

GENERAL NOTES

A. GENERAL INFORMATION

1.

READ STRUCTURAL DOCUMENTS IN CONJUNCTION WITH CONTRACT DOCUMENTS, WHICH INCLUDE, BUT ARE NOT LIMITED TO, ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DOCUMENTS.
2.

CONTRACTOR TO BE RESPONSIBLE FOR CHECKING SITE CONDITIONS AGAINST DOCUMENTS, BEFORE PROCEEDING WITH THE WORK, AND REPORT DISCREPANCIES TO THE CONSULTANT.
3.

CONTRACTOR TO PROVIDE LABOUR, MATERIALS, AND EQUIPMENT TO COMPLETE ALL STRUCTURAL WORK INDICATED.
4.

CARRY OUT CONSTRUCTION OPERATIONS, INCLUDING THE INSTALLATION OF TEMPORARY GUYING AND SHORING REQUIRED, ENSURING THAT THE EXISTING STRUCTURE OR MEMBERS ALREADY ERRECTED ARE NOT LOADED IN EXCESS OF THEIR SAFE LOAD CARRYING CAPACITY.
5.

STRUCTURAL DOCUMENTS DO NOT NECESSARILY SHOW ALL OPENINGS AND SLAB VARIATIONS REQUIRED. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR THE EXACT LOCATION, NUMBER, AND SIZE OF OPENINGS, TRENCHES, PITS, SUMPS, SLEEVES, AND DEPRESSIONS. PROVIDE STRUCTURAL FRAMING AT THESE LOCATIONS IN ACCORDANCE WITH THE APPLICABLE TYPICAL DETAIL.

B. REFERENCE STANDARDS/CODES AND ACTS

1.

CONFORM WITH THE 2012 BUILDING CODE (ONTARIO REGULATION 332/12, AMENDED BY ONTARIO REGULATION 89/23), AND ANY APPLICABLE ACTS OF ANY AUTHORITY HAVING JURISDICTION, AND THE FOLLOWING:

REF	CODE	TITLE
f)	CAN/CSA-S16	LIMIT STATES DESIGN OF STEEL STRUCTURES
n)	S304.1	DESIGN OF MASONRY STRUCTURES

2.

ALL STANDARDS AND PUBLICATIONS REFERENCED BY THE STANDARDS NOTED ABOVE ARE TO APPLY.
3.

WHERE THERE ARE DIFFERENCES BETWEEN THE DOCUMENTS AND THE STANDARDS, CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN.

C. MATERIALS

1.

PROVIDE ONLY NEW STRUCTURAL MATERIALS IN ACCORDANCE WITH THE REFERENCE STANDARDS AND THE FOLLOWING, UNLESS OTHERWISE NOTED.
- 1.1.

WELDED WIRE FABRIC: CONFORM TO CSA G30 SERIES, GRADE 386, IN FLAT SHEETS.
- 1.2.

STRUCTURAL STEEL:

(a)

STRUCTURAL WIDE FLANGE AND WELDED WIDE FLANGE SHAPES (W, WWF) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 350W.

(b)

ANGLES AND CHANNELS (L, C) AND PLATES TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.

(c)

HOLLOW STRUCTURAL SECTIONS (HSS) TO CONFORM TO ASTM A500 GRADE C.
- 1.5.

PRIME PAINT: CONFORM TO CIS/CPMA STANDARD 2-75.
- 1.6.

HOT DIP GALVANIZING: TO CSA G164, MINIMUM ZINC COATING OF 600G/M2.
- 1.7.

STRUCTURAL BOLTS, NUTS AND WASHERS: CONFORM TO ASTM A325M.
- 1.8.

ANCHOR RODS - CONFORM TO THE REQUIREMENTS OF ASTM F1554 GRADE.
- 1.9.

NON-SHRINK GROUT = COMPRESSIVE STRENGTH OF 35 MPa AT 24 HOURS.
- 1.10.

BLOCK: CONFORM TO CAN3-A165 SERIES, MINIMUM COMPRESSIVE STRENGTH = 15.0 MPa (MIN.) BASED ON NET AREA.
- 1.11.

