1.0 GENERAL NOTES

.1 DESIGN AND CONSTRUCTION IS TO CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE. REFER ALSO TO TYPICAL DETAILS, NOTES UNDER PLANS AND SCHEDULES ON THE STRUCTURAL DRAWINGS. ALL DIMENSIONS, OTHER THAN PURELY STRUCTURAL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE CHECKED AGAINST THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
.2 REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR DETAILS BY OTHER TRADES.
.3 UNLESS SPECIFICALLY NOTED ON THE DRAWINGS, NO PROVISION HAS BEEN MADE IN THE DESIGN FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE CONTRACTOR IS TO PROVIDE ALL BRACING AND SHORING REQUIRED TO SAFELY CARRY OUT THE WORK, INCLUDING TEMPORARY SUPPORT OF EXISTING OR ADJACENT STRUCTURES AFFECTED BY THE WORK.

1.1 SHOP DRAWINGS
.1 PROVIDE SHOP DRAWINGS FOR ALL STRUCTURAL COMPONENTS SHOWN ON THE STRUCTURAL DRAWINGS.

1.3 INSPECTION AND TESTING
.1 A SOILS CONSULTANT AND AN INDEPENDENT INSPECTION AND TESTING AGENCY ARE TO BE ENGAGED TO CARRY OUT SOME OR ALL OF THE FOLLOWING SERVICES AS SPECIFIED BY THE ARCHITECT/ENGINEER:
.1 SOIL – FOOTING EXCAVATIONS AT FOUNDING LEVELS.
.2 SLAB ON GRADE – CONFIRM THAT THE REQUIRED DEGREE OF COMPACTION HAS BEEN ATTAINED.
.3 CAST–IN–PLACE CONCRETE – ROUTINE INSPECTION OF MATERIALS, INCLUDING SLUMP, CYLINDER AND AIR ENTRAINMENT TESTS & REINFORCING TESTS WHEN REQUIRED OR DIRECTED IN ACCORDANCE WITH CSA CAN3–A23.2M., BUT NOT LESS THAN ONE TEST FOR THE PROJECT AND/OR ONE TEST/100m3 OF CONCRETE.
.4 STRUCTURAL STEEL – ROUTINE SHOP AND FIELD INSPECTION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF CSA CAN3–S16.1M.
.2 ALL INSPECTION AND TESTING SERVICES ARE TO BE PERFORMED BY COMPANIES CERTIFIED BY THE CANADIAN STANDARDS ASSOCIATION AND FOR WELDING, INSPECTORS ARE TO BE CERTIFIED BY THE CANADIAN WELDING BUREAU.

1.4 FOUNDATIONS
.1 REFER TO NOTES UNDER FOUNDATION PLANS. ALL EXTERIOR FOOTINGS OR OTHER FOOTINGS EXPOSED TO FREEZING IN THE FINISHED BUILDING SHALL BE FOUNDED AT A MINIMUM OF 1200 mm (4'–0") BELOW FINISHED GRADE, UNLESS OTHERWISE NOTED. FOOTINGS EXPOSED TO FROST ACTION DURING CONSTRUCTION SHALL BE PROTECTED BY A MINIMUM OF 1200 mm (4'–0") OF EARTH OR ITS EQUIVALENT TO PREVENT FREEZING.
.2 THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10, MAXIMUM STEP 600 mm (2'–0").
.3 IF ACTUAL JOB SITE OR SOIL CONDITIONS VARY FROM THOSE ASSUMED, THEN WRITTEN DIRECTIONS MUST BE OBTAINED FROM THE STRUCTURAL CONSULTANT BEFORE PROCEEDING WITH THE WORK.

1.5 BACKFILLING AND COMPACTION
.1 SLABS–ON–GRADE AND ALL STRUCTURAL ELEMENTS FRAMING INTO WALLS WHICH RETAIN EARTH MUST BE IN PLACE BEFORE BACKFILLING.
.2 AT FOUNDATION WALLS WITH GRADE ON BOTH SIDES, UNLESS ADEQUATELY SHORED BACKFILL AND COMPACT EACH SIDE OF WALL SIMULTANEOUSLY.
.3 UNDER SLAB–ON–GRADE REMOVE SOFT SPOTS, ORGANIC AND FOREIGN MATTER IN THE SUB–GRADE.
.4 BACKFILL UNDER SLAB–ON–GRADE ONLY WITH APPROVED MATERIAL, CARRIED OUT IN MAXIMUM OF 200 mm (8") LIFTS OF LOOSE FILL, EACH COMPACTED TO A MINIMUM OF 95 % STANDARD PROCTOR MAXIMUM DRY DENSITY.
.5 UNLESS OTHERWISE NOTED, PROVIDE IMMEDIATELY UNDER SLABS–ON–GRADE A MINIMUM OF 200 mm (8") OF COMPACTED GRANULAR 'A' MATERIAL, COMPACTED TO A MINIMUM 98% STANDARD PROCTOR DRY DENSITY.

1.6 CONCRETE WALLS
.1 AT OPENINGS IN CONCRETE WALLS ADD 2 – 15 BARS IN HEADS, JAMBS AND SILLS, UNLESS NOTED OTHERWISE. EXTEND 900 mm (3'–0") BEYOND OPENING.

1.7 MASONRY WALLS
.1 DO NOT CUT ADDITIONAL HOLES THROUGH MASONRY BEARING WALLS OTHER THAN THE HOLES SHOWN ON THE STRUCTURAL DRAWINGS, UNLESS APPROVED BY THE ENGINEER.

1.8 BEARINGS
.1 PROVIDE A MINIMUM BEARING OF 200 mm (8") FOR CONCRETE OR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE.
.2 PROVIDE A MINIMUM SLAB BEARING EQUAL TO THE THICKNESS OF THE SLAB, WITH A MAXIMUM OF 200 mm (8"), UNLESS OTHERWISE NOTED.
.3 MASONRY WALLS: BEARING TO BE ON BRICKS OR 100% SOLID BLOCK UNITS LAID IN CEMENT MORTAR FOR A LENGTH PARALLEL TO THE WALL EQUAL TO TWICE THE LENGTH OF BEARING AND FOR A HEIGHT EQUAL TO THE LENGTH OF BEARING.

1.9 DESIGN
.1 CONCRETE MEMBERS ARE DESIGNED IN ACCORDANCE WITH CSA STANDARD A23.3.
STRUCTURAL STEEL IS DESIGNED IN ACCORDANCE WITH STANDARD CSA S16.1.
TIMBER IS DEIGNED IN ACCORDANCE WITH CSA STANDARD 086.1.
.2 UNIT FLOOR AND ROOF LOADS GIVEN ON THE DRAWINGS ARE SPECIFIED LOADS. MEMBER FORCES GIVEN ON THE DRAWINGS ARE FACTORED LOADS.

1.10 MATERIALS

.1 SEE NOTES UNDER PLANS FOR STRENGTH OF CONCRETE.
PROVIDE EXPOSURE CLASSIFICATION IN ACCORDANCE WITH CSA STANDARD A23.1, CLAUSE 15 AND TABLE 8, AS FOLLOWS:

.1 GENERAL CONCRETE:	CLASS N	
.2 EXTERIOR CONCRETE NOT EXPOSED TO DE–ICING CHEMICALS	CLASS F–2	4–7% AIR ENT.
.3 PARKING STRUCTURES	CLASS C–1	5–8% AIR ENT.
.4 EXTERIOR SLAB ON GRADE, SIDEWALKS, RAMPS, CURBS, GUTTERS	CLASS C–2	5–8% AIR ENT.
.5 FOUNDATION WALLS AND RETAINING WALLS EXPOSED TO DE–ICING CHEMICALS	CLASS C–1	5–8% AIR ENT.
.6 INTERIOR SLAB ON GRADE EXPOSED TO DE–ICING CHEMICALS	CLASS C–4	4–7% AIR ENT.

