STRUCTURAL DRAWING LIST

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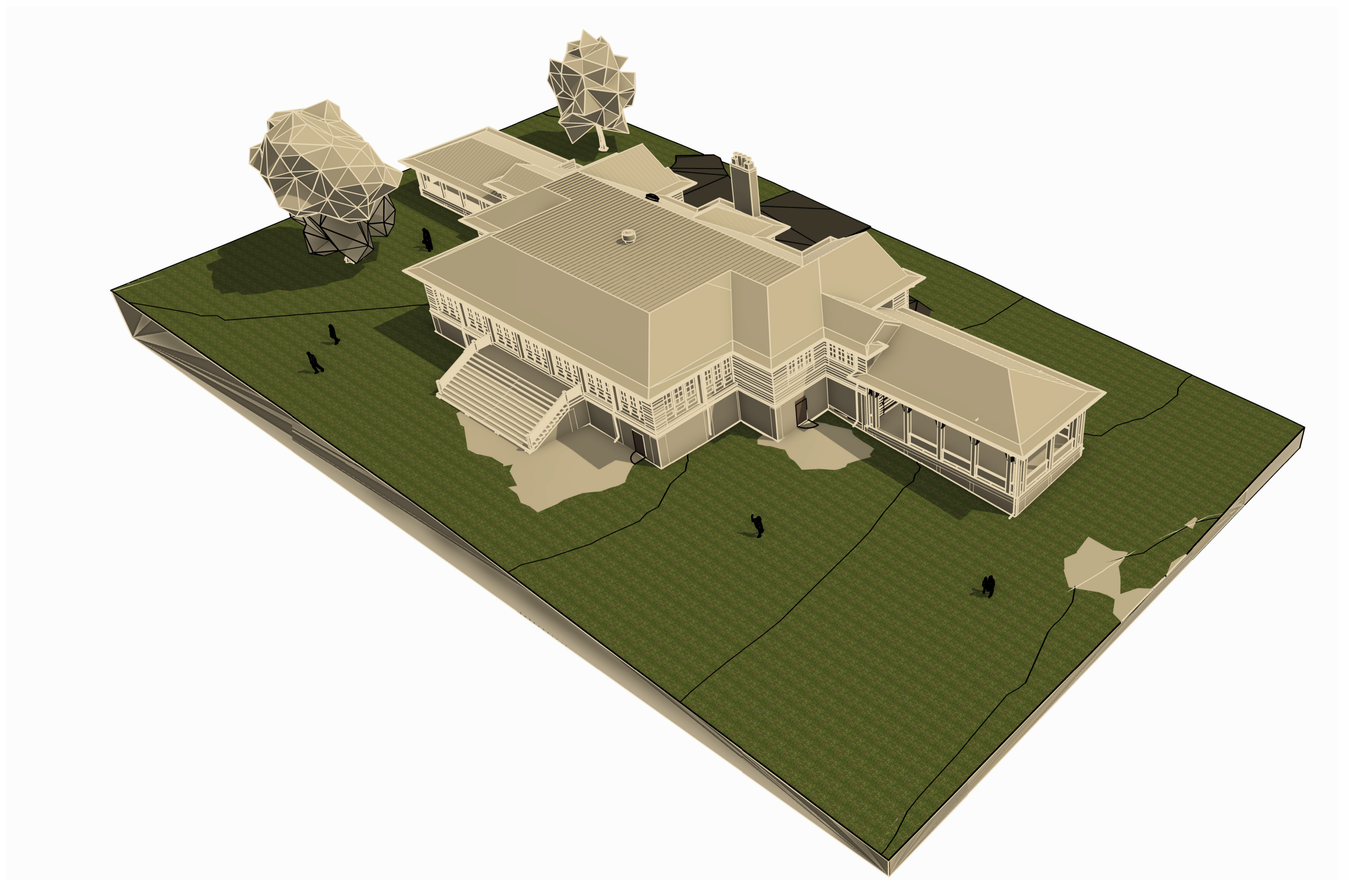

Project **HIGH PARK NATURE AND VISITOR'S CENTER**
Issue **ISSUED FOR TENDER**
Issue Date **2025/02/25**

Owner
Project Address **375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3**
Project Number **22-142**

MANTECON PARTNERS

STRUCTURAL MECHANICAL PROCESS ELECTRICAL CIVIL
ENGINEERS AND PROJECT MANAGERS

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GENERAL NOTES

1. THE GENERAL NOTES MUST BE READ IN CONJUNCTION WITH THE DESIGN DRAWINGS AND SPECIFICATIONS OF ENGINEERING AND ARCHITECTURAL DISIPLINES WHICH FORM PART OF THIS CONTRACT. THIS INCLUDES DRAWING SPECIFICATIONS AND SKETCKES. SHOULD THERE BE CONTRADICTORY INFORMATION BETWEEN DRAWINGS, SKETCHES AND SPECIFICATIONS, THE ONE WHICH IS MOST STRINGENT TAKES PRECEDENCE.
2. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF OPENINGS, TRENCHES, PITS, EQUIPMENT, SLEEVES, DEPRESSIONS, GROOVES AND CHAMFERS NOT INDICATED ON STRUCTURAL DRAWINGS.
3. UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS, NO PROVISION HAS BEEN MADE IN THE DESGIN FOR CONDITIONS OCCURING DURING CONSTRUCTION. THE CONTRACTOR IS TO PROVIDE ALL NECESSARY BRACING AND SHORING REQUIRED FOR STRESSES AND INSTABILITY OCCURRING FROM ANY CAUSE DURING CONSTRUCTION. THE CONTRACTOR SHALL ACCEPT FULL RESPONSIBILITY FOR ALL SUCH MEASURES. IT SHALL ALSO BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL NECESSARY BRACING, SHORING, SHEET PILING OR OTHER TEMPORARY SUPPORTS TO SAFEGUARD ALL EXISTING OR ADJACENT STRUCTURES AFFECTED BY THE WORK.
4. ALL CONNECTIONS CONNECTED TO EXISTING STRUCTURE ARE TO BE SITE VERIFIED.
5. REVIEW OF SHOP DRAWINGS BY STRUCTURAL CONSULTANT IS ONLY TO ASSESS THAT SUBMITTED SHOP DRAWINGS REFLECT THE INTENT OF THE STRUCTURAL DESIGN.
6. REVIEW BY THE STRUCTURAL CONSULTANT SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FORSEEN THAT THE WORK IS COMPLETE, ACCURATE AND IN CONFORMITY WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
7. TYPICAL DETAILS SHALL BE USED WHERE SPECIFIC DETAILS ARE NOT SHOWN ON THE DRAWINGS.
8. ALL WORK REQUIRED, INCLUDING ANY DEMOLITION, SHALL BE CARRIED OUT IN A MANNER THAT WILL NOT DAMAGE THE EXISTING SITE OR STRUCTURE. ANY DAMAGE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.
9. ALL DESIGN, DETAILING, CONSTRUCTION, EXCAVATION AND SHORING, MUST CONFORM TO THE PRESENT ONTARIO BUILDING CODE, OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS LATEST EDITION. ALL ASSOCIATED COST WITH THE DESIGN, SUPPLY AND INSTALLATION OF TEMPORARY SHORING IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. GENERAL CONTRACTOR TO PROVIDE STAMPED, ENGINEERED SHORING DRAWINGS.
10. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE WORK OF ALL SUBCONTRACTORS.
11. THE GENERAL CONTRACTOR MUST REVIEW ALL DIMENSIONS PRIOR TO THE COMMENCEMENT OF ALL WORK AND MUST REPORT ALL DISCREPANCIES TO THE ENGINEER/ARCHITECT.
12. STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL AND ELECTRICAL DRAWINGS.
13. PROVIDE STAMPED STRUCTURAL SHOP DRAWINGS AS NOTED IN THE FOLLOWING TABLE.

ITEMS	REQ'D SUBMITTAL	ENGINEER'S STAMP REQ'D	NOTES
REBAR SHOP DWGS.	YES	NO	
CONC. MIX DESIGNS	YES	NO	
STRUCT. STEEL SHOP DWGS.	YES	YES	
STUD WALL SHOP DWGS.	YES	YES	

14. PROJECTS WHICH INCLUDE ANY DEMOLITION AND OR RENOVATION WORK, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND EXISTING CONSTRUCTION. SHOULD A DISCREPANCY ON EITHER BE FOUND, REPORT FINDINGS TO ENGINEER/ARCHITECT.
15. ALL DETAILS SHOWN ARE SPECIFIC TO THE PROJECT. WHERE A LOCATION IS NOT SPECIFIED FOR A DETAIL, DETAILS IN THE DRAWINGS INCLUDING TYPICAL DETAILS WHICH CLOSELY RESEMBLES THE WORK, WILL APPLY.
16. ALL CODES AND REGULATIONS QUOTED ARE TO BE THE LATEST EDITION.

SLAB ON GRADE NOTES

1. TOP OF FINISHED CONCRETE SLAB ON GRADE ELEVATION AS NOTED ON PLAN.
2. SLAB ON GRADE SHALL BE 125mm THICK UNLESS NOTED OTHERWISE C/W 152x152x10\10 WELDED WIRE MESH @ MID-DEPTH. SLAB SHALL BEAR DIRECTLY ON A MINIMUM OF 200mm WELL COMPACTED 19mm CLEAR STONE.
3. SEE PLAN FOR SAWCUT/CONTROL JOINTS LOCATIONS.CO-ORDINATE LOCATION OF FLOOR CONTROL JOINTS AND/OR CONSTRUCTION JOINTS WITH ARCHITECTURAL FLOOR FINISHES.
4. SEE ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, WALL THICKNESS FLOOR SLOPES AND FLOOR FINISHES NOT SHOWN.
5. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION AND OF ALL PITS,INSERTS,DRAINS AND HOUSEKEEPING PADS.
6. ALL ISOLATION JOINTS AROUND COLUMNS AND FLOOR DRAINS ARE TO BE FORMED NOT SAWCUT.
7. PROVIDE SLAB THICKENING/ISOLATED FOOTING BELOW ALL INTERIOR CONCRETE BLOCK PARTITION WALLS UNLESS NOTED OTHERWISE.
8. DEPRESS AND MAINTAIN SPECIFIED SLAB ON GRADE THICKNESS AT MAT SINKAGES AND OTHER FLOOR DEPRESSIONS. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS.