MORTAR: CONFORM TO CSA A179 TYPE S FOR LOADBearing WALLS UNLESS NOTED.
- 1.12.

MASONRY GROUT: CONFORM TO CSA A179, 15 MPa MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, 250 mm (10") SLUMP, MAXIMUM AGGREGATE SIZE 10 MM (3/8")

D. EXECUTION

1.

FOUNDATIONS

1.1.

FOUND ALL FOOTINGS (AND UNDERPINNING) ON SOIL CAPABLE OF SUSTAINING A MINIMUM ULTIMATE LIMIT STATES/ SERVICE LIMIT STATES BEARING STRESS (ULS/SLS) OF 3100psf/4700psf.

1.2.

FOUND ALL FOOTINGS WHICH WILL BE EXPOSED TO FROST ACTION IN THE COMPLETED BUILDING A MINIMUM OF 1200 mm (4'-0") BELOW FINISHED GRADE.

1.3.

DO NOT EXCEED A RISE OF 7 IN A RUN OF 10 IN THE LINE OF SLOPE BETWEEN ADJACENT FOOTING EXCAVATIONS OR ALONG STEPPED FOOTINGS. FOR STEPPED FOOTINGS, USE STEPS NOT EXCEEDING 600 mm (2'-0") IN HEIGHT AND 1200 mm (4'-0") (MIN.) IN LENGTH.

1.4.

SOIL BEARING CAPACITY SPECIFIED MUST BE VERIFIED BY THE SOIL ENGINEER PRIOR TO THE PLACING OF FOOTINGS AND ANY NON-CONFORMANCE WITH THE SPECIFIED MINIMUM CAPACITIES MUST BE IMMEDIATELY REPORTED TO THE STRUCTURAL ENGINEERS.
2.

SLAB ON GRADE

2.1.

PLACE SLAB-ON-GRADE ON SUB-GRADE MATERIAL CAPABLE OF SUSTAINING A MINIMUM BEARING CAPACITY OF 25 kPa (500 psf) WITHOUT SETTLEMENT RELATIVE TO THE BUILDING FOOTINGS.

2.2.

UNLESS OTHERWISE NOTED, PROVIDE IMMEDIATELY UNDER SLABS-ON-GRADE A MINIMUM OF 200 mm (8") OF COMPACTED (MTC) GRANULAR 'B' MATERIAL. COMPACTION TO ACHIEVE A MINIMUM OF 98% STANDARD PROCTOR MAXIMUM DRY DENSITY.
3.

STRUCTURAL STEEL

3.1.

PAINt ALL STRUCTURAL STEEL TO REQUIREMENTS OF CIS/CPMA 2-75. TOUCH UP ALL FIELD WELDS.

3.2.

ALL STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH CSA G164.

3.3.

ALL WELDS SHALL CONFORM TO CSA STANDARD W59.

3.4.

ALL WELDS EXPOSED TO VIEW SHALL BE GROUND SMOOTH.

3.5.

ANY ORGANIZATION UNDERTAKING TO WELD UNDER THIS CONTRACT SHALL BE CERTIFIED BY THE CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF DIVISION 1 OR DIVISION 2.1 OF W47.1.

3.6.

UNLESS A REINFORCED MASONRY OR CONCRETE LINTEL IS SHOWN, IN MASONRY WALLS OR MASONRY PARTITIONS PROVIDE LOOSE STEEL LINTELS IN ACCORDANCE WITH REQUIREMENTS OF DOCUMENTS OVER ALL DOORWAYS, OTHER OPENINGS, AND RECESSES, INCLUDING THOSE FOR MECHANICAL OR ELECTRICAL SERVICES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE LOCATION, SIZE AND NUMBER OF OPENINGS REQUIRED BY THE MECHANICAL AND ELECTRICAL CONSULTANT.

- 3.7.

