GENERAL NOTES

A. GENERAL INFORMATION

1. READ STRUCTURAL DOCUMENTS IN CONJUNCTION WITH CONTRACT DOCUMENTS, WHICH INCLUDE, BUT ARE NOT LIMITED TO, ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DOCUMENTS.

2. CONTRACTOR TO BE RESPONSIBLE FOR CHECKING SITE CONDITIONS AGAINST DOCUMENTS BEFORE PROCEEDING WITH THE WORK, AND REPORT DISCREPANCIES TO THE CONSULTANT.

3. CONTRACTOR TO PROVIDE LABOUR, MATERIALS, AND EQUIPMENT TO COMPLETE ALL STRUCTURAL WORK INDICATED.

4. CARRY OUT CONSTRUCTION OPERATIONS, INCLUDING THE INSTALLATION OF TEMPORARY GUYING AND SHORING REQUIRED, ENSURING THAT THE EXISTING STRUCTURE OR MEMBERS ALREADY ERECTED ARE NOT LOADED IN EXCESS OF THEIR SAFE LOAD CARRYING CAPACITY.

5. STRUCTURAL DOCUMENTS DO NOT NECESSARILY SHOW ALL OPENINGS AND SLAB VARIATIONS REQUIRED. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR THE EXACT LOCATION, NUMBER, AND SIZE OF OPENINGS, TRENCHES, PITS, SUMPS, SLEEVES, AND DEPRESSIONS. PROVIDE STRUCTURAL FRAMING AT THESE LOCATIONS IN ACCORDANCE WITH THE APPLICABLE TYPICAL DETAIL.

B. REFERENCE STANDARDS / CODES AND ACTS

1. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH AND SHALL BE CONSTRUCTED TO CONFORM WITH THE 2024 ONTARIO BUILDING CODE, ONTARIO REGULATION 203/24 (REFERRED TO AS "THE BUILDING CODE"), ANY APPLICABLE ACTS OF ANY AUTHORITY HAVING JURISDICTION, AND THE FOLLOWING:

TABLE B.1: REFERENCE STANDARDS

REF	CODE	TITLE
a)	CAN/CSA A23.1	CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION
b)	CAN/CSA A23.2	METHODS OF TEST FOR CONCRETE
c)	CAN/CSA A23.3	DESIGN OF CONCRETE STRUCTURES
d)	CAN/CSA-S16	LIMIT STATES DESIGN OF STEEL STRUCTURES
e)	CAN/CSA G40.20/G40.21	STRUCTURAL QUALITY STEEL
f)	RSIC	REINFORCING STEEL INSTITUTE OF CANADA, MANUAL OF STANDARD PRACTICE
g)	CAN/CSA-A370	CONNECTORS FOR MASONRY
h)	CSA-A371	MASONRY CONSTRUCTION FOR BUILDINGS
i)	S304.1	DESIGN OF MASONRY STRUCTURES
j)	CSA G30.18	CARBON STEEL BARS FOR CONCRETE REINFORCING

2. ALL STANDARDS AND PUBLICATIONS REFERENCED BY THE STANDARDS NOTED ABOVE ARE TO APPLY.

3. WHERE THERE ARE DIFFERENCES BETWEEN THE DOCUMENTS AND THE STANDARDS, CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN.

C. MATERIALS

1. PROVIDE ONLY NEW STRUCTURAL MATERIALS IN ACCORDANCE WITH THE REFERENCE STANDARDS AND THE FOLLOWING, UNLESS OTHERWISE NOTED.

1.1. CONCRETE:

1.1.1. CONCRETE STRENGTHS FOR STRUCTURAL ELEMENTS SHALL BE AS PER TABLE BELOW, UNLESS NOTED OTHERWISE ON PLANS, SCHEDULES, AND/OR SECTIONS.

TABLE C.1: CONCRETE STRENGTHS

STRUCTURAL ELEMENT AND EXPOSURE	EXPOSURE CLASS PER CSA A23.1	CONCRETE STRENGTH f'c (MPa)	SLUMP (mm)	MAX W/C RATIO	AIR CONTENT
SLABS ON GRADE NOT EXPOSED TO CHLORIDES OR FREEZING		25	40*	0.45	
NOTES					

1. TOLERANCE FOR SLUMP SHALL BE ± 20 mm FOR SPECIFIED SLUMP LESS THAN 80 mm, AND ± 30 mm FOR SPECIFIED SLUMP BETWEEN 80 mm AND 170 mm.
2. SPECIAL CONCRETE HANDLING AND PLACING METHODS, OR THE USE OF A SUPER PLASTICIZER, WILL BE REQUIRED TO PLACE THIS CONCRETE. FINAL PLASTICIZED SLUMP SHALL BE ± 125 mm.
3. WHERE AGGREGATES SMALLER THAN 14 mm ARE USED, INCREASE AIR CONTENT BY 1%.
4. CONCRETE EXPOSED TO DE-ICING CHEMICALS SHALL HAVE DCI TYPE N CORROSION INHIBITOR AT 18 L/m³ DOSAGE (MINIMUM) OR APPROVED EQUIVALENT.