.2 STRUCTURAL STEEL TO CONFORM TO CSA G40.21 GRADE 300W, EXCEPT HOLLOW STRUCTURAL SECTIONS TO GRADE 350W CLASS H, UNLESS NOTED OTHERWISE.
.3 REINFORCING STEEL TO CONFORM TO CSA G40.12, WITH 400 MPa MINIMUM YIELD STRENGTH.
.4 TIMBER IS TO CONFORM WITH APPLICABLE GRADING STANDARDS FOR SPF No.2 OR BETTER. LAMINATED VENEER LUMBER SHALL CONFORM TO INDUSTRY STANDARDS OF THE MANUFACTURER.

6. CAST–IN PLACE CONCRETE NOTES

6.1 GENERAL
PROVIDE ALL LABOUR, MATERIALS, TOOLS AND EQUIPMENT REQUIRED TO CARRY OUT THE WORK.

6.2 PRODUCTS
.1 PORTLAND CEMENT, WATER AND AGGREGATES SHALL CONFORM TO CSA CAN3–A23.3.1M.
.2 COARSE AGGREGATE: NORMAL EIGHT, MAX. SIZE 20mm.
.3 PROVIDE AN APPROVED WATER REDUCING ADDITIVE IN ALL CONCRETE.
.4 PROVIDE AN APPROVED AIR ENTRAINING ADDITIVE IN ALL CONCRETE WHICH WILL BE EXPOSED TO A FREEZE/THAW CYCLE AND/OR THE ACTION OF DE–ICING CHEMICALS.
.5 ADMIXTURES SHALL CONFORM TO CSA CAN3–A266M SERIES.
.6 FORMWORK SHALL CONFORM TO CSA CAN3–A23.1M AND FALSEWORK SHALL CONFORM TO CSA S269.1.
.7 PROVIDE STANDARD ADJUSTABLE MASONRY ANCHOR SLOTS FOR ALL MASONRY FACING ABUTTING CONCRETE FACES.
.8 REINFORCING STEEL, UNLESS SPECIFICALLY NOTED, SHALL BE DEFORMED BARS CONFORMING TO CSA G30.12M, GRADE 400 (58000psi).
.9 DRY–PACK GROUT TO BE 1 PART PORTLAND CEMENT TO 1–1/2 PARTS SAND TO 2 PARTS OF 8mm PEA GRAVEL WITH ONLY SUFFICIENT WATER TO DAMPEN THE MIXTURE. COMPRESSIVE STRENGTH 50 MPa AT 28 DAYS.
.10 NON–SHRINK GROUT TO BE AN APPROVED PRE–MIXED PROPRIETARY PRODUCT.
.11 CURING AND SEALING COMPOUNDS WHERE APPROVED FOR USE TO CONFORM TO ASTM STANDARD C309. GENERALLY, ALL CONCRETE SURFACES ARE TO BE SEALED UNLESS NOTED OTHERWISE. COMPOUNDS ARE TO BE COMPATIBLE WITH APPLIED FINISHES.
.12 DRILLED BOLTS: KWIK BOLTS BY HILTI CANADA LIMITED, SIZE AND LOCATION AS SPECIFIED ON THE DRAWINGS.

6.3 EXECUTION
MINIMUM COMPRESSIVE STRENGTH FOR CONCRETE AT 28 DAYS SHALL BE AS NOTED ON THE DRAWINGS. (20 MPa MINIMUM)
SLUMP AT THE POINT OF DISCHARGE SHALL BE CONSISTENT AT 90mm (3.5").
CONSTRUCTION JOINTS FOR THE WALLS, SLABS, AND BEAMS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE CONSULTANT BEFORE CONSTRUCTION.
FLOOR FINISHES SHALL CONFORM TO CSA STANDARD CAN3–A23.1M (CLASS A FINISH UNLESS NOTED).

6.4 DESIGN
CONCRETE MEMBERS ARE DESIGNED IN ACCORDANCE WITH CSA STANDARD A23.3.

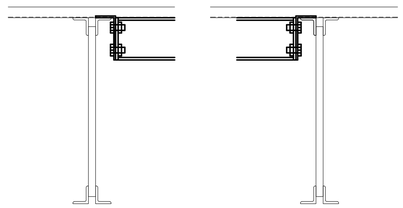
7. LOAD BEARING MASONRY NOTES

7.1 GENERAL
MASONRY CONSTRUCTION TO CONFORM TO CSA STANDARDS CAN3–S304–M, & CAN3–A371–M.

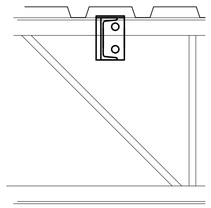
7.2 PRODUCTS
.1 CONCRETE BLOCKS & BRICKS: – TO CONFORM TO ONE OR MORE OF CSA A165.M, .2M, .3M OR .4M. BLOCKS TO BE MODULAR UNITS AS SHOWN ON THE ARCHITECTURAL DRAWINGS &/OR SPECIFICATION, AND UNLESS OTHERWISE NOTED SHALL BE :
.1 FOR BELOW GRADE AND EXTERIOR EXPOSED WALLS USE NORMAL WEIGHT LOAD BEARING UNITS:
STANDARD HOLLOW .... TYPE H/15/A/M
75% SOLID ..... TYPE S/15/A/M
100% SOLID ..... TYPE Sc/15/A/M
.2 FOR INTERIOR ABOVE GRADE WALLS USE LIGHT WEIGHT LOAD BEARING BLOCKS:
STANDARD HOLLOW .... TYPE H/15/C/M
75% SOLID ..... TYPE S/15/C/M
100% SOLID ..... TYPE Sc/15/C/M
.2 CLAY BRICKS: TO CONFORM TO ONE OR MORE OF CSA A82.1M, .3M, .4M, .5M, OR .8M. SEE ARCHITECTURAL DRAWINGS AND OR SPECIFICATIONS FOR TYPES & STYLES OF BRICKS REQUIRED. UNLESS OTHER WISE NOTED, THE MINIMUM COMPRESSIVE STRENGTH GROSS AREA SHALL BE 20MPa.
.3 MORTAR: TO CONFORM TO CSA A179M.
FOR LAYING ALL CONCRETE BLOCKS .... USE TYPE S MORTAR UNLESS NOTED.
FOR LAYING ALL CLAY BRICKS ..... USE TYPE N MORTAR UNLESS NOTED.
.4 MASONRY GROUT: TO CONFORM TO CSA A179–M. THE SLUMP SHALL BE + 200mm AND THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 12.5 MPa.
.5 MASONRY CONNECTORS: (ANCHORS, FASTENERS & TIES) SHALL CONFORM TO CSA CAN 3–A370–M, AND BE INSTALLED TO COMPLY WITH CSA CAN–3–A371.M. FOR CAVITY WALL CONSTRUCTION, USE ECONO–CAVITY LOK BL–21 9 GAUGE HOT DIP GALVANIZED BOX TIES BY BLOK–LOK LIMITED. SPACE TIES AT 400mm c/c HORIZONTAL AND VERTICAL. CONNECTORS SHALL BE CAPABLE OF TRANSMITTING 2.2 kPa PRESSURE BETWEEN THE WYTHES.
.6 COMPOSITE AND CAVITY WALLS: WHERE COURSING OF WYTHES DO NOT ALIGN, OR WHERE IT IS DESIRABLE AND PERMITTED TO BUILD ONE WYTHE AT A TIME, REINFORCING SHALL BE AN APPROVED ADJUSTABLE ROD TYPE WITH A BOX OR AN EYE SECTION WHICH EXTENDS INTO THE COLLAR JOINT OR CAVITY AND RESTRAINS THE TRANSVERSE MOVEMENT OF THE TWO WYTHES. RODS SHALL HAVE A MIN. AREA OF 17.8mm2 AND BE SPACED AT NOT MORE THAN 900mm (3'–0") HORIZONTAL AND 20mm (8") VERTICAL, WITH NOT LESS THAN a 50mm (2") PORTION BENT AT RIGHT ANGLES AT EACH END. FOR CAVITY WALLS, WITH RIGID INSULATION, EXTENSION SHALL BE DESIGNED TO HOLD THE INSULATION IN PLACE BY USE OF PLASTIC WEDGES OR AN APPROVED EQUAL. GALVANIZED HOOK STYLE BOX TIES OR PIN TIES SHALL EXTEND INTO THE FACE WYTHE TO COMPLETE THE ASSEMBLY.
.7 HORIZONTAL JOINT REINFORCEMENT FOR ALL MASONRY WALLS:
.1 CONFORM TO CSA CAN3 A370–M, & CAN3 A371–M.
.2 REINFORCEMENT SHALL BE AN APPROVED CONTINUOUS LADDER TYPE, PREFABRICATED WITH 3.66mm DIA. (9 ga.) LONGITUDINAL AND CROSS WIRES.
.3 REINFORCING SHALL BE HOT DIPPED GALVANIZED TO ASTM STANDARD A153–82,458 gm/sq.m.(15oz./sq.ft.)

