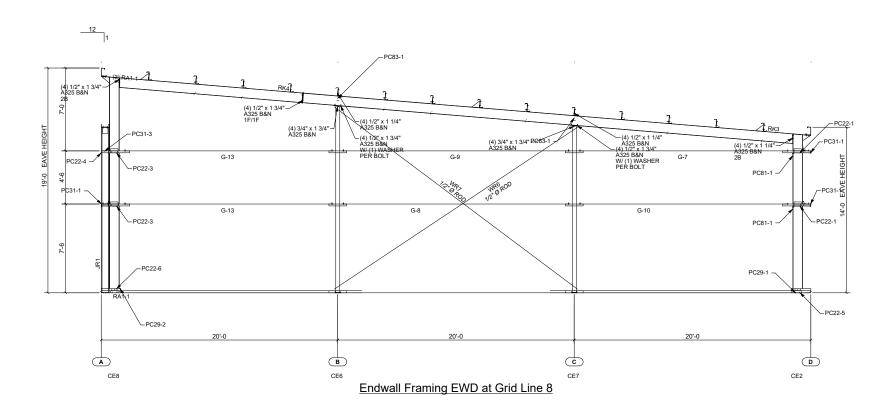
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	TORO STEEL BUILDINGS       1405 DENISON SIREET, MARKHAM, ONTARIO L3R 5V2       TEL: (877) 870-8676 FAX: (877) 474-4445       TEL: (877) 870-8676 FAX: (877) 474-4445       LEULIDINGS LTD DBA       1465 DENISON ST       JOY MIDLETON       Ictus:       [Aba Far Construction]       [Aot Far Construction]       [Aot Far Construction]
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	Project Engineer: JDM
	Job Number: <b>19-B-68083</b>
	Sheet Number: E7 of 14
	The engineer whose seal
	appears hereon is an employee
	for the manufacturer for the materials described herein. Said
	seal or certification is limited to the products designed and
	manufactured by manufacturer
	only. The undersigned engineer is not the overall engineer of
	record for this project.
	S. HOSSEINZADEH, P.ENG
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shown using a digital signature. Printed copies of this do signature must be verified by a 3rd Party Certificate Aut	
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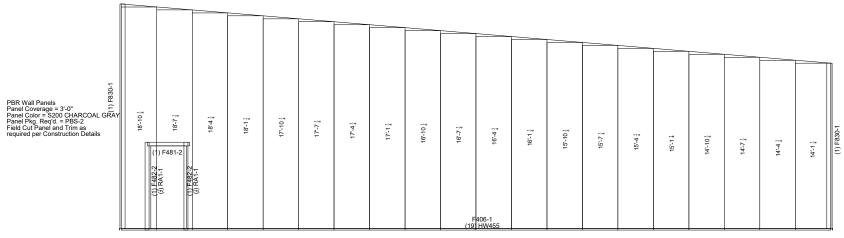
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## Non-Standard PBR Wall Panel Fasteners

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#4A lap fasteners are to be used for panel to panel and panel to trim attachment in lieu of #4A shown on the R Drawings





Endwall Sheeting EWD

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	Project Engineer: JDM
	Job Number: 19-B-68083
	Sheet Number: E8 of 14
	The engineer whose seal
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	S. HOSSEINZADEH, P.ENG
	ONTARIO P.ENG 100541411
This item has been electronically signed and sealed by	S. Hosseinzadeh, P.ENG on the date and/or time stamp
	ocument are not considered signed and sealed and the
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DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN. ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE PERMIT DRAWINGS.