1.2. REINFORCING STEEL: CONFORM TO CSA G30 SERIES, GRADE 400.

1.3. WELDED WIRE FABRIC: CONFORM TO CSA G30 SERIES, GRADE 386, IN FLAT SHEETS.

1.4. STRUCTURAL STEEL:

1.4.1. STRUCTURAL WIDE FLANGE (W) AND WELDED WIDE FLANGE SHAPES (WWF) TO CONFORM TO CAN/CSA G40.20/G40.21 GRADE 350W.

1.4.2. ANGLES (L), CHANNELS (C), AND PLATES TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.

1.5. PRIME PAINT: CONFORM TO CISC/CPMA STANDARD 2-75.

1.6. NON-SHRINK GROUT = COMPRESSIVE STRENGTH OF 35 MPa AT 24 HOURS.

1.7. BLOCK: CONFORM TO CAN3-A165 SERIES, MINIMUM COMPRESSIVE STRENGTH, fm = 15 MPa BASED ON NET AREA.

1.8. MORTAR: CONFORM TO CSA A179 TYPE S FOR LOAD-BEARING WALLS UNLESS NOTED.

1.9. MASONRY GROUT: CONFORM TO CSA A179, 15 MPa MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, 250 mm (10") SLUMP, MAXIMUM AGGREGATE SIZE 10 mm (3/8").

D. EXECUTION

1. SLAB ON GRADE

1.1. PLACE SLAB-ON-GRADE ON SUB-GRADE MATERIAL CAPABLE OF SUSTAINING A MINIMUM BEARING CAPACITY OF 25 kPa (500 psf) WITHOUT SETTLEMENT RELATIVE TO THE BUILDING FOOTINGS.

1.2. UNLESS OTHERWISE NOTED, PROVIDE IMMEDIATELY UNDER SLABS-ON-GRADE A MINIMUM OF 200 mm (8") OF COMPACTED (MTC) GRANULAR 'B' MATERIAL. COMPACTION TO ACHIEVE A MINIMUM OF 98% STANDARD PROCTOR MAXIMUM DRY DENSITY.

2. CONCRETE

2.1. CONSTRUCTION JOINTS FOR WALLS, SLABS, AND BEAMS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION. GENERALLY JOINTS IN SLABS SHALL BE AT RIGHT ANGLES TO THE SPANS, AT MID-SPAN IF POSSIBLE, AND BE CLEAR OF SUPPORTS AND POINT LOADS.

2.2. WHEN ATMOSPHERIC TEMPERATURE IS AT OR BELOW 5°C, OR WHEN THERE IS A POSSIBILITY OF IT FALLING TO THAT LIMIT, PLACE CONCRETE IN ACCORDANCE WITH THE REQUIREMENTS OF CAN/CSA A23.1 "COLD-WEATHER CONCRETING" AND ACI 306 "RECOMMENDED PRACTICE FOR COLD-WEATHER CONCRETING". WHEN ATMOSPHERIC TEMPERATURE IS AT OR ABOVE 27° C, PLACE CONCRETE IN ACCORDANCE WITH CAN/CSA A23.1 "HOT WEATHER CONCRETING" AND ACI 305 "RECOMMENDED PRACTICES FOR HOT WEATHER CONCRETING".

3. STRUCTURAL STEEL

3.1. PAINT ALL STRUCTURAL STEEL TO REQUIREMENTS OF CISC/CPMA 2-75. TOUCH UP ALL FIELD WELDS.

3.2. ALL STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH CSA G164.

3.3. ALL WELDS SHALL CONFORM TO CSA STANDARD W59.

3.4. ALL WELDS EXPOSED TO VIEW SHALL BE GROUND SMOOTH.

3.5. ANY ORGANIZATION UNDERTAKING TO WELD UNDER THIS CONTRACT SHALL BE CERTIFIED BY THE CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF DIVISION 1 OR DIVISION 2.1 OF W47.1.

3.6. UNLESS A REINFORCED MASONRY OR CONCRETE LINTEL IS SHOWN IN MASONRY WALLS OR MASONRY PARTITIONS, PROVIDE LOOSE STEEL LINTELS IN ACCORDANCE WITH REQUIREMENTS OF DOCUMENTS OVER ALL DOORWAYS, OTHER OPENINGS, AND RECESSES, INCLUDING THOSE FOR MECHANICAL OR ELECTRICAL SERVICES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE LOCATION, SIZE, AND NUMBER OF OPENINGS REQUIRED BY THE MECHANICAL AND ELECTRICAL CONSULTANT.

3.7. DO NOT SPLICE STRUCTURAL STEEL SECTIONS WITHOUT PRIOR APPROVAL OF THE CONSULTANT. ALL SPLICES SHALL DEVELOP THE FULL CAPACITY OF THE SECTION AND ARE TO BE TESTED BY NON DESTRUCTIVE METHODS, BY AN INDEPENDENT INSPECTION AND TESTING COMPANY, AT THE CONTRACTOR'S EXPENSE.