7.3 EXECUTION
.1 MASONRY BEARINGS SHALL BE ON SOLID BLOCKS (OR GROUTED SOLID) OR BRICKS LAID IN MORTAR. ALL JOINTS ARE TO BE FULLY FILLED WITH TYPE S MORTAR.
PROVIDE A MINIMUM BEARING OF 200 mm (8") FOR CONCRETE OR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE.
PROVIDE A MINIMUM SLAB BEARING EQUAL TO THE THICKNESS OF THE SLAB, WITH A MAXIMUM OF 200 mm (8"), UNLESS OTHERWISE NOTED.
BEARING TO BE ON BRICKS OR 100% SOLID BLOCK UNITS LAID IN CEMENT MORTAR FOR A LENGTH PARALLEL TO THE WALL EQUAL TO TWICE THE LENGTH OF BEARING AND FOR A HEIGHT EQUAL TO THE LENGTH OF BEARING.
.2 COMPOSITE WALLS: SHALL HAVE VERTICAL COLLAR JOINTS BETWEEN THE WYTHES COMPLETELY FILLED WITH MORTAR AND OR GROUT.

.3 GROUTING: BY FILLING VOIDS OF HOLLOW UNITS AND REINFORCED HOLLOW UNITS SHALL CONFORM TO CSA A–179–M (MORTAR IS NOT ACCEPTABLE).
THE SLUMP SHALL BE+– 200mm AND THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 25MPa. USE LaFARGE AGILIA BLOCK FILLER.



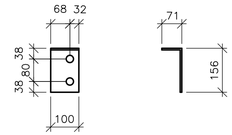
FRONT VIEW



ELEVATION

PLATE HANGER

1:10



SEE PLANS FOR PLATE ORIENTATION

1. 12mm WELDED STEEL PLATES c/w TWO 21mm dia. HOLES FOR TWO M20 A325 CONNECTION BOLTS PLUS NUTS & WASHERS
Fy = 350MPa

2. C150x12 CHANNEL REINFORCING
Fy = 350MPa

3. ALL WELDED CONNECTIONS USE 1/4" FILLET WELDS U/N.
E480XX ELECTRODES.

8. STRUCTURAL STEEL NOTES

8.1 GENERAL
STRUCTURAL STEEL DESIGN SHALL CONFORM TO CSA STANDARD CAN3–S16.1M.
CONNECTIONS SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER EXPERIENCED IN THIS TYPE OF WORK, HIS SEAL SHALL APPEAR ON SHOP DRAWINGS.

8.2 WELDING SHALL CONFORM TO CSA STANDARD W59 AND BE PERFORMED BY A FABRICATOR CERTIFIED TO CSA W47.1.

8.3 BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 50% OF THE BEAM SHEAR CAPACITY UNLESS OTHERWISE NOTED, AND IN NO CASE BE LESS THAN THE LOADS SHOWN ON OR IMPLIED BY THE DRAWINGS.

8.4 PRODUCTS
.1 ALL STRUCTURAL; STEEL MEMBERS SHALL CONFORM TO CSA CAN3– G40.21–M. ROLLED SECTIONS, PLATES, SAG RODS, STRAP ANCHORS AND BARS SHALL BE TYPE 350 W AND HOLLOW STRUCTURAL SECTIONS SHALL BE TYPE 350W, CLASS C FOR SQUARE HSS AND CLASS C FOR ROUND HSS.
.2 BOLTS, NUTS AND WASHERS FOR CONNECTIONS TO CONFORM TO ASTM A325 UNLESS NOTED.
.3 ANCHOR BOLTS, NUTS AND WASHERS FOR BASE PLATES, BEARING PLATES & WELD PLATES TO CONFORM TO ASTM A307 UNLESS NOTED.
.4 WELDING MATERIALS TO CONFORM TO CSA W48–M (SERIES).
.5 PRIMER PAINT TO CONFORM TO CGSB 1–GP–40M OR CISC/CPMA 2–75.
.6 GRATING: WELDED STEEL, OF SIZE AND TYPE SPECIFIED ON THE DRAWINGS SUFFICIENT TO SUPPORT LOADINGS GIVEN ON THE DRAWINGS, WITH MAXIMUM DEFLECTION OF 1/180 OF SPAN, AND NOT TO EXCEED 1/4" MAXIMUM.
.7 DRILLED BOLTS SHALL BE HILTI KWIK BOLTS BY HILTI CANADA OR AN APPROVED EQUIVALENT, OF THE SIZE AND SPACING INDICATED ON THE DRAWINGS.

8.5 EXECUTION
.1 FABRICATION, HANDLING & ERECTION TO CONFORM TO CSA CAN 3–S16.1–M.
.2 BEAMS TO BE WELDED TO BEARING PLATES.
.3 PROVIDE ADJUSTABLE ANCHORS TO ALL STEEL TO BE BUILT INTO, ABUTTED OR FACED WITH MASONRY.
.4 CLEAN, PREPARE SURFACES AND SHOP PRIME STRUCTURAL STEEL WITH ONE COAT OF SPECIFIED PRIMER PAINT IN ACCORDANCE WITH CSA CAN 3–S16.1–M. FIELD TOUCH UP BOLTS WELDS BURNED OR SCRAPED SURFACES AFTER ERECTION.
** PROVIDE ALL NEW STEEL CLEANED TO SSPC–SP10 TO NEAR WHITE SURFACE, AND PRIME PAINTED WITH ONE COAT OF ZINC–RICH PAINT TO CGSB SPECIFICATION 1–GP–181M @ 3 MILS D.F.T., AND TWO COATS OF EPOXY PAINT TO CGSB SPECIFICATION 1–GP–193 @ 4 MILS D.F.T. FINISH WITH ONE COAT OF POLYURETHANE PAINT, TYPE AND COLOUR AS SPECIFIED BY THE ARCHITECT.
.5 NO HOLES OTHER THAN THOSE SHOWN ON REVIEWED SHOP DRAWINGS SHALL BE MADE IN ANY STEEL MEMBER WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL CONSULTANT.
.6 PROVIDE DIRECT CONNECTIONS OF SPANDREL BEAMS TO COLUMNS TO PREVENT TWISTING.