STRUCTURAL STEEL

1. STRUCTURAL STEEL DESIGN,FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF:
   C.S.A. S16.1: LIMIT STATES DESIGN OF STEEL STRUCTURES,
   C.S.A. G40-20: GENERAL REQUIREMENTS FOR ROLLED OR WELDED STRUCTURAL QUALITY STEELS
   C.S.A. G40-21: STRUCTURAL QUALITY STEELS
   C.S.A. W59: WELDED STEEL CONSTRUCTION
   C.S.A. S136: COLD FORMED STEEL STRUCTURAL MEMBERS
2. STRUCTURAL STEEL SHALL CONFORM TO G40.21 GRADE 350W FOR W SHAPES AND CHANNELS, AND GRADE 300W FOR PLATES, ANGLES. SQUARE/RECTANGULAR HSS (HOLLOW STRUCT. SECTIONS) SHALL BE GRADE 350W, CLASS C. ROUND HSS SHALL BE ASTM A500 GRADE C.
3. UNLESS NOTED ON DRAWINGS, ALL BOLTS SHALL CONFORM TO A325 HIGH STRENGTH BOLTS IN BEARING M20 DIAMETER MINIMUM.
4. THE DESIGN OF BEAM SHEAR CONNECTIONS SHALL BE THE GREATER OF 50% OF THE BEAM SHEAR OR THE BEAM REACTION CALCULATED USING THE DESIGN LOADS SHOWN ON THE DRAWINGS, OR THE DESIGN SHEAR SHOWN. USE A MINIMUM OF TWO BOLTS.
5. WELDED CONNECTIONS SHALL BE UNDERTAKEN ONLY BY CERTIFIED WELDERS APPROVED BY C.W.B. TO THE REQUIREMENTS OF W47.1 DIVISION 1 AND 2. WELDING SHOULD BE DONE IN ACCORDANCE WITH W59. USE WELDING ELECTRODES WITH LOW HYDROGEN E480XX (E70XX) OR APPROVED EQUAL.
6. SHOULD THE FABRICATOR ELECT TO USE AN ALTERNATE ELECTRODE, THE ALTERNATE ELECTRODE SHALL MEET THE INTENT OF THE CONNECTION DESIGN AND MUST BE CERTIFIED BY A LICENSED WELDING ENGINEER IN THE PROVINCE OF ONTARIO.THE COST OF THE CERTIFICATION MUST BE BOURN BY THE CONTRACTOR.
7. WHEN WELDING TO EXISTING STEEL OR FIELD WELDING NEW STEEL, THE LOCATION OF THE WELD MUST BE FREE OF PAINT AND PRIMER.
8. CONNECTIONS FOR BRACING MEMBERS MUST BE DESIGNED FOR THE FULL TENSILE STRENGTH OF THE MEMBER, UNLESS LOADS ARE OTHERWISE INDICATED ON THE DRAWINGS.
9. ALL EXTERIOR EXPOSED STEEL INCLUDING MISCELLANEOUS EMBEDDED PLATES SUPPORTING SHELF ANGLES AND SHELF ANGLES SHALL BE HOT DIPPED GALVANIZED.

WOOD AND WOOD JOISTS

1. ALL WOOD SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH CSA STANDARD CAN/CSA O86-01 ENGINEERING DESIGN IN WOOD AND THE WOOD DESIGN MANUAL, PUBLISHED BY THE CANADIAN WOOD COUNCIL.
2. ALL TRUSSES ARE TO BE PRE-ENGINEERED IN ACCORDANCE WITH CSA STANDARD CAN/CSA O86-01 ENGINEERING DESIGN IN WOOD. DESIGN SHALL CONSIDER DEAD LOADS AND LIVE LOADS INCLUDING, BUT NOT LIMITED TO, SNOW PILE-UP AND EQUIPMENT LOADS AS SHOWN ON DRAWINGS. CONTRACTOR SHALL SUBMIT FOR REVIEW FABRICATION DRAWINGS AND CALCULATIONS SHOWING DESIGN LOADS, MEMBER SIZES,BRACING AND CONNECTION DETAILS STAMPED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF ONTARIO.
3. JOISTS HANGERS SHALL BE MINIMUM 20 GAUGE GALVANIZED STEEL AND SHALL CONFORM TO THE INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS "ACCEPTANCE CRITERIA FOR JOIST HANGERS AND SIMILAR DEVICES"
4. TRUSS PLATES SHALL CONFORM TO THE CSA STANDARD S347-M80 "METHOD OF TEST FOR EVALUATION OF TRUSS PLATES USED IN LUMBER JOINTS"
5. NAILS AND SPIKES SHALL CONFORM TO THE CSA STANDARD B111-1974 "WIRE NAILS, SPIKES AND STAPLES"
6. SAWN TIMBER PRODUCTS SHALL CONFORM TO THE CSA STANDARD CAN/CSA-041-91 "SOFTWOOD LUMBER" AND GRADING OF TIMERS IN ACCORDANCE WITH THE NATIONAL LUMBER GRADES AUTHORITY "STANDARD GRADING RULES FOR CANADIAN LUMBER"
7. GLUED-LAMINATED TIMBER PRODUCTS SHALL CONFORM TO CSA STANDARD CAN/CSA-0122-M89 "STRUCTURAL GLUED-LAMINATED TIMBER"
8. STRUCTURAL COMPOSITE LUMBER (SCL) INCLUDING LAMINATED VENEER LUMBER (LVL) AND PARALLEL STRAND LUMBER (PSL) SHALL, BE FABRICATED AND ERECTED IN STRICT ACCORDANCE WITH MANUFACTURERS WRITTEN INSTRUCTIONS.
9. PLYWOOD SHEATHING SHALL CONFORM TO THE CSA STANDARD 0121-M1978 "DOUGLAS FIR PLYWOOD" AND 0151-M1978 "CANADIAN SOFTWOOD PLYWOOD"
10. WAFERBOARD AND ORIENTED STRAND BOARD SHALL CONFORM TO THE CSA STANDARD CAN3-0437.0/0437.1-M85 " WAFERBOARD AND STRANDBOARD"
11. ALL STEEL BEARING AND CONNECTOR PLATES SHALL CONFORM TO THE CSA STANDARD CAN/CSA-G40.21-M92 "WELDED STRUCTURAL QUALITY STEEL/STRUCTURAL QUALITY STEELS HAVING A YIELD STRENGTH OF 300 MPa
12. ALL BOLTS AND THREADED ROD CONNECTING WOOD MEMBERS SHALL CONFORM TO ASTM A307
13. ALL WOOD STUDS SHALL BE SPRUCE-PINE-FIR NO. 1 AND 2 GRADE OR BETTER
14. ALL WOOD JOISTS, NAILERS AND BLOCKING SHALL BE SPRUCE-PINE-FIR NO. 1 AND 2 GRADE OR BETTER
15. ALL BUILT-UP WOOD BEAMS AND COLUMNS SHALL BE SPRUCE-PINE-FIR NO. 1 AND 2 GRADE OR BETTER
16. FOR ALL WOOD CONSTRUCTION NOT DETAILED, FOLLOW THE ONTARIO BUILDING CODE 2012, SECTION 9.23 "WOOD FRAME CONSTRUCTION"

WOODEN NAILERS AND BLOCKING

17. ALL WOODEN NAILERS AND BLOCKING SHALL BE SAWN LUMBER SPF N01/NO2 TO CAN/CSA-086.1 SIZE SHALL BE AS SHOWN ON DRAWINGS
18. WOODEN BLOCKING SHALL BE LOCATED OVER THE NAILER IN BETWEEN JOISTS. THE SIZE OF BLOCKING SHALL BE AS SHOWN ON DRAWINGS. THE LENGTH OF BLOCKING SHALL BE AS LONG AS POSSIBLE BETWEEN JOIST (FINAL LENGTH TO BE COORDINATED WITH JOIST SUPPLIER). THE BLOCKING SHALL BE CONNECTED TO WOODEN NAILER WITH 65MM COMMON WIRE NAILS, 2 NAILS PER ROW, ROWS SPACED AT 250 CENTRES UNLESS OTHERWISE MENTIONED IN DRAWINGS.

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   C.S.A. G40-20: GENERAL REQUIREMENTS FOR ROLLED OR WELDED STRUCTURAL QUALITY STEELS
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6. SHOULD THE FABRICATOR ELECT TO USE AN ALTERNATE ELECTRODE, THE ALTERNATE ELECTRODE SHALL MEET THE INTENT OF THE CONNECTION DESIGN AND MUST BE CERTIFIED BY A LICENSED WELDING ENGINEER IN THE PROVINCE OF ONTARIO.THE COST OF THE CERTIFICATION MUST BE BOURN BY THE CONTRACTOR.
7. WHEN WELDING TO EXISTING STEEL OR FIELD WELDING NEW STEEL, THE LOCATION OF THE WELD MUST BE FREE OF PAINT AND PRIMER.
8. CONNECTIONS FOR BRACING MEMBERS MUST BE DESIGNED FOR THE FULL TENSILE STRENGTH OF THE MEMBER, UNLESS LOADS ARE OTHERWISE INDICATED ON THE DRAWINGS.
9. ALL EXTERIOR EXPOSED STEEL INCLUDING MISCELLANEOUS EMBEDDED PLATES SUPPORTING SHELF ANGLES AND SHELF ANGLES SHALL BE HOT DIPPED GALVANIZED.