DO NOT SPLICE STRUCTURAL STEEL SECTIONS WITHOUT PRIOR APPROVAL OF THE CONSULTANT. ALL SPLICES SHALL DEVELOP THE FULL CAPACITY OF THE SECTION AND ARE TO BE TESTED BY NON DESTRUCTIVE METHODS, BY AN INDEPENDENT INSPECTION AND TESTING COMPANY, AT THE CONTRACTOR'S EXPENSE.
- 3.8.

COMPLETELY FILL VOIDS BENEATH STEEL BASES ON CONCRETE WITH AN APPROVED NON-SHRINK 36 MPa (5 ksi) GROUT
- 3.9.

SEE ARCHITECTURAL DRAWINGS FOR FIREPROOFING REQUIREMENTS. CONFIRM COMPATIBILITY OF FIREPROOFING MATERIAL WITH STEEL PAINT.

4. MASONRY

- 4.1.

PROVIDE A MINIMUM LENGTH OF 200 mm (8") OF 100% SOLID MASONRY UNITS FOR BEARING OF STEEL, CONCRETE OR REINFORCED MASONRY LINTELS.
- 4.2.

SUPPLY AND PLACE REINFORCEMENT AND CONCRETE FOR REINFORCED MASONRY LINTELS IN ACCORDANCE WITH TYPICAL DETAILS SHOWN.
- 4.3.

FOR THE PURPOSE OF DETERMINING THE REQUIRED REINFORCING FOR LOAD-BEARING AND NON-LOAD BEARING MASONRY WALLS THE SEISMIC HAZARD INDEX IEFASA(0.2) = XXXXX. REFER TO TYPICAL DETAILS FOR REINFORCING REQUIREMENTS.
5.

ALTERATIONS AND/OR CONNECTIONS TO EXISTING STRUCTURE

5.1.

INSPECT THE EXISTING BUILDING AND BECOME THOROUGHLY FAMILIAR WITH THE EXISTING CONDITIONS.

5.2.

PRIOR TO FABRICATION OF STRUCTURAL STEEL, OPEN UP ALL AREAS WHERE CONNECTIONS ARE TO BE MADE TO EXISTING WORK AND TAKE FIELD MEASUREMENTS. MODIFY METHODS FOR CONNECTING TO SUIT SITE CONDITIONS FOUND AND TO THE APPROVAL OF THE CONSULTANT. CARRY OUT LOCAL REPAIRS TO THE EXISTING WORK AS NECESSARY AND AS DIRECTED BY THE CONSULTANT.

5.3.

SHORE EXISTING WORK AS REQUIRED UNTIL ALL NEW WORK HAS BEEN COMPLETED AND REVIEWED BY THE CONSULTANT.

5.4.

SHORE FLOORS AS REQUIRED TO SUPPORT CRANES, HOISTS AND OTHER CONSTRUCTION EQUIPMENT.

5.5.

DO NOT CUT CONCRETE REINFORCEMENT UNLESS REVIEWED AND APPROVED BY THE CONSULTANT.

5.6.

WHERE REQUIRED TO AVOID CUTTING EXISTING REINFORCEMENT, MODIFY THE LAYOUT OF NEW THROUGH BOLTS, EXPANSION ANCHORS AND OTHER ANCHORING DEVICES.

5.7.

MAKE GOOD THE EXISTING WORK.
6.

CUTTING AND CORING OF EXISTING STRUCTURE

6.1.

PRIOR TO CUTTING AND CORING ANY OPENINGS IN THE EXISTING BUILDING, PROVIDE THE CONSULTANT WITH A SLEEVEING DRAWING INDICATING THE SIZE AND LOCATION OF OPENINGS RELATIVE TO BUILDING GRIDLINES. EXISTING OPENINGS IN THE VICINITY OF THE NEW OPENING MUST ALSO BE SHOWN.

6.2.

ALL DIMENSIONS PROVIDED TO THE CONSULTANT ARE TO BE CONFIRMED WITH THE APPROPRIATE CONTRACTOR (MECHANICAL OR ELECTRICAL) PRIOR TO CUTTING/CORING.

6.3.

ANY REVISIONS TO THE DIMENSIONS BY THE CONSULTANT MUST BE REVIEWED BY THE APPROPRIATE CONTRACTOR PRIOR TO CUTTING/CORING.