8.6 DESIGN
.1 STRUCTURAL STEEL IS DESIGNED IN ACCORDANCE WITH CSA STANDARD S16.1.
UNIT FLOOR AND ROOF LOADS GIVEN ON THE DRAWINGS ARE SPECIFIED LOADS.
MEMBER FORCES GIVEN ON THE DRAWINGS ARE FACTORED LOADS.

10. SAWN TIMBER NOTES

10.1 REFERENCE STANDARDS CAN/CSA–086.1
CSA 0121 M
CSA 0141 M
ONTARIO BUILDING CODE

10.2 DESIGN CONNECTIONS, BRACKETS, AND HARDWARE TO RESIST THE REACTIONS PRODUCED BY THE FRAMING OR LOAD CONDITIONS.

10.3 THE CONTRACTOR SHALL PROVIDE CCMC REPORTS FOR ALL PRE–ENGINEERED PRODUCTS TO BE USED. BOTH FOR THOSE PRODUCTS SPECIFIED, OR PROPOSED FOR USE.

10.3 MATERIALS
.1 SAWN LUMBER: PROVIDE GRADE NO. 2 SPF LUMBER AS SHOWN ON DRAWINGS CONFORMING TO CSA 0141M AND NLGA GRADING RULES FOR CANADIAN LUMBER. USE ONLY KILN DRIED LUMBER.
.2 LVL – LONGITUDINAL STRAND LUMBER BY LOUISIANA PACIFIC CORP. OR AN APPROVED EQUIVALENT HAVING AN ALLOWABLE BENDING STRESS OF Fb = 37.6 MPa (5452psi) AND E=2.0e6psi.
.3 PSL LUMBER: PARALLEL STRAND BY TRUS JOIST MacMILLAN OR APPROVED EQUIVALENT HAVING A MINIMUM ALLOWABLE BENDING STRESS f'b= 39.5 MPa (5730 psi) AND E=2.0e6psi.
.4 LSL LUMBER: LONGITUDINAL STRAND BY TRUS JOIST MacMILLAN OR APPROVED EQUIVALENT HAVING A MINIMUM ALLOWABLE BENDING STRESS f'b= 30.6 MPa (4435 psi) AND E=1.8e6psi.
.3 PLYWOOD: PANELS FOR ROOF, EXTERIOR WALL AND SUB FLOORING TO MEET SPECIFIED REQUIREMENTS OF CSA 0121M DOUGLAS FIR PLYWOOD (DFP) AND/OR CSA 0151M CANADIAN SOFT PLYWOOD (CSP). PANELS SHALL BE 4'–0" x 8'–0" IN SIZE, AND OF THICKNESS SHOWN ON DRAWINGS.
.4 FASTENINGS AND HARDWARE: NAILS, SPIKES AND STAPLES: TO CONFORM TO CSA STANDARD B111
BOLTS & LAG SCREWS: TO CONFORM TO ASTM STANDARD A307, CARBON STEEL, EXTERNALLY THREADED.
TRUSS PLATES: MANUFACTURED FROM GALVANIZED SHEET STEEL CONFORMING TO GRADE B OF ASTM STANDARD A446, SHEET STEEL, ZINC COATED BY THE HOT DIP PROCESS, STRUCTURAL QUALITY.
JOIST HANGERS: MINIMUM .040" THICK SHEET STEEL GALVANIZE 696 COATING DESIGNATION, 1500 lb. BEARING STRENGTH.
.5 ADHESIVES: SUB FLOORING ADHESIVE TO CONFORM TO CGSB 71–GP–26M

10.5 ERECTION
MAKE ADEQUATE PROVISION FOR HORIZONTAL AND VERTICAL ERECTION LOADS AND FOR SUFFICIENT TEMPORARY BRACING TO KEEP THE STRUCTURAL FRAME PLUMB AND IN TRUE ALIGNMENT UNTIL THE COMPLETION OF ERECTION AND THE INSTALLATION OF MASONRY, CONCRETE WORK, AND FLOOR AND ROOF DECKS WHICH WILL PROVIDE THE NECESSARY PERMANENT BRACING.
PROVIDE TEMPORARY WOOD MEMBERS AS MAY BE REQUIRED FOR ERECTION PURPOSES AND REMOVE THEM WHEN NO LONGER REQUIRED.

10.6 WOOD FRAME CONSTRUCTION
COMPLY WITH THE REQUIREMENTS OF THE ONTARIO BUILDING CODE, SECTION 9.23, EXCEPT WHERE OTHERWISE SPECIFIED OR SHOWN ON THE DRAWINGS. SEE ALSO PREFABRICATED TIMBER ROOF TRUSS NOTES.
ANCHOR ROOF DECKS TO SUPPORTING FRAME TO RESIST UPLIFTS SPECIFIED IN THE NATIONAL BUILDING CODE OF CANADA AND SUPPLEMENT.
GLUE AND SCREW DECKING MATERIAL TO FLOOR JOISTS.

STRUCTURAL DRAWING LIST

PLOT DATE: May 4, 2024

#	DATE	REVISION
2	24/07/04	ISSUED FOR TENDER
1	24/06/21	ISSUED FOR BUILDING PERMIT
#	DATE	ISSUED

ISSUED FOR CONSTRUCTION

SIGNATURE _____

DATE _____

client:

MISSISSAUGA

client project no.

project:

consultant:

Hamann Engineering
Structural Consultants Ltd.
44 Caronridge Crescent
Toronto, Ontario, M1W 1L2
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north

seal

REGISTERED PROFESSIONAL ENGINEER
24/06/21
S. D. HAMANN
PROVINCE OF ONTARIO

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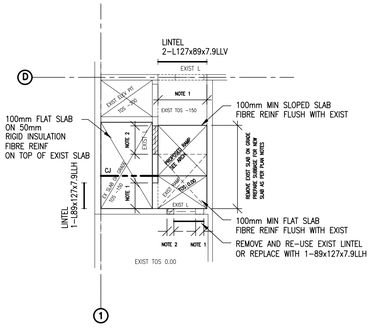
project architect

project designer

sheet title
GENERAL NOTES AND DETAILS

scale	drawn by	checked by	drawing no.
	SH		S1.1
project no. 24004	plot date: 2024-05-15		



PART EXISTING GROUND FLOOR PLAN

1:100

1. SLAB ON EARTH SHALL BE PLACED ON SOIL CAPABLE OF SUSTAINING 250psf EXCEPT AS CROSSED AND NOTED, AND OF SUFFICIENT COMPACTION THAT NO DIFFERENTIAL SETTLEMENT SHALL TAKE PLACE BETWEEN THE SLAB ON EARTH AND THE THE BUILDING FOOTINGS. INFORMATION RELATING TO THE VALUE OF THE SOIL UNDER THE FOOTINGS IS BASED ON THE INFORMATION AVAILABLE AT THE TIME DRAWINGS ARE ISSUED. THE CONTRACTOR SHALL PLACE FLOORS ON SOIL CAPABLE OF SUPPORTING THE PRESSURES GIVEN ON THE DRAWINGS. ANY ADJUSTMENTS CONSIDERED NECESSARY SHALL BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

2. FINISHED GROUND FLOOR IS AT ELEVATION AS CROSSED AND NOTED. TOP OF CONCRETE SLAB IS 0mm BELOW FINISHED GROUND FLOOR.

3. CONCRETE STRENGTH SHALL BE 25 MPa, UNLESS NOTED. EXPOSED CONCRETE SHALL BE AIR ENTRAINED.

4. RIGID INSULATION WHERE INDICATED SHALL BE DOW SM OR EQUIVALENT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 40 psi.