SOILS AND FOUNDATIONS

1. ALL THE SPREAD FOOTINGS AND STRIP FOOTINGS TO BE CONSTRUCTED ON UNDISTURBED NATIVE SOIL OR ENGINEERED FILL CAPABLE OF RESISTING 150 kPa (3133 PSF) ACCRODING TO GEOTECHNNICAL INVESTIGATION REPORT BY ORBIT ENGINEERING LIMITED (PROJECT NO.OE231465DG, MARCH 31, 2023). THE GEOTECHNICAL CONSULTANT TO CONFIRM THE SOIL BEARING RESISTANCE BEFORE CONSTRUCTION.
2. THE GEOTECHNICAL REPORT PROVIDED IS A GUIDE ONLY. MANTECON PARTNERS INC. IS NOT RESPONSIBLE FOR ITS CONTENT, RECOMMENDATIONS, CORRECTNESS AND OMISSIONS. THE GENERAL CONTRACTOR SHOULD FAMILIARIZE HIMSELF WITH THE REPORT AND THE SITE.
3. FOR THE DURATION OF THE EXCAVATION, THE GEOTECHNICAL AND STRUCTURAL ENGINEERS MUST BE MADE AWARE OF ALL SOIL CONDITIONS FOUND WHICH ARE DIFFERENT THAN REPORTED IN THE GEOTECHNICAL REPORT.
4. FOUNDING ELEVATION, BACKFILL AND COMPACTION MUST BE APPROVED BY THE GEOTECHNICAL ENGINEER.
5. ALL FOOTINGS MUST BE FOUNDED AT THE ELEVATIONS SHOWN ON THE CONTRACT DOCUMENTS, UNLESS POORER SOIL CONDITIONS ARE ENCOUNTERED, WHERE THE GEOTECHNICAL ENGINEER WILL DETERMINE FOUNDING ELEVATIONS.
6. ALL EXTERIOR FOOTINGS AND FOOTINGS EXPOSED TO FREEZING MUST BE FOUNDED TO A MINIMUM FROST PROTECTION DEPTH OF 1.2M (4') BELOW FINISHED GRADE. REFER TO GRADING PLAN FOR FINISH EXTERIOR GRADE ELEVATIONS.
7. ALL SPREAD FOOTINGS, CONTINUOUS FOOTINGS AND DEEP FOUNDATIONS, WHICH INCLUDES BUT IS NOT LIMITED TO CAISSONS AND PILES, MUST BE CONSTRUCTED CONCENTRIC TO THE COLUMN AND/OR WALL WHICH THEY ARE SUPPORTING UNLESS OTHERWISE NOTED.
8. ALL EXCAVATIONS MUST BE CARRIED OUT IN CONFORMANCE TO THE GEOTECHNICAL REPORT AND OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS, LATEST EDITION GUIDELINES.
9. DO NOT EXCEED A MAXIMUM RISE TO RUN OF 7 TO 10 SLOPE BETWEEN ADJACENT FOOTINGS UNLESS DIRECTED IN WRITING BY THE GEOTECHNICAL ENGINEER.
10. BACKFILL MATERIAL AND COMPACTION SHOULD BE IN CONFORMANCE WITH GEOTECHNICAL REPORT.
11. PRIOR TO BACKFILLING, CONCRETE FLOOR OR STRUCTURAL STEEL FLOOR AND SLAB ON GRADE MUST BE IN PLACE TO PREVENT WALLS FROM COLLAPSE. THE CONCRETE MUST HAVE ACHIEVED A STRENGTH OF MINIMUM 75% OF ITS DESIGN STRENGTH.
12. IN WALLS WHERE THE CONTRACT DOCUMENTS CALL FOR WATER STOPS AT THE INTERFACE OF THE TOP OF FOOTING AND THE UNDERSIDE OF THE WALL, THE GENERAL CONTRACTOR MUST PROVIDE THE STRUCTURAL ENGINEER SKETCHES OF THE PROPOSED INSTALLATION FOR REVIEW. SIMILAR DIRECTION MUST BE FOLLOWED FOR WALL CONSTRUCTION JOINTS.

CONCRETE AND REINFORCING

1. CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION, TESTING AND STANDARD PRACTICES FOR CONCRETE SHALL BE IN ACCORDANCE WITH CSA STANDARD A23.1/A23.2 (LATEST EDITION).
2. CONCRETE DESIGN SHALL BE IN ACCORDANCE WITH THE DESIGN OF CONCRETE STRUCTURES CSA STANDARD A23.3 (LATEST EDITION).
3. SUPPLY AND PLACE CONCRETE IN ACCORDANCE TO TABLE 1

TABLE 1

	LOCATION	MIN. COMPRESSIVE STRENGTH (f'c) AT 28 DAYS MPa (PSI)	SLUMP mm (in)	EXPOSURE CLASS	AIR CONTENT (%)
FTGS	FND. WALL FOOTINGS	25 (3500)	80 ± 30 (3 ± 1)	N	0
WALLS	FND. WALLS,SHEAR WALLS, ABOVE GRADE WALLS RETAINING WALLS	35 (5077)	80 ± 30 (3 ± 1)	C-1	5-8
SLABS, BEAMS COLUMNS AND STAIRS	INTERIOR SLAB ON GRADE, AND CONC. SLAB ON DECK	25 (3500)	80 ± 30 (3 ± 1)	N	0
	INTERIOR SLAB, BEAMS, COLUMNS AND STAIRS	25 (3500)	80 ± 20 (3 ± 3/4)	N	0
OTHER	SIDEWALK/CURBS PAVING SLABS, EXTERIOR CONC. AND TOPPING	32 (4650)	40 ±20 (1 1/2 ± 3/4)	C-2	5-8
	HOUSEKEEPING PADS	25 (3500)	80 ± 30 (3 ± 1)	N	0
	NON-SHRINKABLE GROUT	30	AS PER MANUF. RECOMEN.	N	0
	LEAN MIX CONCRETE	8 (1000)	80 ± 30 (3 ± 1)	N	0

4. THE COMPRESSIVE STRENGTH OF THE CONCRETE IS BASED ON THE FOLLOWING CONDITIONS:
   a. TYPE GU NORMAL PORTLAND CEMENT UNLESS OTHERWISE NOTED OR APPROVED
   b. MAXIMUM SIZE OF AGGREGATE 20mm (3/4") WASHED IRREGULAR CUT CLEAR STONE
   c. SLUMP SHOWN ON THE TABLE IS SLUMP WITHOUT SLUMP AID ADMIXTURE. WHERE THE USE OF AN ADMIXTURE IS PREFERRED TO INCREASE THE SLUMP, THE SUPERPLASTICIZED CONCRETE SLUMP MUST REMAIN BELOW THE POINT AT WHICH SEGREGATION WILL OCCUR.
5. REINFORCEMENT SHALL CONFORM TO CSA G30.3,G30.5 AND G30.18 (LATEST EDITION) YIELD STRENGTH FOR CONCRETE AND MASONRY REINFORCEMENT, fy=400MPa YIELD STRENGTH FOR WELDED WIRE FABRIC fy=360MPa
6. WHEN COLUMNS AND WALLS ARE POURED INTEGRALLY USE THE HIGHER STRENGTH CONCRETE OF THE ELEMENT WHICH SPECIFIED IN TABLE 1.
7. MINIMUM CONCRETE COVER FOR REINFORCING, WHERE NOT SHOWN ON DESIGN DRAWINGS SHALL BE AS FOLLOWS:
   a. ALL STEEL NOT CAST IN FORMS PERMANENTLY AGAINST EARTH OR ROCK AND IN A NON-CORROSIVE ENVIRONMENT, COVER SHALL BE 75mm (3").
   b. ALL STEEL CAST IN FORMS SHALL FOLLOW TABLE 2 OR AS NOTED ON DRAWINGS.

TABLE 2

STRUCTURAL ELEMENT	COVER mm (in)	STRUCTURAL ELEMENT	COVER mm (in)
CONCRETE POURED IN FORMS BUT EXPOSED TO WEATHER OR EARTH		CONCRETE NOT EXPOSED TO WEATHER OR EARTH	
-BARS LARGER THAN 15M	50 (2")	-SLABS AND WALLS	25 (1")
-BARS 15M AND SMALLER	38 (1 1/2")	-BEAMS AND GIRDERS	38 (1 1/2")
		-COLUMNS MAIN STEEL	50 (2")
FTGS. & OTHER ELEMENTS POURED AGAINST EARTH	75 (3")		

8. THE GENERAL CONTRACTOR MUST COORDINATE THE INSTALLATION OF MECHANICAL AND ELECTRICAL OPENINGS AND SLEEVES. THEY MUST FOLLOW THE GUIDE LINES BELOW:
   a. NO SLEEVES SHALL BE PLACED VERTICALLY OR HORIZONTALLY THROUGH BEAMS UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
   b. NO OPENINGS SHALL BE MADE IN FLAT SLABS OR TWO WAY SLAB COLUMN STRIPS EXCEPT AS SHOWN ON THE DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.
   c. WHERE A CORE DRILL OR AN OPENING IS REQUIRED IN HARDENED CONCRETE THE GENERAL CONTRACTOR MUST SEEK THE APPROVAL OF THE STRUCTURAL ENGINEER.
   d. ELECTRICAL CONDUITS SHALL NOT PASS THROUGH COLUMNS AND ARE NOT TO RUN HORIZONTALLY IN WALLS.
   e. CONDUITS WITHIN SLABS MUST NOT HAVE A (OUTER) DIAMETER GREATER THAN ONE-QUARTER OF THE SLAB THICKNESS. SPACING BETWEEN CONDUITS MUST BE AT LEAST 3 TIMES THE OUTER DIAMETER (CLEAR SPACING). CONDUITS MUST BE PLACED WITHIN MIDDLE THIRD OF SLAB. CONDUITS SHALL BE LAID SUCH THAT ONLY SINGLE CROSS OVERS OCCUR WITHIN MAXIMUM 500mm OF ONE ANOTHER. ALL CONDUITS WITHIN SLAB ARE SUBJECT TO APPROVAL BY STRUCTURAL CONSULTANT.
9. REFER TO DESIGN DRAWINGS FOR TYPICAL DETAILS OF CONTROL JOINTS, EXPANSION JOINTS AND CONSTRUCTION JOINTS. UNLESS OTHERWISE NOTED ON THE DESIGN DRAWINGS, THE FOLLOWING MAXIMUM DISTANCE BETWEEN JOINTS MUST BE FOLLOWED:
   a. CONTROL JOINTS IN WALLS 6m (20') MAXIMUM
   b. MAXIMUM POUR LENGTH FOR SLAB ON GRADE IS 30m (100').
   c. ALL SAWCUTS MUST BE MADE WITHIN 24 HRS. FR0M PLACING OF CONCRETE. THE DEPTH OF THE SAWCUT MUST BE 1/3 THE DEPTH OF THE SLAB.

TOPPING

10. THE CONTRACTOR SHALL PROVIDE A SUITABLE TOP FINISH TO ACCEPT DIRECT APPLICATION OF FINISHED FLOORING/ROOFING AS PER ROOM FINISH SCHEDULE

TESTING AND INSPECTION

1. THE FOLLOWING ITEMS REQUIRE TESTING OR INSPECTION BY A CERTIFIED INDEPENDENT TESTING OR INSPECTION AGENCY UNLESS NOTED OTHERWISE. THE AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE ENGINEER FOR REVIEW.

ITEMS	REQ'D?	COMMENTS
SOIL BEARING CAPACITY	YES	BY SOILS ENGINEER
SOIL COMPACTION	YES	BY SOILS ENGINEER
REINF. STEEL PLACEMENT	YES	INSPECT FINAL PLACEMENT
CONC. COMPRESSIVE TESTS	YES	MIN. 2 SETS PER X m^3
CONC. SLUMP	YES	
STEEL CONNECTIONS	YES	INSPECT ALL CONNECTIONS
MORTAR CUBES	NO	
GROUT CUBES	NO	

* CONCRETE POURS IN WINTER MONTHS TO HAVE MIN. 2 SETS LAB CURED AND 2 SETS FIELD CURED.