6.4.

THE CONSULTANT MAY IDENTIFY AREAS WHERE EXISTING REINFORCEMENT AND EMBEDDED SERVICES MUST BE LOCATED PRIOR TO CUTTING/CORING. THIS REINFORCEMENT IS TO BE LOCATED BY A POSITIVE MEANS, (I.E. X-RAYING OR SCANNING OF SLAB).

6.5.

AFTER REINFORCEMENT AND EMBEDDED SERVICES HAS BEEN LOCATED IN THESE AREAS, NOTIFY CONSULTANT WHO WILL REVIEW AND APPROVE OF LOCATION PRIOR TO CUTTING/CORING. MAKE ANY NECESSARY ADJUSTMENTS TO THE HOLE LOCATION AS DIRECTED BY THE CONSULTANT.

6.6.

FOR ANY OPENINGS WHICH ARE TO BE SAWCUT INTO THE EXISTING STRUCTURE, PRE-DRILL THE CORNERS USING A 100 MM DIAMETER CORE DRILL. DO NOT OVERCUT CORNERS OF OPENING.

6.7.

ALL PRICES FOR CUTTING/CORING ARE TO INCLUDE ANY COSTS ASSOCIATED WITH X-RAYING, SCANNING, ETC.

6.8.

FOR ANY AREAS WHERE REINFORCEMENT IS CUT, THE CONTRACTOR SHALL INDICATE THE DIRECTION AND LAYER OF REINFORCEMENT ON THE AS-BUILT SLEEVEING DRAWINGS.

F. QUALITY CONTROL

1.

GENERAL

1.1.

IMPLEMENT A SYSTEM OF QUALITY CONTROL TO ENSURE THAT THE MINIMUM STANDARDS SPECIFIED HEREIN ARE ATTAINED.

1.2.

BRING TO THE ATTENTION OF THE CONSULTANT ANY DEFECTS IN THE WORK OR DEPARTURES FROM THE CONTRACT DOCUMENTS, WHICH MAY OCCUR DURING CONSTRUCTION. THE CONSULTANT WILL DECIDE UPON CORRECTIVE ACTION AND GIVE RECOMMENDATIONS IN WRITING.

1.3.

THE CONSULTANT'S GENERAL REVIEW DURING CONSTRUCTION AND INSPECTION AND TESTING BY INDEPENDENT INSPECTION AND TESTING AGENCIES REPORTING TO THE CONSULTANT ARE BOTH UNDERTAKEN TO INFORM THE OWNER/CLIENT OF THE CONTRACTOR'S PERFORMANCE AND SHALL IN NO WAY AUGMENT THE CONTRACTOR'S QUALITY CONTROL OR RELIEVE THE CONTRACTOR OF CONTRACTUAL RESPONSIBILITY.
2.

NOTIFICATION

2.1.

PRIOR TO COMMENCING SIGNIFICANT SEGMENTS OF THE WORK, GIVE THE CONSULTANT AND INDEPENDENT INSPECTION AND TESTING COMPANIES APPROPRIATE NOTIFICATION (MINIMUM 24 HOURS) SO AS TO AFFORD THEM REASONABLE OPPORTUNITY TO REVIEW THE WORK. FAILURE TO MEET THIS REQUIREMENT MAY BE CAUSE FOR THE CONSULTANT TO CLASSIFY THE WORK AS DEFECTIVE.
3.

INSPECTION AND TESTING

3.1.

AN INDEPENDENT INSPECTION AND TESTING COMPANY SHALL MAKE INSPECTIONS OR PERFORM TESTS AS THE CONSULTANT DIRECTS. THE INDEPENDENT INSPECTION AND TESTING COMPANIES SHALL BE RESPONSIBLE ONLY TO THE CONSULTANT AND SHALL MAKE ONLY SUCH INSPECTIONS OR TESTS AS THE CONSULTANT MAY DIRECT.

3.2.