4. MASONRY

4.1. PROVIDE A MINIMUM LENGTH OF 200 mm (8") OF 100% SOLID MASONRY UNITS FOR BEARING OF STEEL, CONCRETE OR REINFORCED MASONRY LINTELS.

4.2. SUPPLY AND PLACE REINFORCEMENT AND CONCRETE FOR REINFORCED MASONRY LINTELS IN ACCORDANCE WITH TYPICAL DETAILS SHOWN.

5. ALTERATIONS AND/OR CONNECTIONS TO EXISTING STRUCTURE

5.1. INSPECT THE EXISTING BUILDING AND BECOME THOROUGHLY FAMILIAR WITH THE EXISTING CONDITIONS.

5.2. PRIOR TO FABRICATION OF STRUCTURAL STEEL, OPEN UP ALL AREAS WHERE CONNECTIONS ARE TO BE MADE TO EXISTING WORK AND TAKE FIELD MEASUREMENTS. MODIFY METHODS FOR CONNECTING TO SUIT SITE CONDITIONS FOUND AND TO THE APPROVAL OF THE CONSULTANT. CARRY OUT LOCAL REPAIRS TO THE EXISTING WORK AS NECESSARY AND AS DIRECTED BY THE CONSULTANT.

5.3. SHORE EXISTING WORK AS REQUIRED UNTIL ALL NEW WORK HAS BEEN COMPLETED AND REVIEWED BY THE CONSULTANT.

5.4. MAKE GOOD THE EXISTING WORK.

6. CUTTING AND CORING OF EXISTING STRUCTURE

6.1. FOR ANY OPENINGS WHICH ARE TO BE SAW-CUT INTO THE EXISTING STRUCTURE, PRE-DRILL THE CORNERS USING A 100 mm (4") Ø CORE DRILL. DO NOT OVER-CUT CORNERS OF OPENING.

6.2. ALL PRICES FOR CUTTING / CORING ARE TO INCLUDE ANY COSTS ASSOCIATED WITH X-RAYING, SCANNING, ETC.

E. QUALITY CONTROL

1. GENERAL

1.1. IMPLEMENT A SYSTEM OF QUALITY CONTROL TO ENSURE THAT THE MINIMUM STANDARDS SPECIFIED HEREIN ARE ATTAINED.

1.2. BRING TO THE ATTENTION OF THE CONSULTANT ANY DEFECTS IN THE WORK OR DEPARTURES FROM THE CONTRACT DOCUMENTS, WHICH MAY OCCUR DURING CONSTRUCTION. THE CONSULTANT WILL DECIDE UPON CORRECTIVE ACTION AND GIVE RECOMMENDATIONS IN WRITING.

1.3. THE CONSULTANT'S GENERAL REVIEW DURING CONSTRUCTION AND INSPECTION AND TESTING BY INDEPENDENT INSPECTION AND TESTING AGENCIES REPORTING TO THE CONSULTANT ARE BOTH UNDERTAKEN TO INFORM THE OWNER / CLIENT OF THE CONTRACTOR'S PERFORMANCE AND SHALL IN NO WAY AUGMENT THE CONTRACTOR'S QUALITY CONTROL OR RELIEVE THE CONTRACTOR OF CONTRACTUAL RESPONSIBILITY.

2. NOTIFICATION

2.1. PRIOR TO COMMENCING SIGNIFICANT SEGMENTS OF THE WORK, GIVE THE CONSULTANT AND INDEPENDENT INSPECTION AND TESTING COMPANIES APPROPRIATE NOTIFICATION (MINIMUM 24 HOURS) SO AS TO AFFORD THEM REASONABLE OPPORTUNITY TO REVIEW THE WORK. FAILURE TO MEET THIS REQUIREMENT MAY BE CAUSE FOR THE CONSULTANT TO CLASSIFY THE WORK AS DEFECTIVE.

3. DEFECTIVE MATERIALS AND WORK

3.1. WHERE EVIDENCE EXISTS THAT DEFECTIVE WORK HAS OCCURRED OR THAT WORK HAS BEEN CARRIED OUT INCORPORATING DEFECTIVE MATERIALS, THE CONSULTANT MAY HAVE TESTS, INSPECTIONS OR SURVEYS PERFORMED, ANALYTICAL CALCULATIONS OF STRUCTURAL STRENGTH MADE, AND THE LIKE, IN ORDER TO HELP DETERMINE WHETHER THE WORK MUST BE CORRECTED OR REPLACED. TESTS, INSPECTIONS OR SURVEYS, OR CALCULATIONS CARRIED OUT UNDER THESE CIRCUMSTANCES WILL BE MADE AT THE CONTRACTOR'S EXPENSE, REGARDLESS OF THEIR RESULTS, WHICH MAY BE SUCH THAT, IN THE CONSULTANT'S OPINION, THE WORK MAY BE ACCEPTABLE.

3.2. ALL TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE, EXCEPT WHERE THIS WOULD, IN THE CONSULTANT'S OPINION, CAUSE UNDUE DELAY OR GIVE RESULTS NOT REPRESENTATIVE OF THE REJECTED MATERIAL IN PLACE. IN THIS CASE, THE TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARDS GIVEN BY THE CONSULTANT.