STRUCTURAL ABBREVIATIONS

A.B.	ANCHOR BOLT	L.L.H.	LONG LEG HORIZONTAL
ALT.	ALTERNATE	L.L.V.	LONG LEG VERTICAL
ALUM.	ALUMINUM	LP.	LOW POINT
ANCH'S.	ANCHORS	LG.	LONG
APPROX.	APPROXIMATELY	L.S.H.	LONG SIDE HORIZONTAL
ARCH.	ARCHITECTURAL	L.S.V.	LONG SIDE VERTICA
B	BOTTOM	L.L.B.B.	LONG LEG BACK TO BACK
B/F	BOTTOM FACE	M.C.	MOMENT CONNECTION
B/FTG.	BOTTOM OF FOOTING	MAX.	MAXIMUM
B.A.	BASE PLATE	MECH.	MECHANICAL
BLK.	BLOCK	MET'L.	METAL
BM.	BEAM	MIN.	MINIMUM
BTM.	BOTTOM	MISC.	MISCELLANEOUS
BRG.	BEARING	m	METER
BT.PL.	BENT PLATE	mm	MILLIMETRE
B.L.L	BOTTOM LOWER LEVEL	MPa	MEGAPASCAL
B.U.L.	BOTTOM UPPER LEVEL	N.I.C.	NOT IN CONTRACT
C/W	COMPLETE WITH	N.T.S.	NOT TO SCALE
C/C	CENTRE TO CENTRE	No.	NUMBER
C.J.	CONTROL JOINT	NS/FS	NEAR SIDE/FAR SIDE
BT.	CENTRE LINE	O.A.E.	OR APPROVED EQUAL
CLG.	CEILING	O.C.	ON CENTRE
COL.	COLUMN	O.C.B.	ON CENTRE BOTTOM
CONC.	CONCRETE	O.D.	OUTSIDE DIAMETER
CONN.	CONNECTION	O.H.	OVER HEAD
CONST.	CONSTRUCTION	OWSJ	OPEN WEB STEEL JOIST
CONT.	CONTINUOUS	OPG.	OPENING
DEMO.	DEMOLITION	O.S.F.V.	OUTSIDE FACE VERTICAL
DET'L.	DETAIL	PART'N.	PARTITION
DIA.	DIAMETER	PL.	PLATE
DIM.	DIMENSION	R.C.	REINFORCED CONCRETE
DO	DIDO	R.D.	ROOF DRAIN
DP.	DEEP	R.O.	ROUGH OPENING
DWG.	DRAWING	REF.	REFERENCE
DWL.	DOWEL	REINF.	REINFORCED
E.F.	EACH FACE	REQ'D	REQUIRED
E.F.H.	EACH FACE HORIZ.	SECT.	SECTION
E.J.	EXPANSION JOINT	S.D.F.	STEP DOWN FOOTING
ELEC.	ELECTRICAL	S.L.H.	SHORT LEG HORIZONTAL
E.S.	EACH SIDE	S.L.V.	SORT LEG VERTICAL
E.W.	EACH WAY	S.L.B.B.	SHORT LEG BACK TO BACK
EA.	EACH.	S.O.G	SLAB ON GRADE
ELEV.	ELEVATION	S.P.D.D.	STANDARD PROCTOR DRY DENSITY
EQ.	EQUAL	S.S.	STAINLESS STEEL
EXIST.	EXISTING	STL.	STEEL
F.F.	FACE TO FACE	STIFF	STIFFENER
FIN.	FINISHED	STRUCT.	STRUCTURAL
FL.	FLOOR	T	TOP
FDN.	FOUNDATION	T/C	TOP OF CONCRETE
FTG.	FOOTING	T/F	TOP OF FOOTING
GA.	GAUGE	T/O	TOP OF
GALV.	GALVANIZED	T/S	TOP OF STEEL
GRD.	GRADE	T/WALL	TOP OF WALL
HORIZ.	HORIZONTAL	T.L.L.	TOP LOWER LEVEL
H.D.	HEAVY DUTY	TYP.	TYPICAL
H.D.G.	HOT DIPPED GALVANIZED	U/G	UNDERGROUND
H.E.F.	HORIZONTAL EACH FACE	U.N.O.	UNLESS NOTED OTHERWISE
H.P.	HIGH POINT	U/S	UNDERSIDE
HSS	HOLLOW STRUCT. SECTION	VERT.	VERTICAL
HT.	HEIGHT	V.E.F	VERTICAL EACH FACE
I.D.	INSIDE DIAMETER	V.I.F.	VERTICAL INSIDE FACE
INV. ELEV.	INVERT ELEVATION	V.O.F.	VERTICAL OUTSIDE FACE
I.S..V.	INSIDE FACE VERTICAL	V.S.C.	VERTICAL SLOTTED CONNECTION
kN.	KILONEWTON	W.P.	WORKING POINT
kN/m	KILONEWTON METERS	W.W.M.	WELDED WIRE MESH
kPa	KILOPASCAL	@	SPACED AT
L	ANGLE		

DESIGN LOADS

GRAVITY LOADS

1 DEAD LOADS

	EXISTING (kPa)	NEW (kPa)
LEVEL 1 WOOD FLOOR	0.5	0.65
LEVEL 1 DECK + CONC.	N/A	2.8
ATTIC (LIMITED ACCESS)	0.1	0.15
ATTIC (MECHANICAL)	N/A	0.4
ROOF (MAIN)	0.7	N/A

(DL OF WOOD FLOORS INCLUDE JOIST SELF WEIGHTS) + SELF WEIGHT OF NEW STRUCTURAL MEMBERS

SOLAR PANEL ALLOWANCE 0.5 kPa (INCLUDING FRAMING)
M+E+CEILING ALLOWANCE 0.3 kPa
SPRINKLER ALLOWANCE 0.2 kPa

2. LIVE LOADS

ATTIC (LIMITED ACCESS - NO STORAGE) 0.5 kPa
ATTIC LEVEL (NEW MECHANICAL SPACE) 2.4 kPa
WASHROOMS 2.4 kPa
OTHERS 4.8 kPa

3. SNOW LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE O.B.C. USING THE FOLLOWING CRITERIA:

TORONTO, ON
Ss = 0.9 kPa, Sr = 0.4 kPa
BASIC SNOW LOAD = 1.12 kPa

REFER TO PLANS FOR SNOW PILE UP CONDITIONS.
NO GAP IS ALLOWED BETWEEN FUTURE SOLAR PANELS AND ROOF

LATERAL LOADS

1. WIND LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE 0.B.C USING THE FOLLOWING CRITERIA:

TORONTO,ON
q10 = 0.34 kPa q50 = 0.44 kPa
NO GAP IS ALLOWED BETWEEN FUTURE SOLAR PANELS AND ROOF

2. SEISMIC LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE O.B.C. USING THE FOLLOWING CRITERIA; TORONTO, ON:

Sa (0.2) = 0.249 Sa (0.5) = 0.126 Sa (1.0) = 0.063
Sa (2.0) = 0.029 Sa (5.0) = 0.0071 Sa (10.0) = 0.0028
PGA = 0.16 PGV = 0.099

3. LATERAL EARTH PRESSURE HAS BEEN DETERMINED BASED ON THE FOLLOWING CRITERIA:

k(active) = 0.5
SOIL DENSITY = 20.5 kN/m3

4. VEHICLE GUARDRAILS HAVE BEEN DESIGNED FOR LOADING GIVEN IN 4.1.5.16 OF OBC.
5. HANDRAILS HAVE BEEN DESIGNED FOR LOADING GIVEN IN 2012 OBC 3.4.6.5(12)

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

Rev.#	Description	Date
4	ISSUED FOR TENDER	2025/02/25
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2	ISSUED FOR COSTING	2024/02/08
1	ISSUED FOR PERMIT	2024/01/31

Consultants

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15 Foundry Street, Dundas, ON, L9H 2V6
Phone: (905)648-0373 www.manteconpartners.com

Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
GENERAL NOTES

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:
S0.1

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

1 / S1.0 FLOOR PLAN - BASEMENT - PHASE 1

1 : 100

LINTEL SCHEDULE			
MARK	SIZE	DETAIL	NOTES
L1	2-L152x152x9.5 C/W 300mm (W) x 9.5mm (T) STL. PLATE	BRICK	REF. 1/S4.0 FOR DETAIL
L2	2-L102x102x6.4 C/W 300mm (W) x 6.4mm (T) STL. PLATE	BRICK	

COLUMN SCHEDULE

MARK	SIZE	BASEPLATE	ANCHOR BOLTS
C1	6"x6" SPF TYPE 1/2		
C2	4"x4" SPF TYPE 1/2		
C3	3 PLY 2"x10"		
C5	HS102x102x6.4	300 x 300 x19	4-Ø1/2" ANCHOR BOLTS
C5A	HS102x102x6.4 (A)		
C6	HANGER D220mm ROD		
EXIST. WOOD COL. 1	152mm x 152mm		

PIER SCHEDULE

Type Mark	SIZE	REINFORCEMENT	TIES	REMARKS
P1	400mm x 400mm	8-15M	2 SETS 10M@250mm TIE	

FOOTING SCHEDULE

MARK	SIZE	THICKNESS (mm)	REINFORCEMENT		ALLOWABLE SOIL BEARING PRESSURE	REMARKS
			BOTTOM	TOP		
F1	800x800mm	250	3-15M E.W.			HOOKED END
F2	REF. PLAN DETAIL	250	REF. PLAN DETAIL	REF. PLAN DETAIL		

STRUCTURAL FLOOR SCHEDULE

Type Mark	Description
A	125mm S.O.G. C/W 152x152x10\.10 WELDED WIRE MESH @MID-DEPTH. SLAB SHALL BEAR DIRECTLY ON COMPACTED GRANULAR. REFER TO GEO-TECNICAL REPORT FOR SPECIAL REQUIREMENT
B	19mm PLYWOOD GLUED AND SCREWED
C	19mm PLYWOOD TOP & BOTTOM GLUED AND SCREWED C/W 2"x4" @ 400mm O.C.