THE FOLLOWING ITEMS REQUIRE TESTING AND/OR INSPECTION BY A CERTIFIED, INDEPENDENT INSPECTION AND TESTING COMPANY UNLESS OTHERWISE NOTED. THE TESTING FIRM SHALL SUBMIT COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE CONSULTANT FOR REVIEW:
4.

DEFECTIVE MATERIALS AND WORK

4.1.

WHERE EVIDENCE EXISTS THAT DEFECTIVE WORK HAS OCCURRED OR THAT WORK HAS BEEN CARRIED OUT INCORPORATING DEFECTIVE MATERIALS, THE CONSULTANT MAY HAVE TESTS, INSPECTIONS OR SURVEYS PERFORMED, ANALYTICAL CALCULATIONS OF STRUCTURAL STRENGTH MADE, AND THE LIKE, IN ORDER TO HELP DETERMINE WHETHER THE WORK MUST BE CORRECTED OR REPLACED. TESTS, INSPECTIONS OR SURVEYS OR CALCULATIONS CARRIED OUT UNDER THESE CIRCUMSTANCES WILL BE MADE AT THE CONTRACTOR'S EXPENSE, REGARDLESS OF THEIR RESULTS, WHICH MAY BE SUCH THAT, IN THE CONSULTANT'S OPINION, THE WORK MAY BE ACCEPTABLE.

- 4.2.

ALL TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2012 BUILDING CODE (ONTARIO REGULATION 332/12), EXCEPT WHERE THIS WOULD, IN THE CONSULTANT'S OPINION, CAUSE UNDUE DELAY OR GIVE RESULTS NOT REPRESENTATIVE OF THE REJECTED MATERIAL IN PLACE. IN THIS CASE, THE TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARDS GIVEN BY THE CONSULTANT.
- 4.3.

MATERIALS OR WORK, WHICH FAIL TO MEET SPECIFIED REQUIREMENTS, MAY BE REJECTED BY THE CONSULTANT WHENEVER FOUND AT ANY TIME PRIOR TO FINAL ACCEPTANCE OF THE WORK REGARDLESS OF PREVIOUS INSPECTION. IF REJECTED, DEFECTIVE MATERIALS OR WORK SHALL BE PROMPTLY REMOVED AND REPLACED OR REPAIRED TO THE SATISFACTION OF THE CONSULTANT, AT NO EXPENSE TO THE OWNER.

LIST OF STRUCTURAL DRAWINGS

SHEET NO.	SHEET TITLE
S101	GENERAL NOTES
S111	TYPICAL DETAILS
S201	PART FOUNDATION/PART SECOND FLOOR & LOW ROOF FRAMING PLAN
S202	PART HIGH ROOF FRAMING PLAN
	FRAMING PLANS



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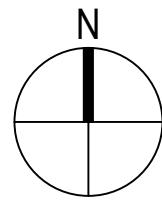
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Professional Seal

No.	Revision Description	YYYY-MM-DD
1	ISSUED FOR PERMIT	2024 - 11 - 27

North



Project Title:

DUNBARTON H.S. - SOUTH CAMPUS
NEW WALL OPENINGS

655 SHEPPARD AVE,
PICKERING, ON

Designed by:	MAM	Scale:	AS NOTED
Drawn by:	MAM	Date:	NOV 26, 2024
Sheet Title			

GENERAL NOTES

Sheet Number

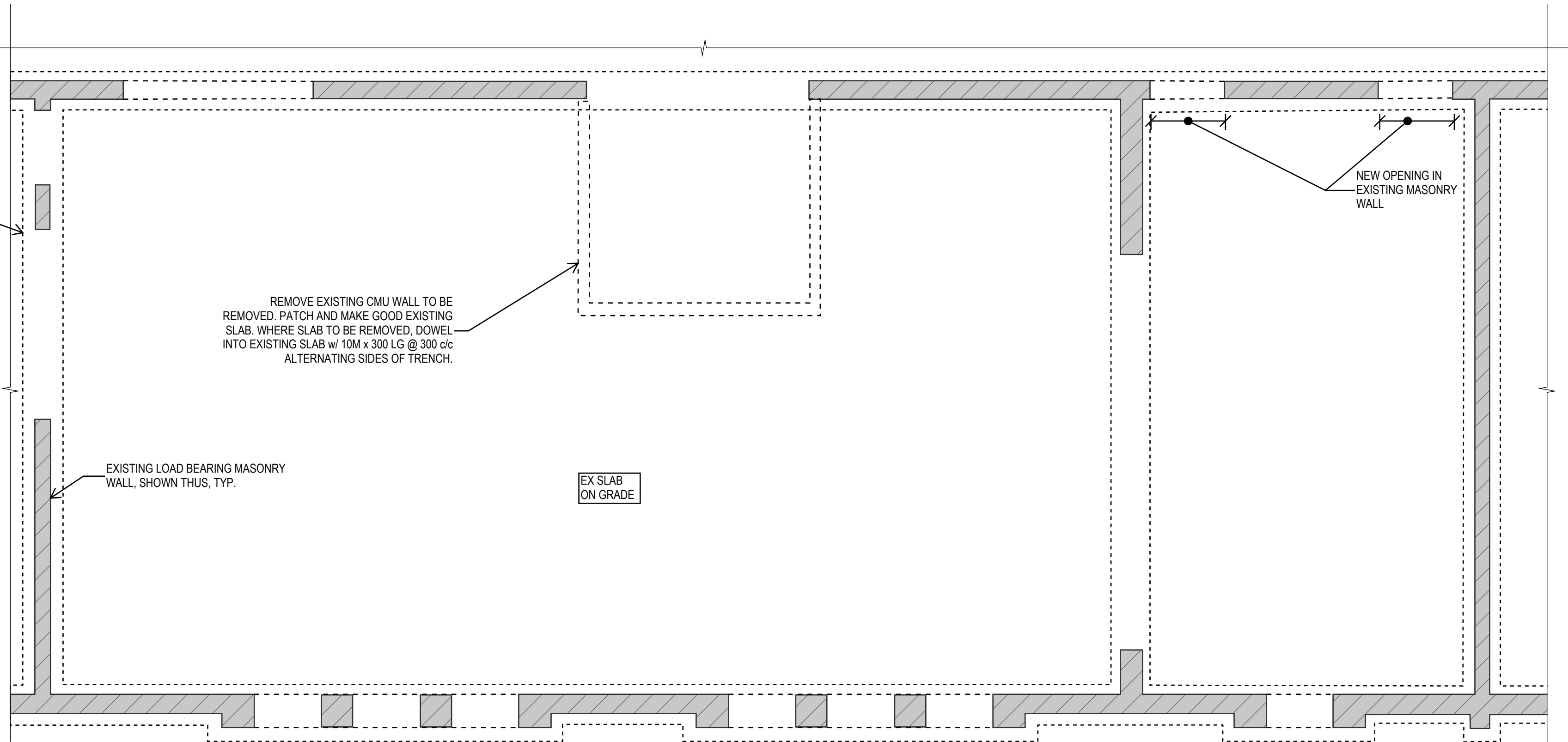
S101

No.	Revision Description	YYYY-MM-DD
1	ISSUED FOR PERMIT	2024 - 11 - 27

A PART FOUNDATION PLAN
S201 1/4"=1'-0"