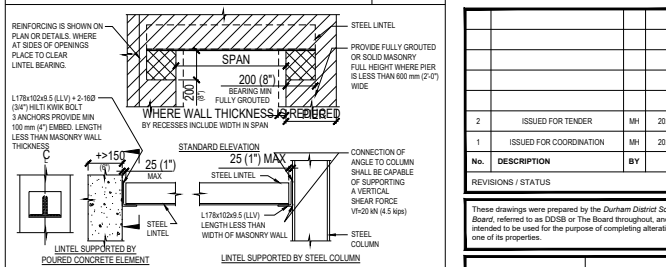
3.3. MATERIALS OR WORK, WHICH FAIL TO MEET SPECIFIED REQUIREMENTS, MAY BE REJECTED BY THE CONSULTANT WHENEVER FOUND AT ANY TIME PRIOR TO FINAL ACCEPTANCE OF THE WORK REGARDLESS OF PREVIOUS INSPECTION. IF REJECTED, DEFECTIVE MATERIALS OR WORK SHALL BE PROMPTLY REMOVED AND REPLACED OR REPAIRED TO THE SATISFACTION OF THE CONSULTANT, AT NO EXPENSE TO THE OWNER.

LIST OF STRUCTURAL DRAWINGS

STRUCTURAL ABBREVIATIONS — TD-G01

A BOLT	ANCHOR BOLT	fc	28 DAYS CONCRETE COMPRESSIVE STRENGTH	OF	OUTSIDE FACE		
ADJ	ADJUSTABLE	FDN	FOUNDATION	OPEN	OPENING		
AFF	ABOVE FINISHED FLOOR	FF	FAR FACE	OWSJ	OPEN WEB STEEL JOIST		
AIFB	ASPHALT IMPREGNATED FIBREBOARD	FIN	FINISHED	Pf	AXIAL FORCE (FACTORED)		
ALT	ALTERNATE	FL	FLOOR	PC	PRECAST		
ARCH	ARCHITECTURAL	ft	FOOT, FEET	PL	PLATE		
ASL	ADDITIONAL ACCUMULATED SNOW LOAD	FTG	FOOTING	PLF	POUNDS PER LINEAR FOOT		
@	AT	Fy	YIELD STRENGTH	PROJ	PROJECTION		
B. BOTT	BOTTOM	GA	GAUGE	PSF	POUNDS PER SQUARE FOOT		
B/B	BACK TO BACK	GALV	GALVANIZED	PT	PRESSURE TREATED		
BEW	BOTTOM EACH WAY	GEN	GENERAL	RD	ROOF DRAIN		
BH	BOREHOLE	HEF	HORIZONTAL EACH FACE	Rf	REACTION (FACTORED)		
BLL	BOTTOM LOWER LAYER	HF	HORIZONTAL FORCE (FACTORED)	RAD	RADIUS		
BLDG	BUILDING	HH	HOOK EACH END	REINF	REINFORCED, REINFORCEMENT		
BM	BEAM	HIF	HORIZONTAL INSIDE FACE	REF	REFERENCE		
BPL	BEARING/BASE PLATE	HOF	HORIZONTAL OUTSIDE FACE	RE	RIGHT END		
BRDG	BRIDGING	H, HORZ	HORIZONTAL	REQ'D	REQUIRED		
BUL	BOTTOM UPPER LAYER	HSC	HORIZONTALLY SLOTTED CONNECTION	REV	REVISION, REVISED		
c	CAMBER	HSS	HOLLOW STEEL SECTION	R/W	REINFORCED WITH		
C	EPOXY COATED	IF	INSIDE FACE	SAD	SEE ARCHITECTURAL DRAWINGS		
c/c, o/c	CENTRE TO CENTRE	IN	INCH(ES)	SDF	STEP DOWN FOOTING		
CA	COLUMN ABOVE	INT	INTERIOR	SECT	SECTION		
CB	COLUMN BELOW	JT	JOINT	SIM	SIMILAR		
CANT	CANTILEVER	K	KIP, 1000 LBS	SL	SLAB		
Cf	COMPRESSIVE FORCE (FACTORED)	K-ft	KIP FEET	SOG	SLAB ON GRADE		
CJ	CONTROL JOINT	KLF	KIPS PER LINEAR FOOT	SPDD	STANDARD PROCTOR DRY DENSITY		
℄	CENTRELINE	kN	KILONEWTON	ST	STRAIGHT		
COL	COLUMN	kN-m	KILONEWTON METRE	STIFF	STIFFENER		
COMP	COMPOSITE	kN/m	KILONEWTON PER METRE	STIR	STIRRUP		
CONC	CONCRETE	kPa	KILOPASCAL	STRUCT	STRUCTURAL		
CONT	CONTINUOUS	KSF	KIPS PER SQUARED FOOT	STD	STANDARD		
C/W	COMPLETE WITH	KSI	KIPS PER SQUARED INCH	SQ	SQUARE		
DEMO	DEMOLITION	L	SINGLE ANGLE	T	TOP		
DET	DETAIL	LE	LEFT END	TF	TENSILE FORCE (FACTORED)		
DIA, Ø	DIAMETER	LG	LONG	TEMP	TEMPORARY, TEMPERATURE		
DIAG	DIAGONAL	LL	LIVE LOAD, LOWER LAYER	TEW	TOP EACH WAY		
DIM	DIMENSION	LLH	LONG LEG HORIZONTAL	TJ	TIE JOIST		
DL	DEAD LOAD	LLV	LONG LEG VERTICAL	TLL	TOP LOWER LAYER		
DP	DEEP	m	METRE	TMf	TORSIONAL MOMENT (FACTORED)		
DWG(S)	DRAWING(S)	MC ▶	MOMENT CONNECTION	TOD	TOP OF DECK		
DWL(S)	DOWEL(S)		(FULL MOMENT UNLESS NOTED)	TOS	TOP OF STEEL/SLAB		
DN	DOWN	MECH	MECHANICAL	TYP	TYPICAL		
EA	EACH	Mf	MOMENT (FACTORED)	UL	UPPER LAYER		
EE	EACH END	ML	MIDDLE LAYER	U/N	UNLESS NOTED OTHERWISE		
EF	EACH FACE	mm	MILLIMETRE	U/S	UNDERSIDE		
ELEC	ELECTRICAL	MPa	MEGAPASCAL	V, VERT	VERTICAL		
EL	ELEVATION	Mxf	BENDING MOMENT ABOUT x-x AXIS (FACTORED)	Vf	VERTICAL SHEAR FORCE (FACTORED)		
ELEV	ELEVATOR	Myf	BENDING MOMENT ABOUT y-y AXIS (FACTORED)	VBF	VERTICAL BRACED FRAME		
EMBED	EMBEDMENT			VEF	VERTICAL EACH FACE		
EQ	EQUAL			VIF	VERTICAL INSIDE FACE		
ES	EACH SIDE	NF	NEAR FACE	VOF	VERTICAL OUTSIDE FACE		
EX, EXIST	EXISTING	NIC	NOT IN CONTRACT	VSC	VERTICALLY SLOTTED CONNECTION		
EJ, EXP JT	EXPANSION JOINT	N-S	NORTH-SOUTH	W	WIDE FLANGE BEAM		
E-W	EAST WEST	NTS	NOT TO SCALE	WT	WEIGHT, STRUCTURAL TEE		
EW	EACH WAY			WWF	WELDED WIRE FABRIC OR WELDED WIDE FLANGE		
EXT	EXTERIOR			W.P.	WORKING POINT		