Rev #	Description	Date
4	ISSUED FOR TENDER	2025/02/25
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Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:

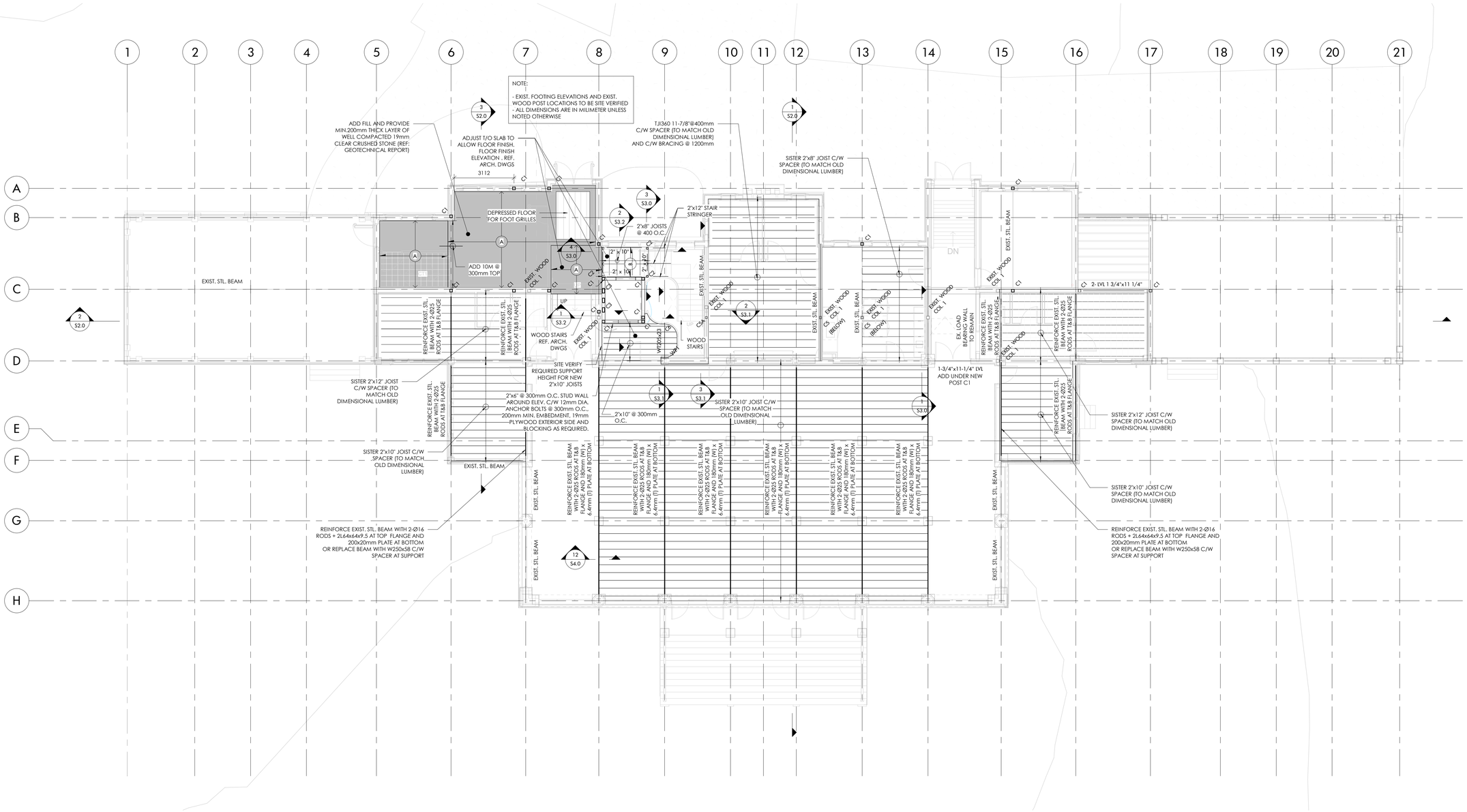
FLOOR PLAN - BASEMENT - PHASE 1

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:

S1.0



1 S1.1 FLOOR PLAN - LEVEL 1 - PHASE 1

1 : 100

COLUMN SCHEDULE

MARK	SIZE	BASEPLATE	ANCHOR BOLTS
C1	6"x6" SPF TYPE 1/2		
C2	4"x4" SPF TYPE 1/2		
C3	3 PLY 2"x10"		
C5	HS102x102x6.4	300 x 300 x19	4-Ø1/2" ANCHOR BOLTS
C5A	HS102x102x6.4 (A)		
C6	HANGER D20mm ROD		
EXIST. WOOD COL. 1	152mm x 152mm		

STRUCTURAL FLOOR SCHEDULE

Type Mark	Description
A	125mm S.O.G. C/W 152x152x10\10 WELDED WIRE MESH @MID-DEPTH. SLAB SHALL BEAR DIRECTLY ON COMPACTED GRANULAR. REFER TO GEO-TECNICAL REPORT FOR SPECIAL REQUIREMENT
B	19mm PLYWOOD GLUED AND SCREWED
C	19mm PLYWOOD TOP & BOTTOM GLUED AND SCREWED C/W 2"x4" @ 400mm O.C.

WALL PLATE SCHEDULE

WALL PLATES	PLATE SIZE (mm)	WALL PLATE REMARKS
WP1	300 x 300 x 19	4-Ø19mm THROUGH BOLTS

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

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Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

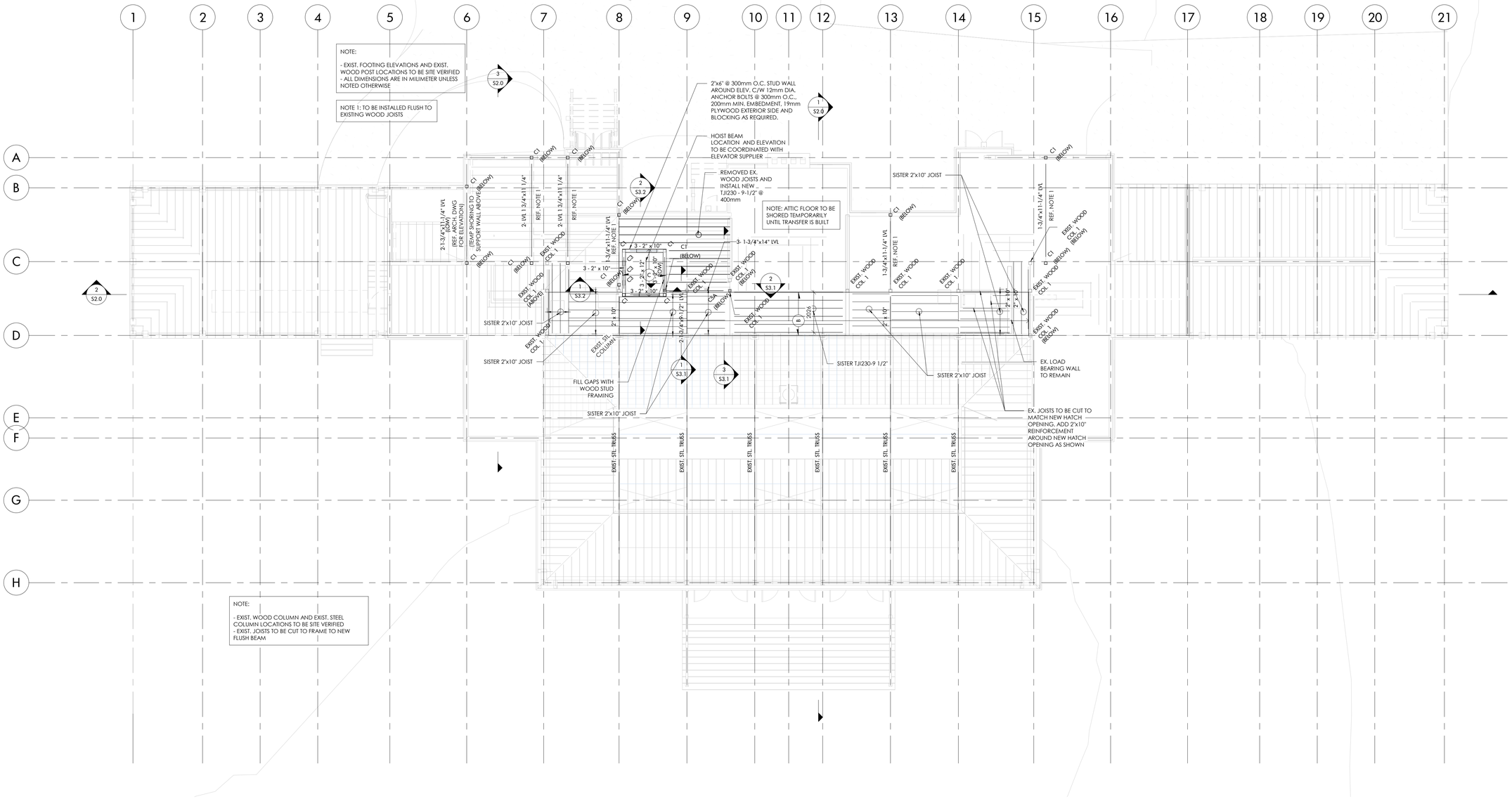
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
FLOOR PLAN - LEVEL 1 - PHASE 1

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:
S1.1



1 ATTIC LEVEL -PHASE 1
S1.2 1 : 100

COLUMN SCHEDULE

MARK	SIZE	BASEPLATE	ANCHOR BOLTS
C1	6"x6" SPF TYPE 1/2		
C2	4"x4" SPF TYPE 1/2		
C3	3 PLY 2"x10"		
C5	HS102x102x6.4	300 x 300 x19	4-Ø1/2" ANCHOR BOLTS
C5A	HS102x102x6.4 (A)		
C6	HANGER D20mm ROD		
EXIST. WOOD COL. 1	152mm x 152mm		

STRUCTURAL FLOOR SCHEDULE

Type Mark	Description
A	125mm S.O.G. C/W 152x152x10\10 WELDED WIRE MESH @MID-DEPTH. SLAB SHALL BEAR DIRECTLY ON COMPACTED GRANULAR. REFER TO GEO-TECNICAL REPORT FOR SPECIAL REQUIREMENT
B	19mm PLYWOOD GLUED AND SCREWED
C	19mm PLYWOOD TOP & BOTTOM GLUED AND SCREWED C/W 2"x4" @ 400mm O.C.

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

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Seals

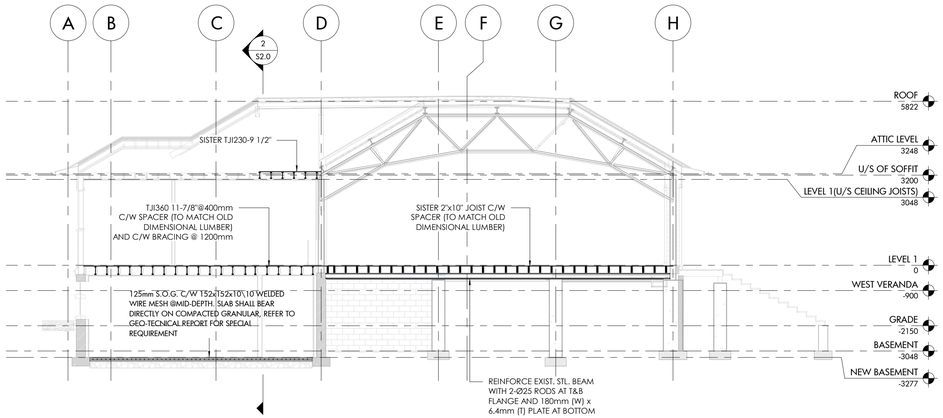
375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