5. SLAB-ON-GRADE:

   4" MIN. SLAB-ON-GRADE. REINFORCE SLAB WITH 3/4" FIBRILLATED, POLYPROPYLENE FIBRES FROM FIBREMESH CANADA LIMITED AT 0.9 kg/m3. (0.1 % YIELD BY VOLUME) ADDED AT THE READY-MIX CONCRETE PLANT MEETING ASTM STANDARD C1116-89, SECTION 4.1.3.
   GRANULAR UNDERBED: SUB-FILL TO OPSS 1010 FOR TYPE B AGGREGATE, BUT WITH 100 PASSING THE 22.4 mm SIEVE. OVERLAY THIS WITH 200mm OF COMPACTED GRANULAR 'A' MATERIAL COMPACTED TO A MINIMUM 98% STANDARD PROCTOR DRY DENSITY.

   PROVIDE CONTROL JOINTS AT LOCATIONS WHERE INDICATED ON PLAN. SAWCUT CONTROL JOINTS USING 3/16" BLADE FOR 1/4 DEPTH OF SLAB. SAWCUT JOINTS AS SOON AS POSSIBLE AFTER PLACING CONCRETE BUT NOT LATER THAN 18 HOURS. ALTERNATIVELY, PREFORM CONTROL JOINTS WITH A PROPRIETARY PRODUCT SUCH AS VINYLEX Zip Strips OR APPROVED EQUAL. WHERE NOT INDICATED, PROVIDE SAW CUT JOINTS AT SPACING 15'-0" EACH WAY MAX.

   FILL CONTROL JOINTS AS FOLLOWS:
   FLOORS TO BE COVERED WITH A FINISH MATERIAL USE LATEX MODIFIED MORTAR.
   INTERIOR FLOORS TO BE LEFT EXPOSED USE EPOXY URETHANE FILLER
   EXTERIOR FLOORS TO BE LEFT EXPOSED USE ELASTOMERIC SEALANT FILLER

5.1 HAMANN ENGINEERING IS NOT RESPONSIBLE FOR CEMENTITIOUS OR NON-CEMENTITIOUS TOPPING MATERIAL, PLACEMENT, CONTROL JOINTS OR CURING. IT IS THE RESPONSIBILITY OF THE TOPPING CONTRACTOR TO REVIEW THE PROPOSED OR AS BUILT SLAB ON GRADE FOR COMPATIBILITY WITH THEIR PRODUCTS AND ADVISE IN WRITING IF MODIFICATIONS ARE REQUIRED TO ENSURE THE INTEGRITY OF THE TOPPING INSTALLATION.

6. CONTRACTOR SHALL OBTAIN INFORMATION RELATING TO EXISTING AND PROPOSED MECHANICAL SERVICES ADJACENT FOOTING LOCATIONS. ADJUST FOOTING FOUNDING LEVELS ACCORDINGLY. DIRECT INTERFERENCES SHALL BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.

7. DEFINITIONS (ALL DEFINITIONS BELOW MAY NOT BE USED ON THIS DRAWING)
   CJ – SAW CUT CONTROL JOINT. SEE SLAB ON GRADE NOTE 9.
   EX – EXISTING
   EXIST – EXISTING
   PW – PARTITION WALL; NON-LOAD BEARING 2x6@16 STUD WALL U/N.
   TOS – TOP OF SLAB

8. NEW/EXISTING CONSTRUCTION
   1. AT OPENINGS THRU EXISTING MASONRY WALLS: DRILL AND GROUT 10@16 DOWELS INTO FACE OF CUT WALL, PRIOR TO FORMING AND POURING 4" MIN. x THICKNESS OF WALL VERTICAL CONCRETE CAP. IN ADDITION TO CONC. BEARING PAD FOR SUPPORTED BEAM AS PER PLAN.
   2. AT NEW/EXISTING SLAB ON GRADE JOINTS, PROVIDE 10@16x18" LG DOWELS DRILLED & GROUTED 6" INTO EXISTING SLAB EDGE.

ALL STRUCTURAL INFORMATION RELATING TO EXISTING MEMBERS, PROPERTIES AND LOCATIONS ARE ASSUMED ONLY. EFFORTS HAVE BEEN MADE TO REPRODUCE EXISTING FRAMING BASED ON EXISTING DRAWINGS (IF AVAILABLE), SITE VISITS AND BY VIEWING FROM LOCAL FINISH PENETRATIONS. HAMANN ENGINEERING IS NOT RESPONSIBLE FOR DISCREPANCIES OF ASSUMED EXISTING CONDITIONS ON DRAWINGS AND ACTUAL CONDITIONS DISCOVERED ON SITE.

THE CONTRACTOR SHALL NOTE AND REPORT TO THE ENGINEER ANY DISCREPANCIES BETWEEN EXISTING CONDITIONS AND CONDITIONS INDICATED ON THESE DRAWINGS. REMEDIAL WORK WILL BE ISSUED AS REQUIRED BY THE ENGINEER.

NOTE 1

DEMOLISH AND REMOVE EXISTING MASONRY WALL ABOVE TO 12" BELOW FLOOR LEVEL. MAKE GOOD FLOOR/SLAB.

NOTE 2

FILL EXISTING WALL OPENING ABOVE. LEAVE EXIST LINTEL IN PLACE U/N. USE SAME TYPE OF CONSTRUCTION AS EXISTING, UNLESS NOTED.

PLOT DATE: JULY 4, 2024

#	DATE	REVISION
2	24/07/04	ISSUED FOR BUILDING TENDER
1	24/06/21	ISSUED FOR BUILDING PERMIT

#	DATE	ISSUED

THE ARCHITECT IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, STRUCTURAL, MECHANICAL, ELECTRICAL, ETC. ENGINEERING INFORMATION SHOWN ON THE DRAWING. REFER TO THE APPROPRIATE ENGINEERING DRAWINGS BEFORE PROCEEDING WITH WORK.

CONTRACTOR SHALL CHECK ALL DIMENSIONS ON THE WORK AND REPORT ANY DISCREPANCY TO THE ARCHITECT BEFORE PROCEEDING. CONSTRUCTION MUST CONFORM TO ALL APPLICABLE CODES AND REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.

THE DRAWING IS NOT TO BE SCALED.

THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION UNLESS SIGNED BY THE ARCHITECT.

ISSUED FOR CONSTRUCTION

SIGNATURE _____

DATE _____

client:

MISSISSAUGA

client project no.

project:

consultant:

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REGISTERED PROFESSIONAL ENGINEER
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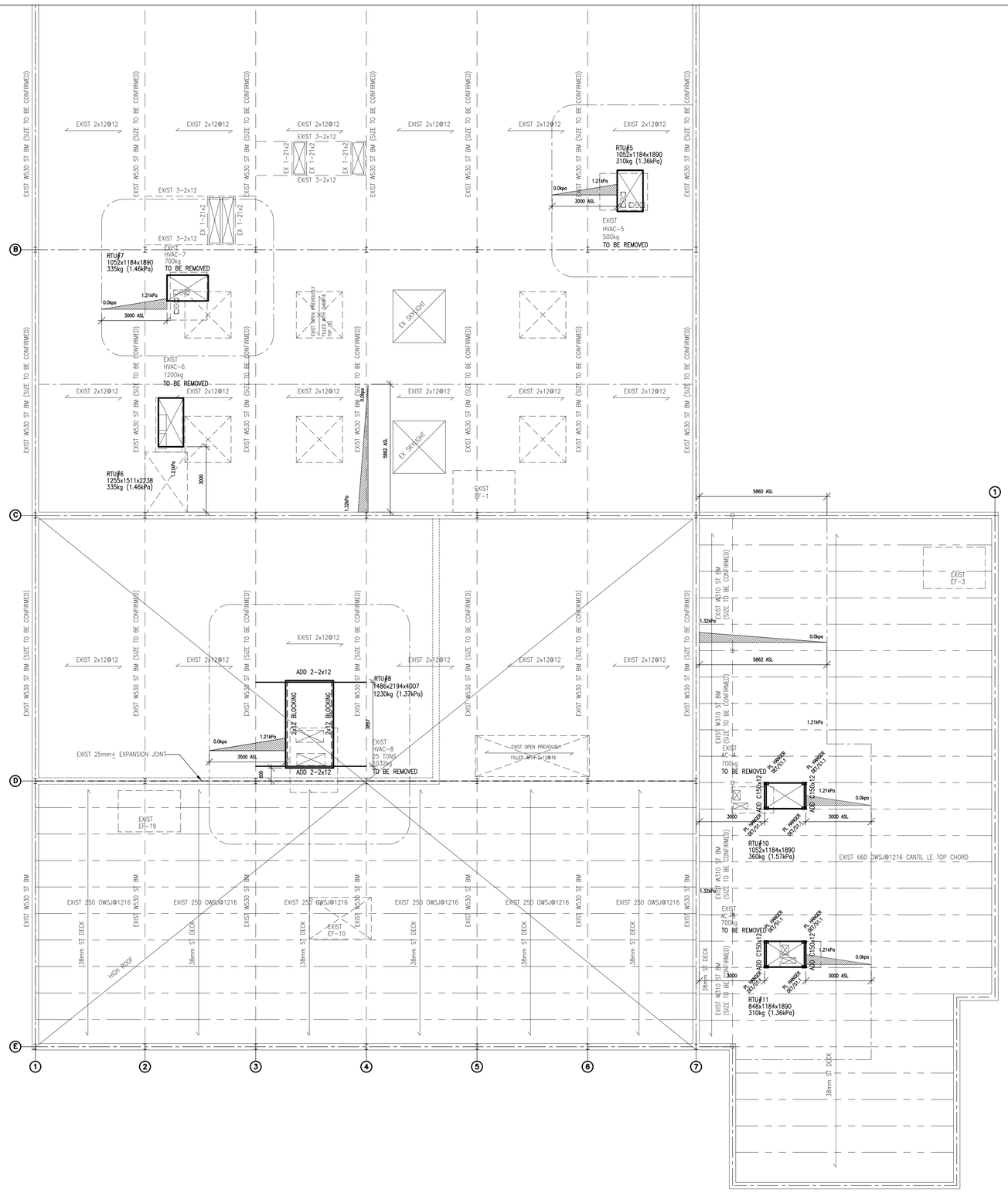
project architect

project designer

sheet title

GROUND FLOOR ALTERATIONS

scale	drawn by	checked by	drawing no.
	SH		S2.1
project no. 24004	plot date: 2024-05-15		



PART EXISTING ROOF FRAMING PLAN

1:100

1. TOP OF DECK 0mm BELOW ROUGH ROOF EXCEPT AS CROSSED AND NOTED.
2. TOP OF STEEL AS FOLLOWS:
   STEEL BEAMS SUPPORTING TIMBER ROOF -57 BELOW ROUGH ROOF UNLESS NOTED.
   STEEL BEAMS SUPPORTING STEEL ROOF -138 BELOW ROUGH ROOF UNLESS NOTED.
3. PRESENT LIVE LOAD: MINIMUM SNOW LOAD 1.28kPa MISSISSAUGA PLUS ASL. WHERE NOTED, ACCUMULATED SNOW LOAD DUE TO ROOF TOP MECH UNITS AND ROOF ELEVATION CHANGES AS NOTED ON PLAN
   PRESENT DEAD LOAD:

ROOFING	0.50kPa
DECK	0.10kPa
FRAMING	0.25kPa
MECHANICAL& CEILING	0.11kPa

4. ASL – ACCUMULATED SNOW LOAD.
5. L90x90x8 AROUND MECHANICAL UNITS WHERE NOTED.
6. TYPICAL EXISTING JOIST SHOE DEPTH FOR OPEN WEB STEEL JOISTS (OWSJ) IS 100mm. TYPICAL JOIST SHOE DEPTH FOR LONG SPAN STEEL JOISTS (LSSJ) 127mm.
7. FLAT ROOF DECK SHALL BE 5/8" T&G DOUGLAS FIR PLYWOOD OR 19/32" DRYGUARD ENHANCED OSB BY GEORGIA PACIFIC FOR SUPPORT FRAMING UP TO 24".
4. ⟵ INDICATES SPAN DIRECTION OF LOAD BEARING JOISTS.
7. STRUCTURAL DRAWINGS ARE FOR MEMBER SIZES ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS, DETAILS AND SPECIFICATIONS. ALL FRAMING TO BE IN
8. ALL TIMBER MEMBERS ARE TO BE NO. 2 SPF UNLESS NOTED. MEMBER SIZES ARE MINIMUM FOR STRUCTURAL REQUIREMENTS. ALL EXTERIOR TIMBER SHALL BE PRESSURE TREATED AND ALL HARDWARE GALVANIZED. LWMF HANGERS IN CONTACT WITH PT TIMBER APPROPRIATE TYPE WITH EXTRA HEAVY GALV. COATING, AS PER MANUFACTURER'S RECOMMENDATIONS. EXPOSED PSL AND LVL MATERIAL SHALL BE WEATHER PROTECTED,

#	DATE	REVISION
2	24/07/04	ISSUED FOR BUILDING TENDER
1	24/06/21	ISSUED FOR BUILDING PERMIT

#	DATE	ISSUED

ISSUED FOR CONSTRUCTION

SIGNATURE _____

DATE _____

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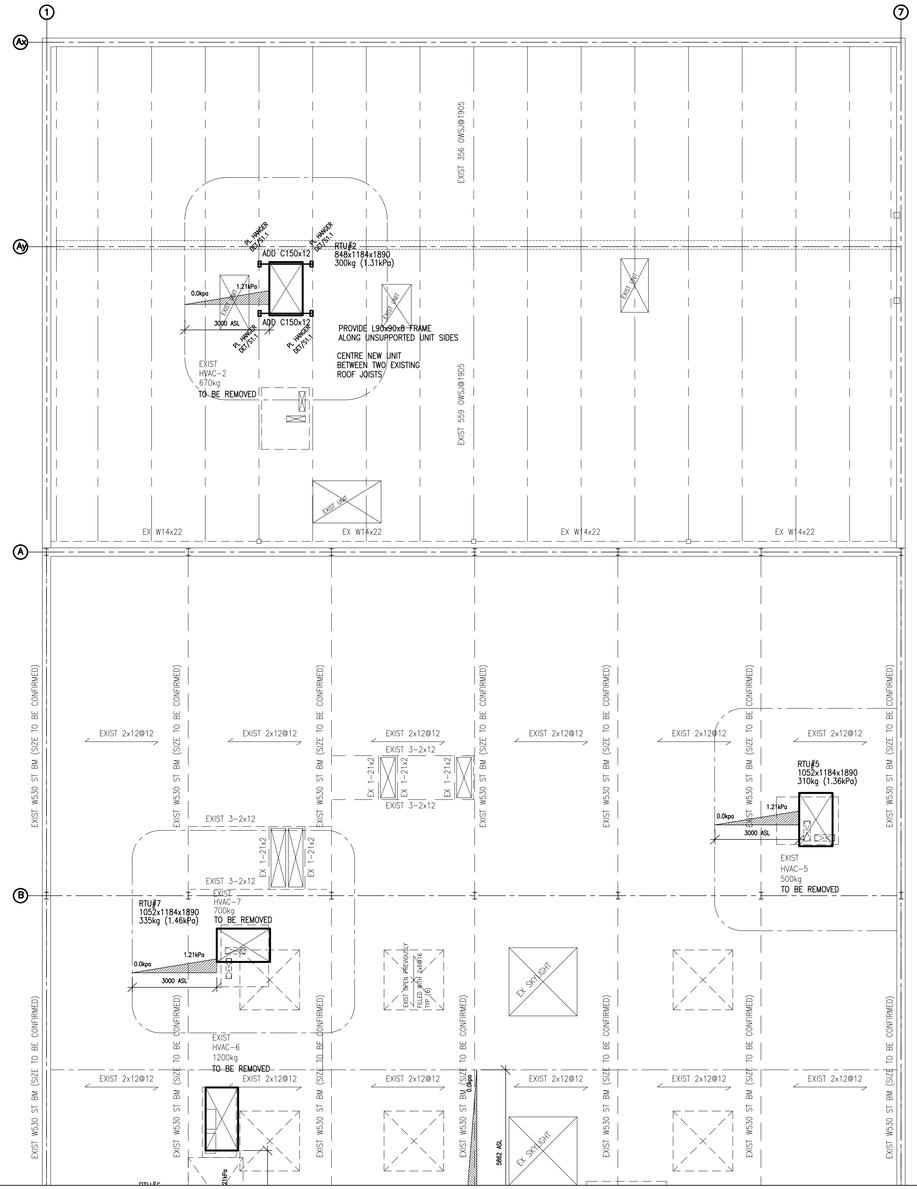
sheet title