GROUND FLOOR @ GRADE PLAN NOTES

- 1.TOP OF SLAB ON GRADE TO BE 0'-0" (0.0 m) BELOW GROUND FLOOR ELEVATION 0'-0", EXCEPT AS CROSSED AND NOTED.
- 2.CENTRE LINES OF COLUMNS, CAPS AND FOOTINGS ARE COINCIDENT UNLESS OTHERWISE NOTED.
- 3.REFER TO GENERAL NOTES FOR REQUIRED SOIL BEARING CAPACITY.
- 4.REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL NEW BELOW SLAB SERVICES AND TRENCHING REQUIREMENTS. LOCALLY REMOVE THE SLAB-ON-GRADE AS REQUIRED FOR INSTALLATION OF SUCH SERVICES. MAKE GOOD SLAB-ON-GRADE PER TYPICAL DETAILS.
- 5.REFER TO ARCHITECTURAL DRAWINGS FOR FULL EXTENT OF DEMOLITION AND ALL OPENING SIZES.
- 6.PROVIDE CLEAN SAW CUT AND CORE LINES AT ALL NEW ARCHITECTURAL AND MECHANICAL OPENINGS. MAKE GOOD ALL DAMAGED BLOCK/BRICK ADJACENT TO OPENINGS TO THE ARCHITECT'S AND OWNER'S REQUIREMENTS, UNLESS OTHERWISE NOTED ON PLAN. PROVIDE STEEL LINTELS ABOVE ALL NEW OPENINGS IN ACCORDANCE WITH TYPICAL DETAILS. REFER TO ARCHITECTURAL/MECHANICAL DRAWINGS FOR NUMBER AND LOCATION OF ALL NEW OPENINGS.
- 7.MAKE GOOD ALL EXTERIOR AND INTERIOR FINISHES IMPACTED BY THE WORKS AS PER THE ARCHITECT'S AND OWNER'S REQUIREMENTS.
- 8.FULLY INFILL ALL ABANDONED DUCT OPENINGS THROUGH EXISTING MASONRY WALLS USING CONCRETE BLOCK MATCHING EXISTING. PACK NEW TIGHT TO UNDERSIDE EXISTING LINTELS/SLAB ABOVE. CONTRACTOR TO CONFIRM EXACT LOCATION AND NUMBER OF ABANDONED OPENINGS WITH MECHANICAL.
- 9.STRUCTURAL DOCUMENTS DO NOT NECESSARILY SHOW ALL NEW MECHANICAL/ELECTRICAL SERVICE PENETRATIONS THROUGH EXISTING CONCRETE BLOCK WALLS. REFER TO MECHANICAL/ELECTRICAL DRAWINGS FOR ALL SUCH LOCATIONS. FOR OPENINGS THROUGH WALLS GREATER THAN 300MM (1'-0") WIDE PROVIDE NEW STEEL LINTEL PER TYPICAL DETAIL TO-301. OFFSET OPENINGS SUCH THAT THE DO NOT FALL DIRECTLY BELOW BEARING OF EXISTING STRUCTURAL BEAMS/JOISTS ABOVE. (STAFF NOTE: ALWAYS CONFIRM THIS VALUE. LINTELS MAY BE REQUIRE OVER SMALLER OPENING UNDER CERTAIN LOADING CONDITIONS).