HIGH PARK NATURE AND VISITOR'S CENTER

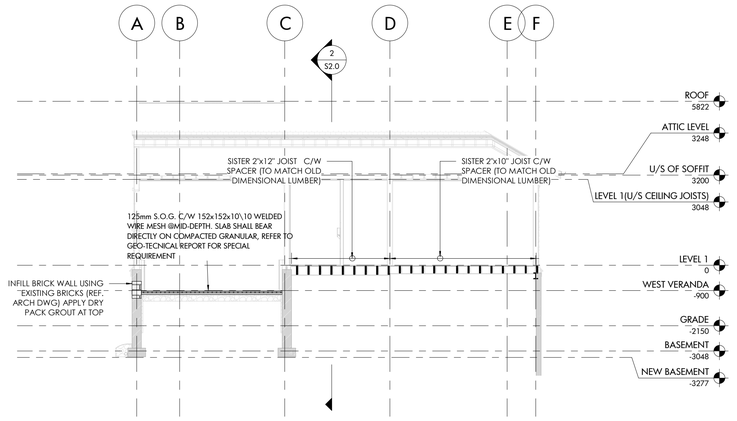
Drawing Name:
ATTIC PLAN - PHASE 1

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.
NORTH ARROW

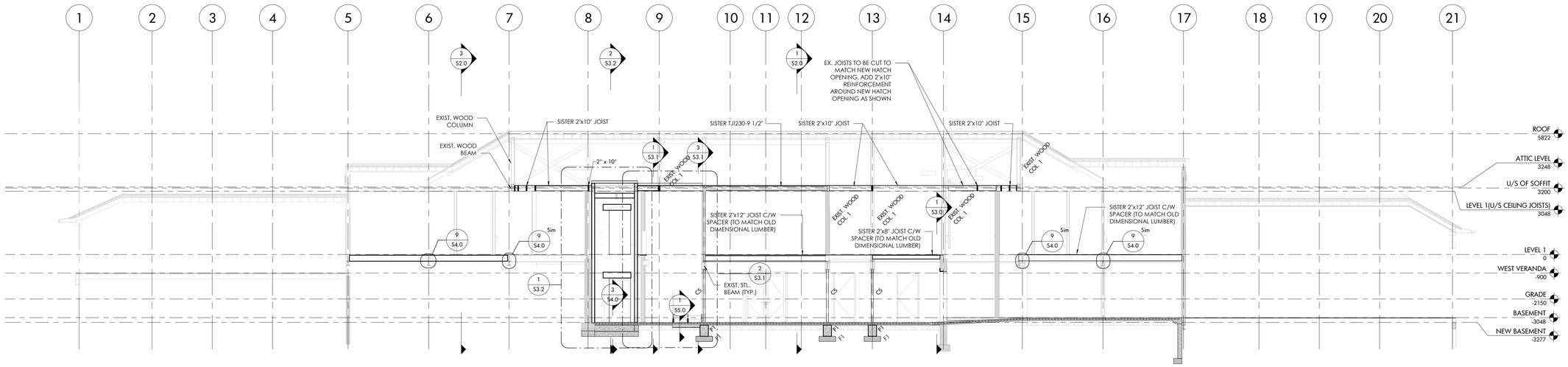
Drawing No.:
S1.2



1 S2.0 SECTION - 100 - AT GRID 12 - PHASE 1
1 : 100



3 S2.0 Section 52
1 : 100



2 S2.0 SECTION - 100 - PHASE 1
1 : 100

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

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375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

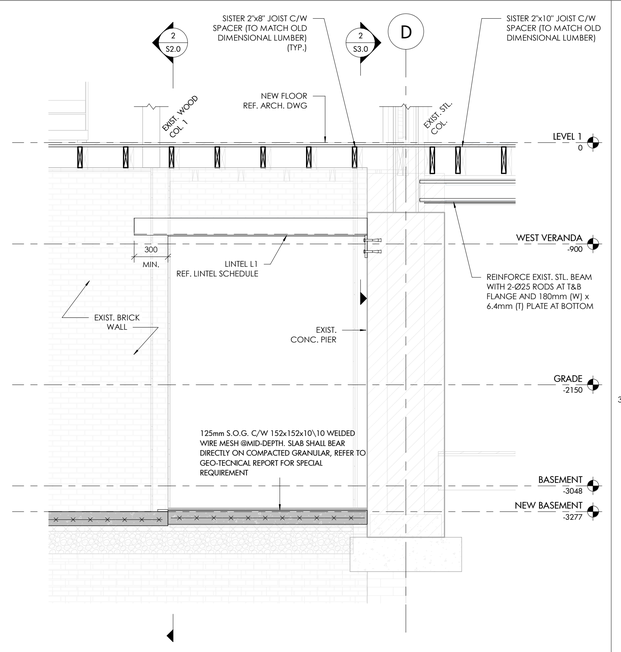
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
BUILDING SECTION

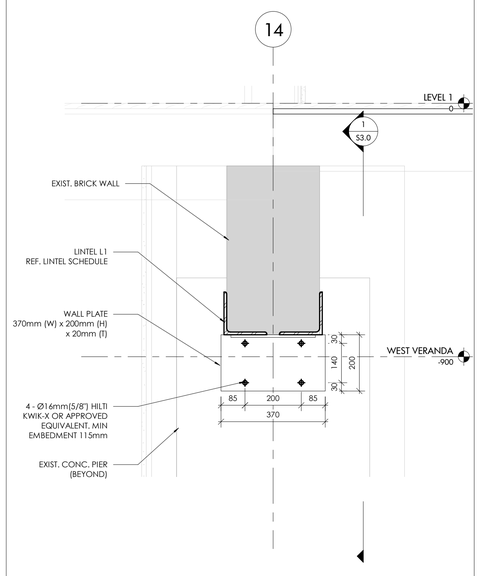
Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

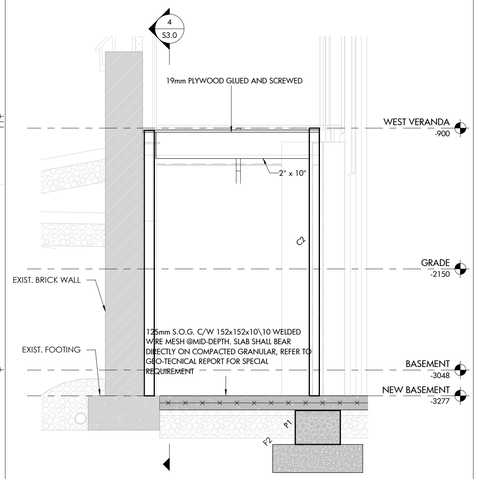
Drawing No.:
S2.0



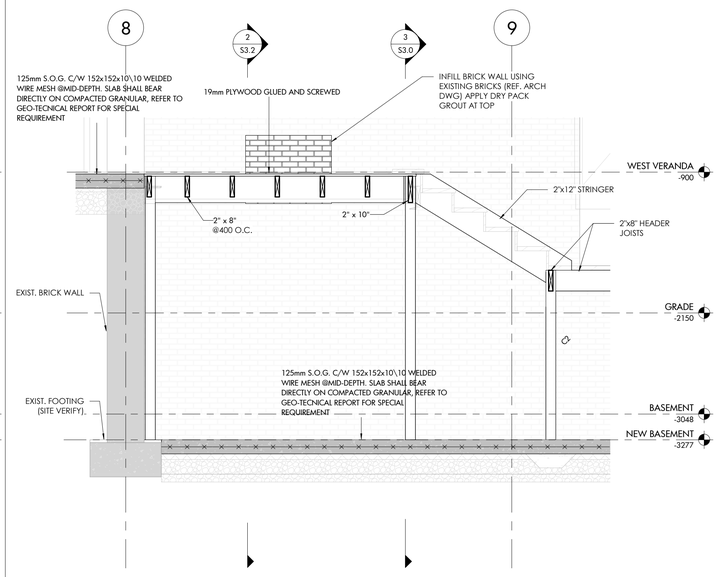
1 S3.0 SECTION 1
1 : 25



2 S3.0 LINTEL 1 DETAIL
1 : 10



3 S3.0 SECTION 57
1 : 25



4 S3.0 SECTION 53
1 : 25

5 S3.0 SECTION 63
1 : 25

6 S3.0 SECTION 64
1 : 25

7 S3.0 SECTION 65
1 : 25

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

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Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

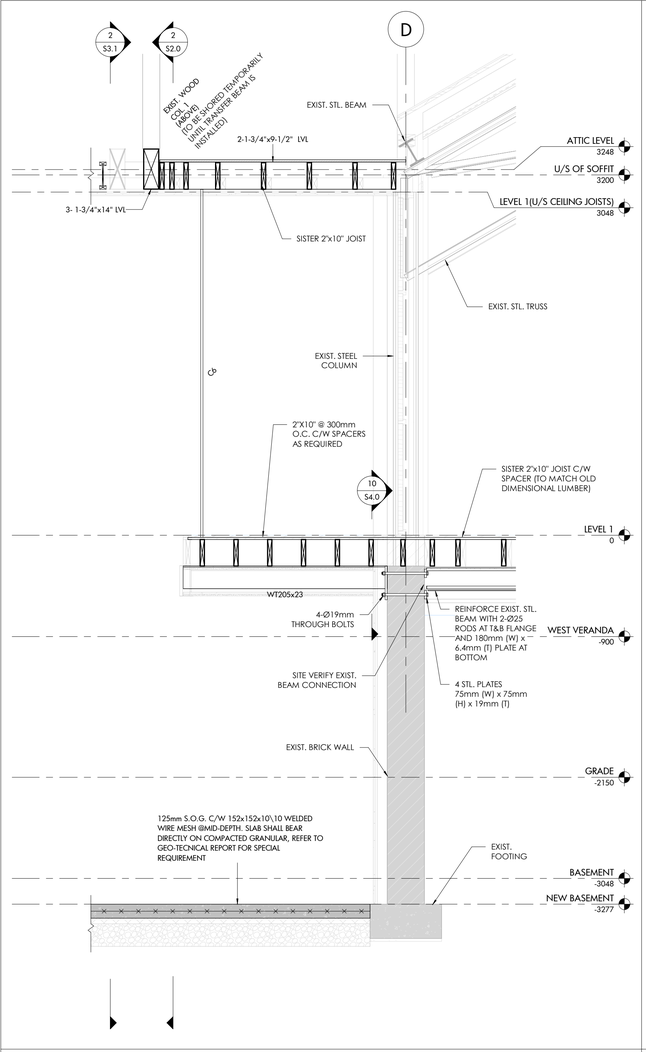
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
SECTIONS