STEEL LINTELS FOR NON-LOAD BEARING MASONRY WALLS — TD-S01



CLEAR SPAN	WALL THICKNESS: 90 (3 1/2") VENEER	140 (5 1/2")	190 (7 1/2")	240 (9 1/2")	290 (11 1/2")
UP TO 1200 (4'-0")	L89x89x6.4	2-L64x64x6.4	2-L76x89x6.4 (LLH)	L76x102x6.4 + L76x127x6.4 (LLH)	3-L76x89x6.4 (LLH)
1200 (4'-0") TO 1800 (6'-0")	L127x89x6.4 (LLV)	2-L89x64x6.4 (LLV)	2-L89x89x6.4	L102x102x6.4 + L76x127x6.4 (LLH)	3-L89x89x6.4
1800 (6'-0") TO 2400 (8'-0")	L127x89x6.4 (LLV)	2-L89x64x7.9 (LLV)	2-L102x89x7.9 (LLV)	L102x102x9.5 + L89x127x7.9 (LLH)	3-L102x89x6.4 (LLV)
2400 (8'-0") TO 3000 (10'-0")	L127x89x9.5 (LLV)	2-L89x64x9.5 (LLV)	2-L152x89x7.9 (LLV)	L152x102x7.9 (LLV) + L127x127x7.9	3-L127x89x6.4 (LLV)
DETAL	L	JL	JL	JL	JLL

NOTES:

1. CONNECT BACK TO BACK DOUBLE ANGLE LINTELS USING 16 mm (5/8") Ø BOLTS AT 450 mm (18") c/c MAX OR BY WELDING AT TOP AND BOTTOM USING 6 mm (1/4") WELDS x 50 mm (2") LONG AT 450 mm (18") c/c MAX. FIRST BOLT OR WELD TO BE A MAX OF 75 mm (3") FROM END OF LINTEL
2. FULLY PACK LINTEL ENDS WITH STEEL SHIMS TO ENSURE EVEN BEARING.
3. LINTELS AS COVERED UNDER THIS DETAIL ARE NOT NECESSARILY SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS AND SIZES.

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LICENSED PROFESSIONAL ENGINEER
2025/02/28
M. G. C. HAYMAN
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PROVINCE OF ONTARIO

No.	DESCRIPTION	BY	DATE
2	ISSUED FOR TENDER	MH	2025/02/28
1	ISSUED FOR COORDINATION	MH	2025/02/21

REVISIONS / STATUS

PROJECT:

VAUGHAN WILLARD KINDERGARTEN, SCIENCE, & MUSIC ALTERATIONS

Project No: T25-??