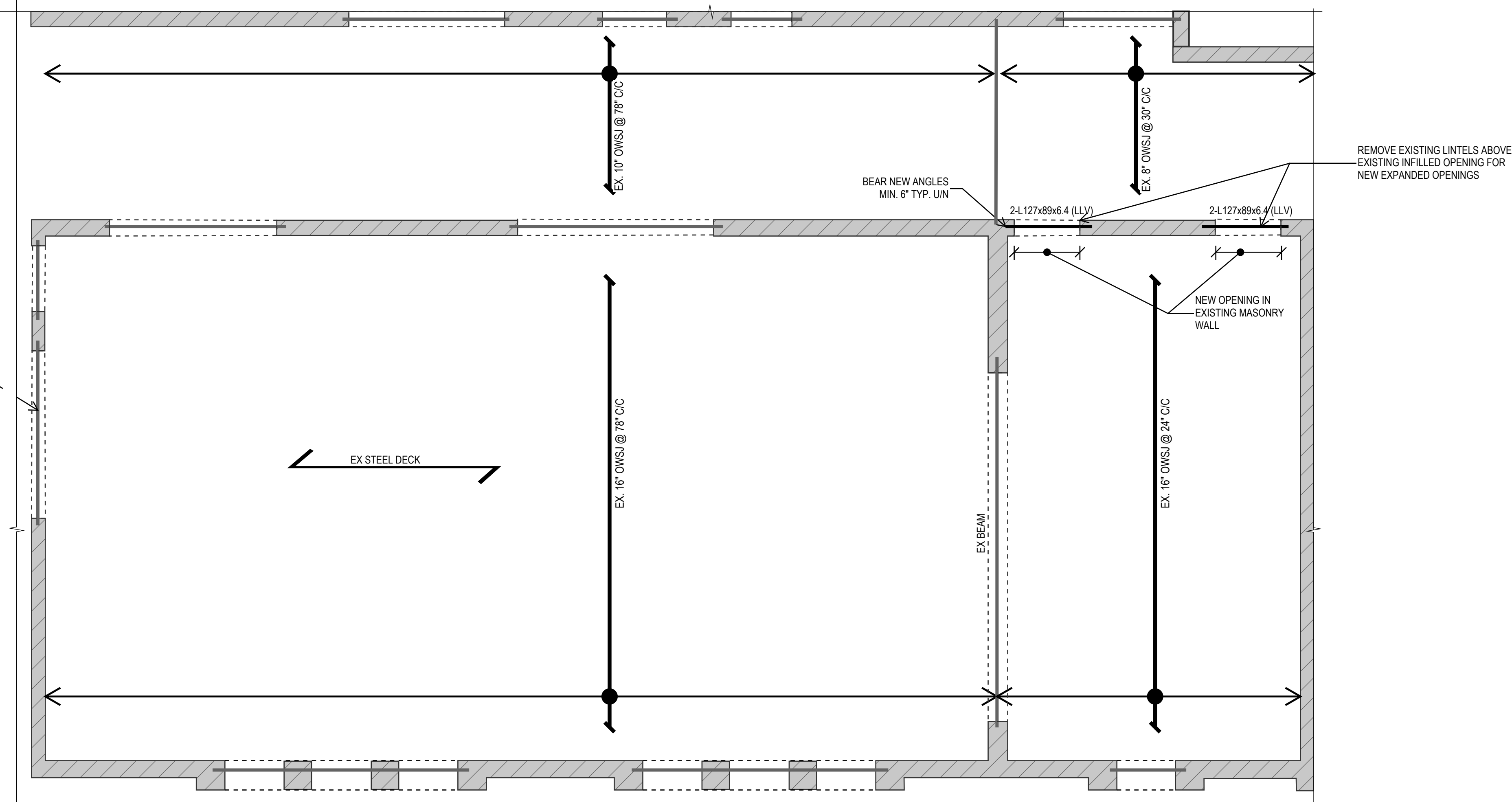


B PART LOW ROOF AND SECOND FLOOR FRAMING PLAN
S201 1/4"=1'-0"

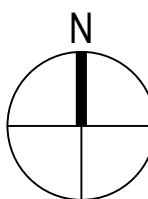
STEEL FLOOR PLAN NOTES

- 1.TOP OF CONCRETE SLAB TO BE 0'-0" (0.0 m) BELOW SECOND FLOOR ELEVATION 12'-0", EXCEPT AS CROSSED AND NOTED. AREAS CROSSED AND NOTED TO BE READ FROM GROUND FLOOR ELEVATION 0'-0".
- 2.JOIST SHOES ARE TO BE 4" (100 mm) DEEP UNLESS OTHERWISE NOTED.
- 3.EXISTING STRUCTURE HAS BEEN CHECKED FOR NEW LOADS (INCL. WIND LOADS) AND WAS FOUND TO BE ACCEPTABLE.
- 4.LIVE LOADS ARE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN:
CLASSROOMS 50psf
STAIRS AND CORRIDORS 100psf
LOW ROOF 29psf + ASL
- 6.SUPERIMPOSED DEAD LOADS ARE:
STAIRS AND CORRIDORS 75 psf
CLASSROOMS 55 psf

EX. LINTELS SHOWN THUS, TYP.



North



Project Title:

**DUNBARTON H.S. - SOUTH CAMPUS
NEW WALL OPENINGS**

655 SHEPPARD AVE,
PICKERING, ON

Designed by:	MAM	Scale:	1/4" = 1'-0"
Drawn by:	MAM	Date:	NOV 26, 2024

Sheet Title

**PART FOUNDATION & PART
LOW ROOF/SECOND FLOOR
FRAMING PLANS**

Sheet Number

S201