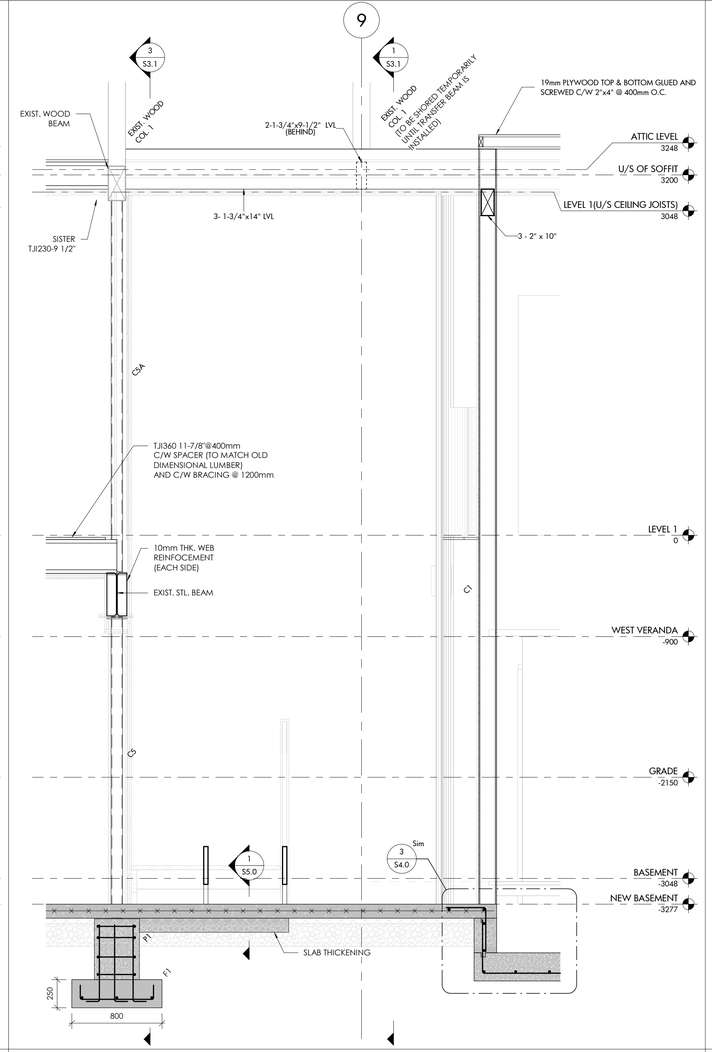
Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

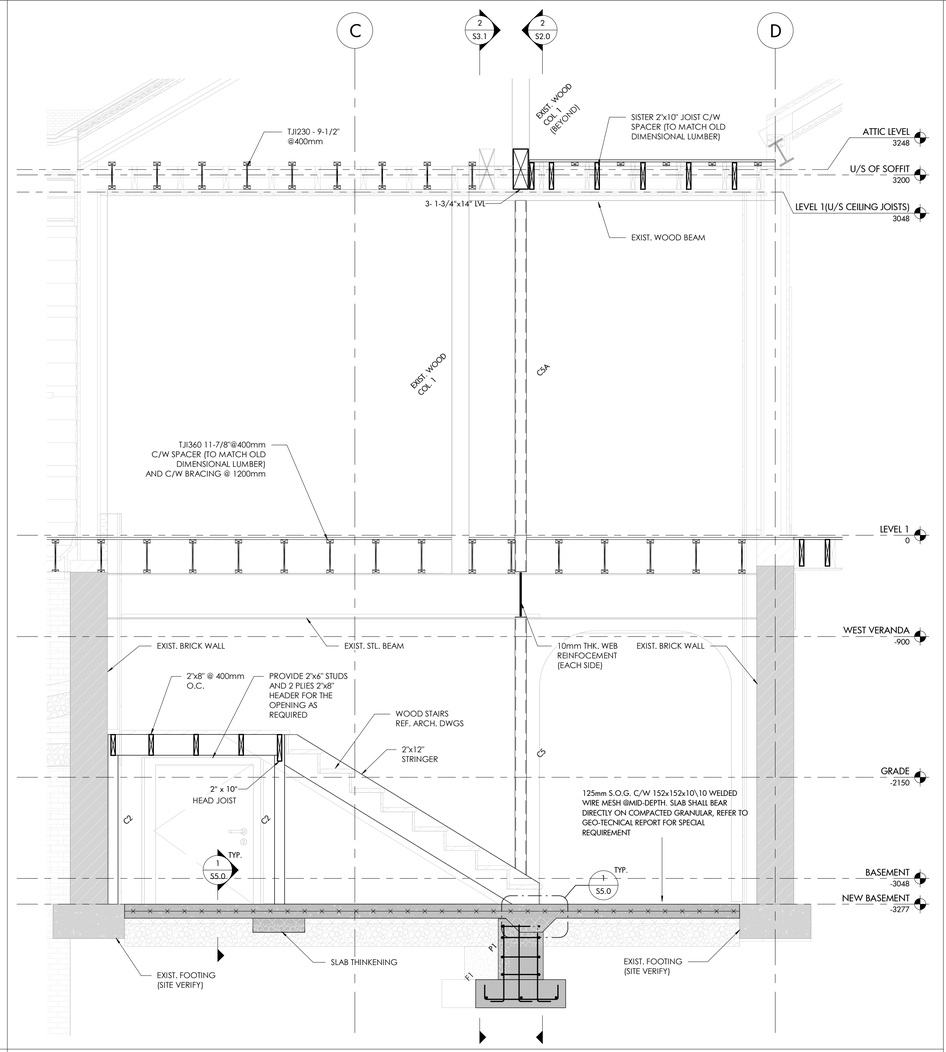
Drawing No.:
S3.0



1 S3.1 SECTION 68
1 : 25



2 S3.1 SECTION 70
1 : 25



3 S3.1 SECTION 71
1 : 25

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Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

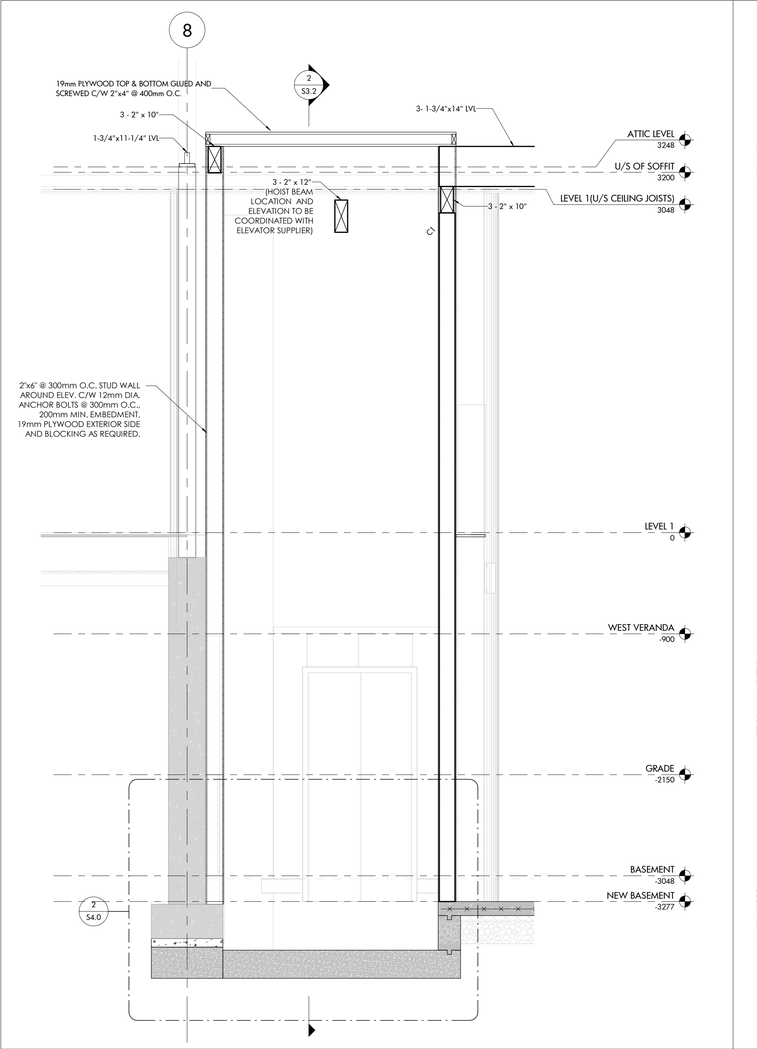
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
SECTIONS

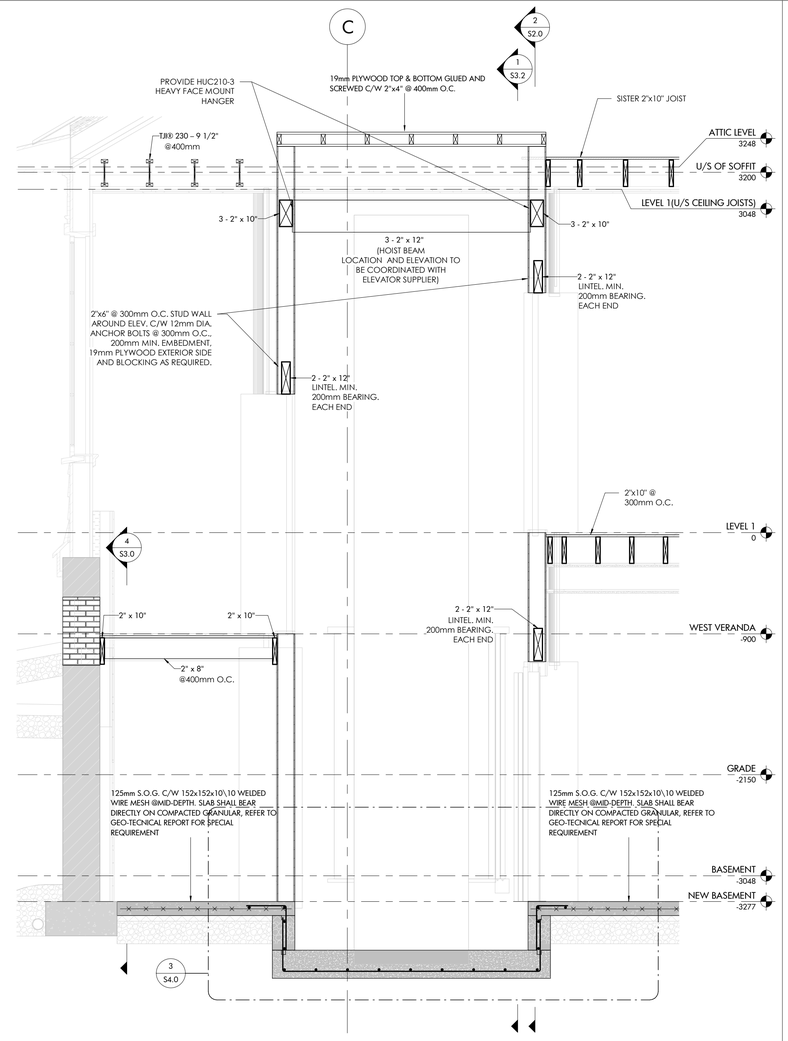
Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:
S3.1