Scale:	AS NOTED
Drawn by:	MH
Checked by:	MH
Address:	1911 Dixie Rd N, Pickering, ON L1V 1V4

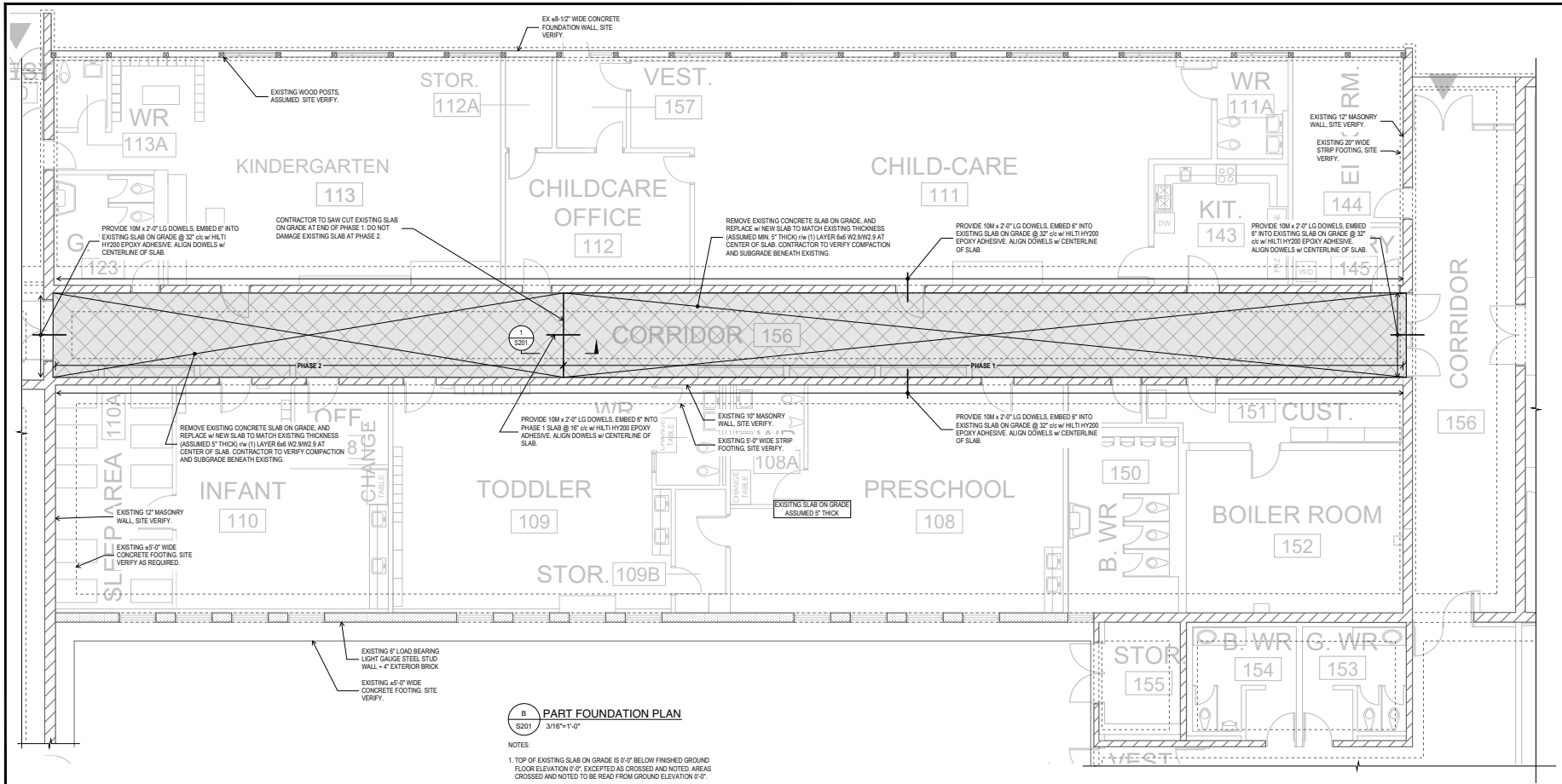
TITLE:

PART FOUNDATION PLAN, PART ROOF FRAMING PLAN

DDSB

DRAWING No:

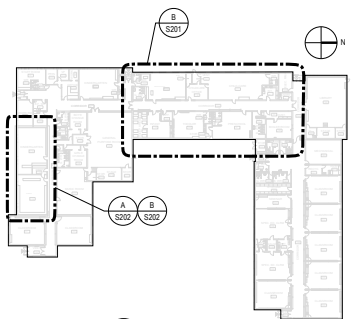
S101



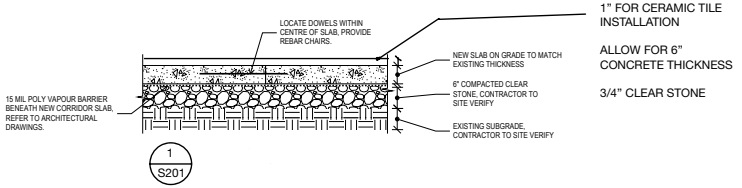
B S201 PART FOUNDATION PLAN
3/16"=1'-0"