ROOF PLAN FOR MECHANIAL UNITS

scale	drawn by	checked by	drawing no.
	SH		S2.2
project no.	plot date:		
24004	2024-05-15		

PLOT DATE: MAY 4, 2024



PART EXISTING ROOF FRAMING PLAN

1:100

1. SEE S2.2 FOR PLAN NOTES.

#	DATE	REVISION
2	24/07/04	ISSUED FOR BUILDING TENDER
1	24/06/21	ISSUED FOR BUILDING PERMIT

#	DATE	ISSUED

ISSUED FOR CONSTRUCTION

SIGNATURE _____

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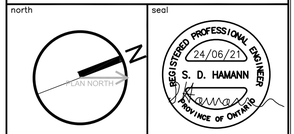

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project architect

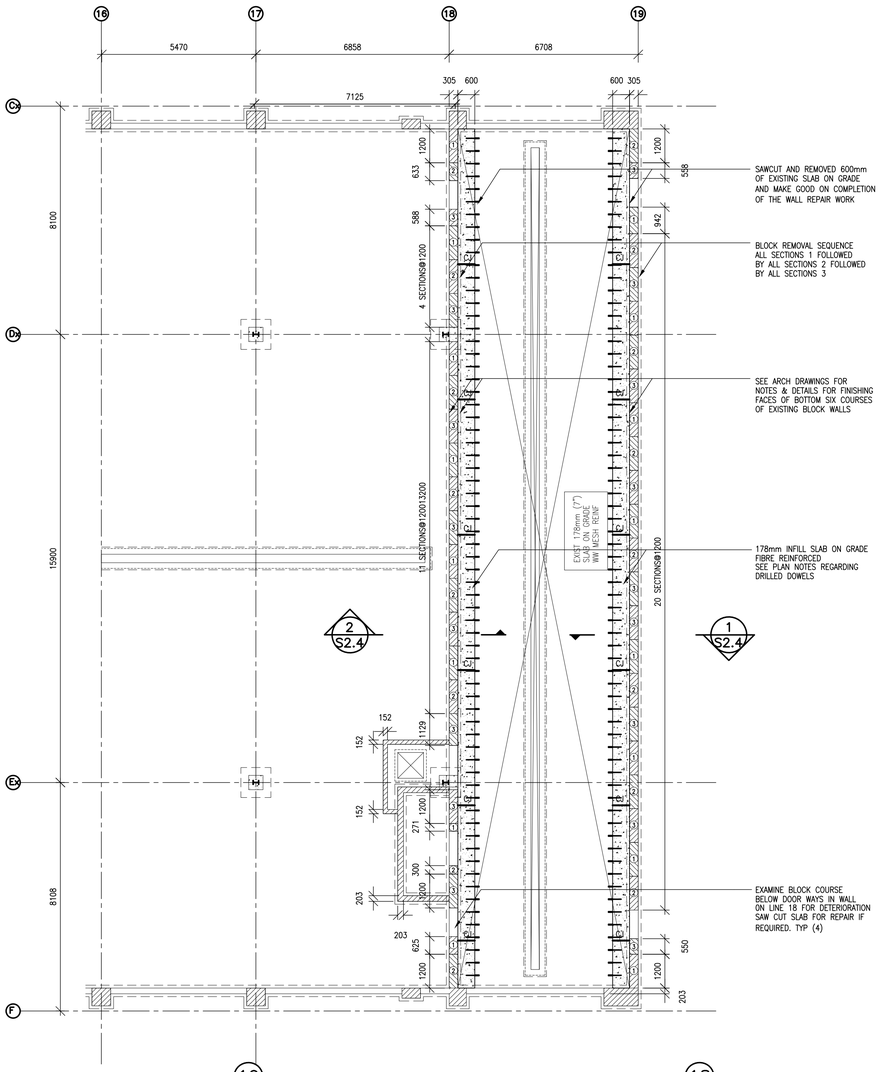
project designer

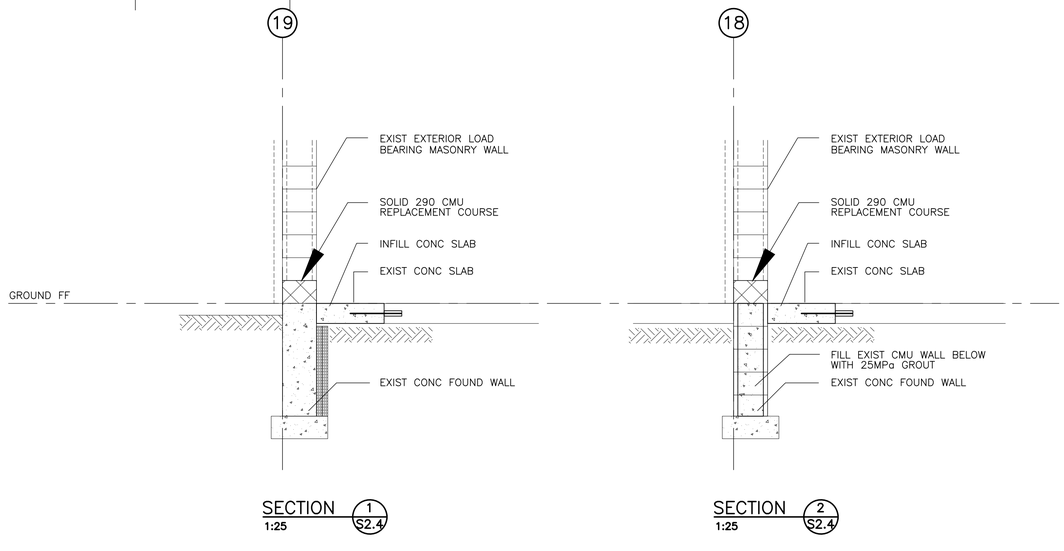
sheet title

ROOF PLAN FOR MECHANIAL UNITS

scale	drawn by	checked by	drawing no.
	SH		S2.3
project no.	plot date:		
24004	2024-05-15		