NOTES:

1. TOP OF EXISTING SLAB ON GRADE IS 0'-0" BELOW FINISHED GROUND FLOOR ELEVATION 0'-0", EXCEPTED AS CROSSED AND NOTED. AREAS CROSSED AND NOTED TO BE READ FROM GROUND ELEVATION 0'-0".
2. MAKE GOOD ALL EXTERIOR AND INTERIOR FINISHES AS PER OWNER REQUIREMENTS.
3. CONTRACTOR TO MAKE GOOD ALL CONCRETE SAW CUTS AS PER OWNER REQUIREMENTS.
4. CONTRACTOR TO SITE VERIFY THICKNESS OF EXISTING SLAB ON GRADE. ASSUMED THICKNESS IS 5".
5. CONCRETE TO BE A HIGH-EARLY STRENGTH CONCRETE MIX.



A S201 KEY PLAN
N.T.S.



1 S201
3/4" = 1'-0"

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KEY PLAN & PART FOUNDATION PLAN

DDSB

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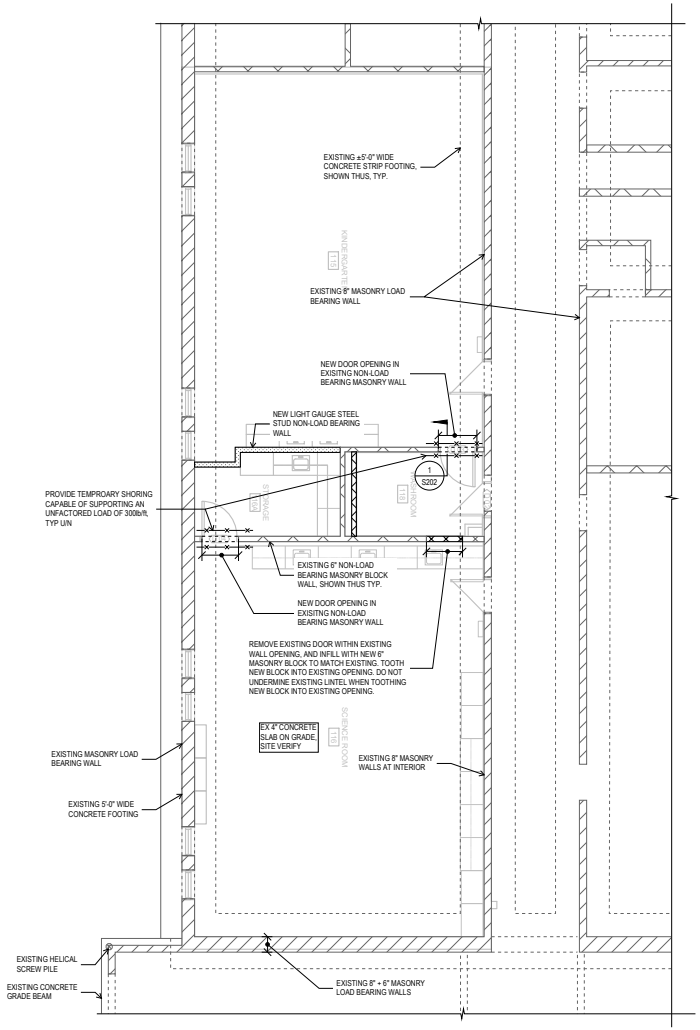
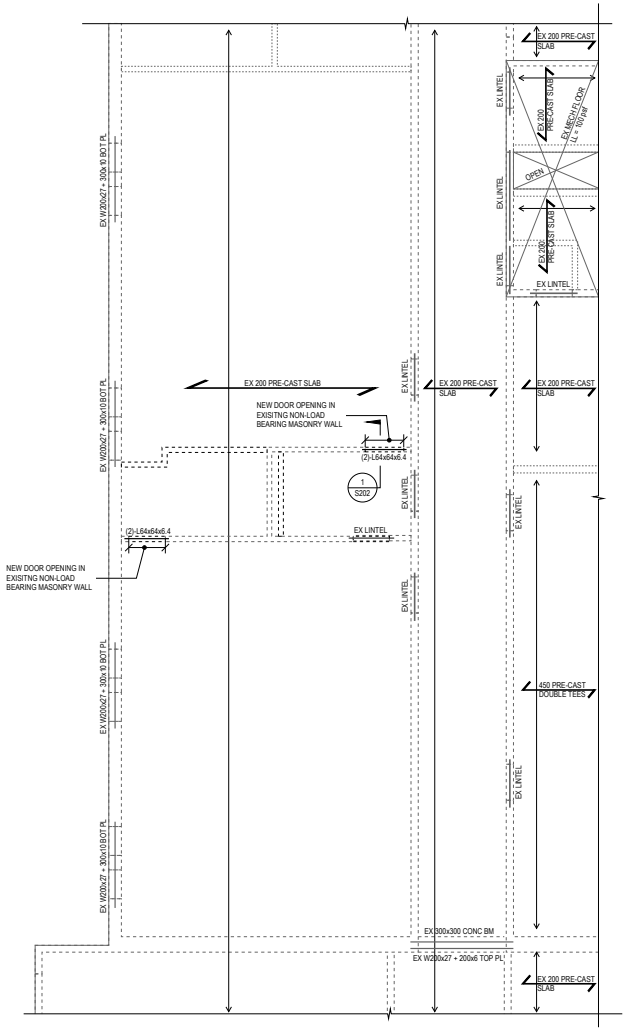


A S202 PART FOUNDATION PLAN
3/16"=1'-0"

NOTES:

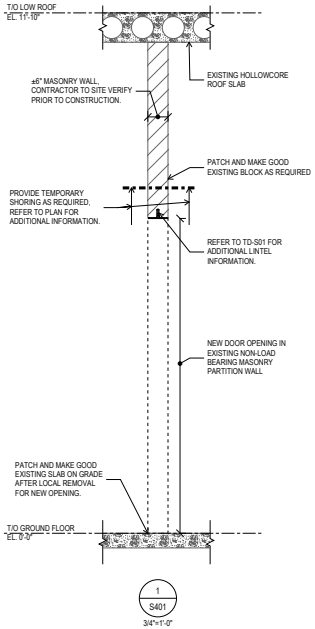
1. TOP OF EXISTING SLAB ON GRADE IS 0'-0" BELOW FINISHED GROUND FLOOR ELEVATION 0'-0", EXCEPTED AS CROSSED AND NOTED. AREAS CROSSED AND NOTED TO BE READ FROM GROUND ELEVATION 0'-0".
2. MAKE GOOD ALL EXTERIOR AND INTERIOR FINISHES AS PER OWNER REQUIREMENTS.
3. CONTRACTOR TO MAKE GOOD ALL BLOCK SAW CUTS AS PER OWNER REQUIREMENTS.
4. REFER TO OWNERS DRAWINGS FOR ELEVATION OF ALL NEW LINTELS AT OPENINGS THROUGH NON-LOAD BEARING MASONRY PARTITION WALLS.
5. WHERE NEW MASONRY INTERSECTS EXISTING MASONRY, PROVIDE DUR-O-WAL DA2200 JOINT STABILIZER ANCHORS @ 400 c/c VERTICALLY.
6. FULLY INFILL ALL ABANDONED DUCT OPENINGS. INFILL USING MASONRY BLOCK TO MATCH EXISTING. PACK TIGHT TO U/S EXISTING LINTELS. CONTRACTOR TO CONFIRM EXACT LOCATION AND NUMBER OF ABANDONED OPENINGS.



B S202 PART ROOF FRAMING PLAN
3/16"=1'-0"

NOTES:

1. TOP OF PRE-CAST CONCRETE PANELS ARE 0" BELOW LOW ROOF ELEVATION +11'-10", EXCEPT AS CROSSED AND NOTED ON PLAN. AREAS CROSSED AND NOTED TO BE READ FROM ELEVATION 11'-10".
2. READ DRAWING IN COMBINATION WITH GENERAL NOTES AND TYPICAL DETAILS.
3. EXISTING STRUCTURE HAS BEEN CHECKED FOR NEW LOADS (INCL. WIND LOADS) AND WAS FOUND TO BE ACCEPTABLE.
4. SNOW LOADS ARE: 25.1 psf + ASL
   MULTIPLIED BY HIGH IMPORTANCE (I_{LL} = 1.15, I_{SL} = 0.9)
5. LIVE LOADS ARE:
   MECHANICAL ROOM 75 psf
6. SUPERIMPOSED DEAD LOADS FOR THE MECHANICAL ROOM ARE:
   CONC TOPPING 25 psf
   M&E 4 psf
7. SUPERIMPOSED DEAD LOADS FOR THE LOW ROOF ARE:
   ROOFING 8 psf
   CEILING FINISH 6 psf
   M&E 6 psf
8. REMOVE EXISTING CEILING FINISHES, MECHANICAL SERVICES AND THE LIKE TO COMPLETE THE STRUCTURAL WORK. PATCH AND MAKE GOOD.



LICENSED PROFESSIONAL ENGINEER
2025/02/28
M. G. C. HAYMAN
10/20105/02
PROVINCE OF ONTARIO

No.	DESCRIPTION	BY	DATE
2	ISSUED FOR TENDER	MH	2025/02/28
1	ISSUED FOR COORDINATION	MH	2025/02/21

REVISIONS / STATUS

These drawings were prepared by the *Durham District School Board*, referred to as DDSB or The Board throughout, and are intended to be used for the purpose of completing alterations on one of its properties.

PROJECT:
VAUGHAN WILLARD KINDERGARTEN, SCIENCE, & MUSIC ALTERATIONS
Project No: T25-??

Scale:	AS NOTED
Drawn by:	MH
Checked by:	MH
Address:	1911 Dixie Rd N, Pickering, ON L1V 1V4

TITLE:
PART FOUNDATION PLAN, PART LOW ROOF FRAMING PLAN, AND SECTIONS AND DETAILS

DDSB

DRAWING No:
S202