PLOT DATE: Apr 4, 2024





SECTION 1 / S2.4
1:25

SECTION 2 / S2.4
1:25

PART EXISTING FOUNDATION PLAN

1:100

1. SLAB ON EARTH SHALL BE PLACED ON SOIL CAPABLE OF SUSTAINING 250psf EXCEPT AS CROSSED AND NOTED, AND OF SUFFICIENT COMPACTION THAT NO DIFFERENTIAL SETTLEMENT SHALL TAKE PLACE BETWEEN THE SLAB ON EARTH AND THE THE BUILDING FOOTINGS. INFORMATION RELATING TO THE VALUE OF THE SOIL UNDER THE FOOTINGS IS BASED ON THE INFORMATION AVAILABLE AT THE TIME DRAWINGS ARE ISSUED. THE CONTRACTOR SHALL PLACE FLOORS ON SOIL CAPABLE OF SUPPORTING THE PRESSURES GIVEN ON THE DRAWINGS. ANY ADJUSTMENTS CONSIDERED NECESSARY SHALL BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

2. FINISHED GROUND FLOOR IS AT ELEVATION AS CROSSED AND NOTED. TOP OF CONCRETE SLAB IS 0mm BELOW FINISHED GROUND FLOOR.

3. CONCRETE STRENGTH SHALL BE 25 MPa, UNLESS NOTED. EXPOSED CONCRETE SHALL BE AIR ENTRAINED.

4. RIGID INSULATION WHERE INDICATED SHALL BE DOW SM OR EQUIVALENT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 40 psi.

5. SLAB-ON-GRADE:

   178mm MIN. SLAB-ON-GRADE. REINFORCE SLAB WITH 3/4" FIBRILLATED, POLYPROPYLENE FIBRES FROM FIBREMESH CANADA LIMITED AT 0.9 kg/m3. (0.1 % YIELD BY VOLUME) ADDED AT THE READY-MIX CONCRETE PLANT MEETING ASTM STANDARD C1116-89, SECTION 4.1.3.
   GRANULAR UNDERBED: SUB-FILL TO OPSS 1010 FOR TYPE B AGGREGATE, BUT WITH 100 PASSING THE 22.4 mm SIEVE. OVERLAY THIS WITH 200mm OF COMPACTED GRANULAR 'A' MATERIAL COMPACTED TO A MINIMUM 98% STANDARD PROCTOR DRY DENSITY.

   PROVIDE CONTROL JOINTS AT LOCATIONS WHERE INDICATED ON PLAN. SAWCUT CONTROL JOINTS USING 3/16" BLADE FOR 1/4 DEPTH OF SLAB. SAWCUT JOINTS AS SOON AS POSSIBLE AFTER PLACING CONCRETE BUT NOT LATER THAN 18 HOURS. ALTERNATIVELY, PREFORM CONTROL JOINTS WITH A PROPRIETARY PRODUCT SUCH AS VINYLEX Zip Strips OR APPROVED EQUAL. WHERE NOT INDICATED, PROVIDE SAW CUT JOINTS AT SPACING 15'-0" EACH WAY MAX.

   FILL CONTROL JOINTS AS FOLLOWS:
   FLOORS TO BE COVERED WITH A FINISH MATERIAL USE LATEX MODIFIED MORTAR.
   INTERIOR FLOORS TO BE LEFT EXPOSED USE EPOXY URETHANE FILLER
   EXTERIOR FLOORS TO BE LEFT EXPOSED USE ELASTOMERIC SEALANT FILLER

6. CONTRACTOR SHALL OBTAIN INFORMATION RELATING TO EXISTING AND PROPOSED MECHANICAL SERVICES ADJACENT FOOTING LOCATIONS. ADJUST FOOTING FOUNDING LEVELS ACCORDINGLY. DIRECT INTERFERENCES SHALL BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.

7. DEFINITIONS (ALL DEFINITIONS BELOW MAY NOT BE USED ON THIS DRAWING)
   CJ – SAW CUT CONTROL JOINT. SEE SLAB ON GRADE NOTE 9.
   EX – EXISTING
   EXIST – EXISTING
   TOS – TOP OF SLAB

8. NEW/EXISTING CONSTRUCTION
   1. AT NEW/EXISTING SLAB ON GRADE JOINTS, PROVIDE 10Ø400x450mm LG DOWELS DRILLED & GROUTED 150mm INTO EXISTING SLAB EDGE.

ALL STRUCTURAL INFORMATION RELATING TO EXISTING MEMBERS, PROPERTIES AND LOCATIONS ARE ASSUMED ONLY. EFFORTS HAVE BEEN MADE TO REPRODUCE EXISTING FRAMING BASED ON EXISTING DRAWINGS (IF AVAILABLE), SITE VISITS AND BY VIEWING FROM LOCAL FINISH PENETRATIONS. HAMANN ENGINEERING IS NOT RESPONSIBLE FOR DISCREPANCIES OF ASSUMED EXISTING CONDITIONS ON DRAWINGS AND ACTUAL CONDITIONS DISCOVERED ON SITE.

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SCOPE OF WORK

1. SAW CUT AND REMOVE 600mm OF THE EXISTING SLAB ON GRADE ADJACENT TO THE MASONRY WALLS ON EACH SIDE OF THE TRUCK WASH BAY.
2. ON LINE 18, FILL THE BLOCK SOLID TO THE FOOTING BELOW THE SLAB ON GRADE FULL LENGTH.
3. ON LINE 19, EXAMINE, CLEAN AND REPAIR THE TOP OF THE CONCRETE FOUNDATION WALL BELOW THE SLAB ON GRADE TO RECEIVE THE NEW BLOCK COURSE.
4. ON LINE 18 AND LINE 19, REMOVE AND REPLACE THE FIRST COURSE OF THE EXISTING CMU BLOCK WALL ON EACH SIDE OF THE TRUCK WASH BAY IN THE SEQUENTIAL MANNER INDICATED ON THE PLAN.
5. FOLLOWING THE REPAIR/REPLACEMENT OF THE BLOCK, REMOVE PITTED MATERIAL LAITANCE FROM THE FACE OF THE SECOND AND THIRD THIRD COURSES ABOVE THE IN PREPARATION FOR ARCHITECTURAL FINISH. SEE ALSO ARCHITECTURAL DRAWINGS.
6. ADVISE THE ENGINEER IF DAMAGED BLOCK IS ENCOUNTERED ABOVE THE THE FIRST COURSE WHEN CLEANING THE SECOND AND THIRD COURSES ABOVE THE SLAB. AND THIRD COURSES ABOVE THE SLAB.
7. PROVIDE A UNIT PRICE PER BLOCK FOR ADDITIONAL BLOCK REPLACEMENT ABOVE THE BOTTOM COURSE ON EACH WALL INDICATED ON THE DRAWINGS. SEE CONTRACT SPECIFICATIONS FOR CASH ALLOWANCES IF ANY, FOR EXTRA WORK ARE TO BE CARRIED.
8. REPLACE THE SLAB ON GRADE AS SHOWN ON THE PLAN, AND AS PER THE PLAN NOTES.
9. CLEAN SITE OF ALL WASTE BUILDING MATERIALS AND LEAVE IN A CONDITION SATISFACTORY TO THE OWNER.

#	DATE	REVISION
2	24/08/27	ISSUED FOR BUILDING TENDER
1	24/06/21	ISSUED FOR BUILDING PERMIT

#	DATE	ISSUED

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ISSUED FOR CONSTRUCTION

SIGNATURE _____

DATE _____

client: MISSISSAUGA

client project no.

project:

consultant:

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seal: REGISTERED PROFESSIONAL ENGINEER — S. D. HAMANN — 24/06/21 — PROVINCE OF ONTARIO

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project architect | project designer

sheet title: COMPONENT PRICE 1
TRUCK WASH BAY REPAIR

scale	drawn by	checked by	drawing no.
	SH		S2.4
project no. 24004	plot date: 2024-05-15		

PLOT DATE: August 28, 2024