1 S3.2 Section 74
1 : 25



2 S3.2 Section 75
1 : 25

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

Rev.#	Description	Date
2	ISSUED FOR TENDER	2025/02/25
1	ISSUED FOR TENDER REVIEW	2025/02/21

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Seals

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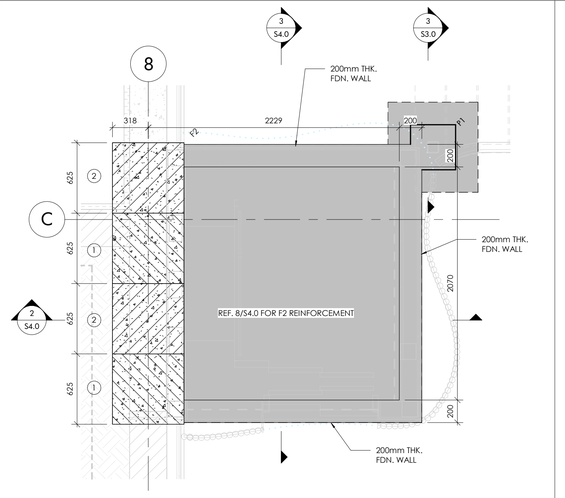
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
SECTIONS

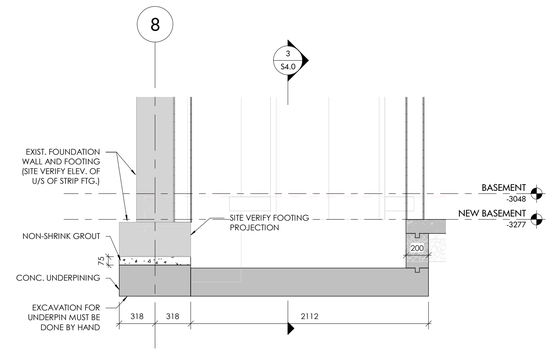
Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:
S3.2



1 S4.0 UNDERPIN FOUNDATION DETAIL
1 : 25



2 S4.0 Section 31
1 : 25

UNDERPINNING NOTES:

1. UNDERPINNING TO BE PLACED USING 30MPa CONCRETE AT 28 DAYS. MINIMUM STRENGTH AT 3 DAYS TO BE 25MPa

2. UNDERPINNING SECTIONS IN 625mm MAXIMUM WIDTHS IN ORDER 1 AND 2 AS SHOWN ON PLAN

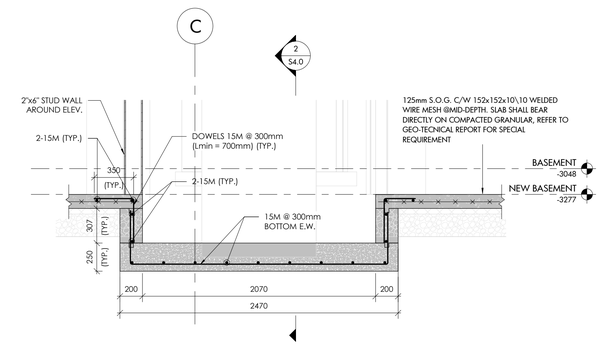
3. EXCAVATION SEQUENCE: PERFORM STAGED EXCAVATION AS PER DRAWING 1/S2.0. EACH SECTION MUST BE COMPLETED PRIOR TO STARTING NEXT ONE. UNDERPINNING IS TO BE PLACED DIRECTLY AGAINST EXISTING UNDISTURBED SOIL. ENSURE AGAINST VOIDS NOT FILLED WITH CONCRETE. ALLOW A MIN. OF 24 HRS. FOR UNDERPINNING CONC. AND THEN A MIN. OF 24 HRS. AFTER PLACING GROUT BEFORE STARTING AN ADJACENT SECTION.

4. RAM PACK (75mm) OF NON-SHRINK GROUT OR 1:3 CEMENT/SAND GROUT BETWEEN UNDERSIDE OF EXISTING FOOTING AND TOP OF UNDERPINNING NO SOONER THAN 24 HRS. AFTER PLACING UNDERPINNING.

5. ENSURE AGAINST LOSS OF MATERIAL FROM BENEATH EXISTING BUILDING.

6. PROVIDE ALL NECESSARY SHORING (PROVIDE AT LEAST 6'-0" CLEARANCE FROM THE STAGING AREA ON BASEMENT SLAB FOR SHORING) AND NEEDLING IN ORDER TO SAFELY CARRY OUT THIS WORK AND TO PREVENT DISTRESS IN THE EXISTING BUILDING.

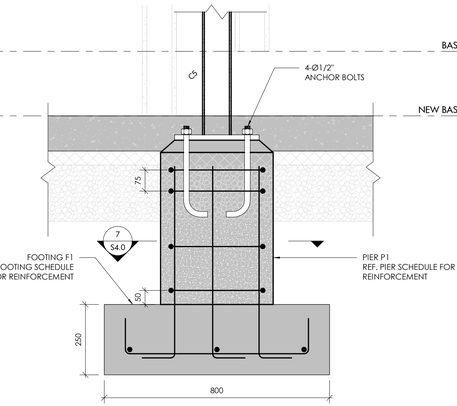
7. DETAILS SHOWN ARE FOR ASSUMED CONDITIONS. ANY VARIATIONS MUST BE REPORTED TO THE STRUCTURAL CONSULTANT BEFORE PROCEEDING.



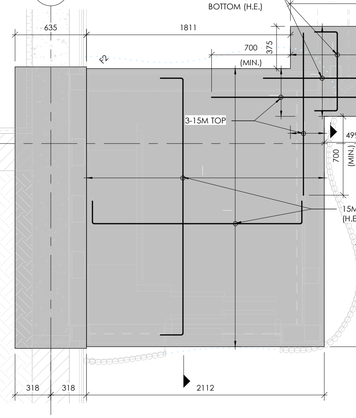
3 S4.0 SECTION 30
1 : 25

REVIEW ALL DRAWINGS AND VERIFY ALL DIMENSIONS AT THE SITE. DO NOT SCALE THE DRAWINGS. REPORT ALL DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH ANY CONSTRUCTION OR SHOP FABRICATION. ALL DRAWINGS, SPECIFICATIONS AND RELATED DOCUMENTS ARE THE COPYRIGHT PROPERTY OF 'MANTECON PARTNERS' AND MUST BE RETURNED UPON REQUEST. REPRODUCTION OF DRAWINGS, SPECIFICATIONS AND RELATED DOCUMENTS IN PART OR WHOLE IS FORBIDDEN WITHOUT THE ENGINEER'S WRITTEN PERMISSION.

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION



5 S4.0 PIER 1 DETAIL
1 : 10



7 S4.0 PIER 1 REINFORCEMENT DETAIL
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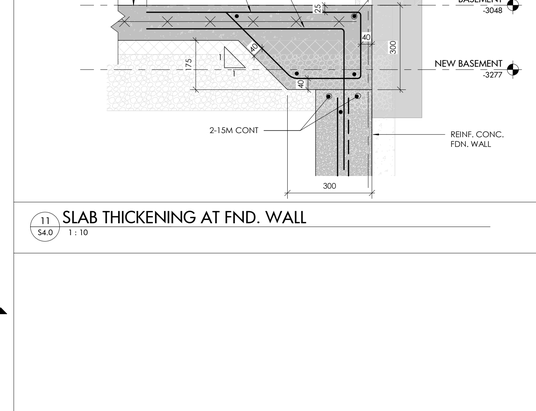
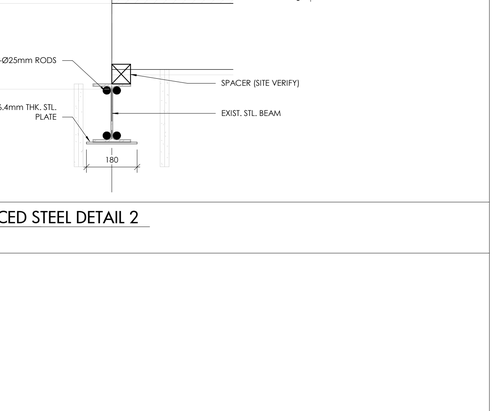


9 S4.0 REINFORCED STEEL BEAM DETAIL 1
1 : 10



10 S4.0 WALL PLATE WP1 DETAIL
1 : 10

6 S4.0 PIER 1 SECTION
1 : 10

8 S4.0 F2 MAT FOUNDATION DETAIL
1 : 25

11 S4.0 SLAB THICKENING AT FND. WALL
1 : 10

12 S4.0 REINFORCED STEEL DETAIL 2
1 : 10

Rev.#	Description	Date
4	ISSUED FOR TENDER	2025/02/25
3	ISSUED FOR TENDER REVIEW	2025/02/21
2	ISSUED FOR COSTING	2024/02/08
1	ISSUED FOR PERMIT	2024/01/31

Consultants

MANTECON PARTNERS
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Seals

375 COLBORNE LODGE DR, TORONTO, ON M6R 2Z3

HIGH PARK NATURE AND VISITOR'S CENTER

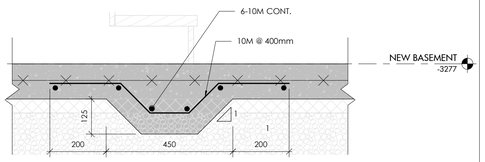
Drawing Name:
DETAILS

Project Number:	22-142
Drawing Scale:	As Indicated
Date:	2025/02/25
File Name:	
Drawn By:	D.N
Reviewed By:	D.U.

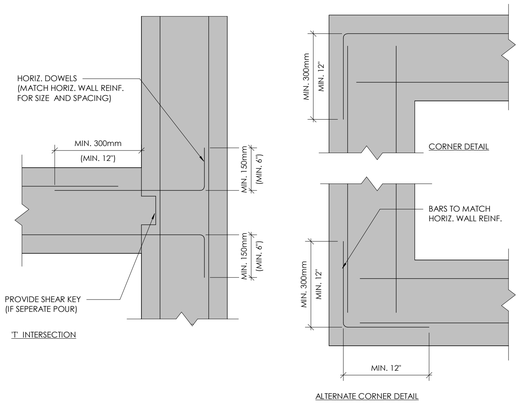
NORTH ARROW

Drawing No.:

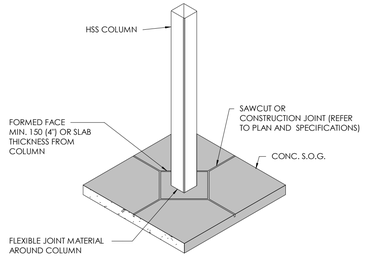
S4.0



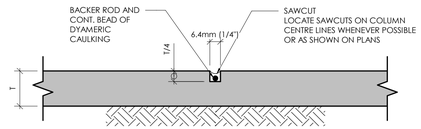
1 S5.0 TYP. SLAB THICKENING
1 : 10



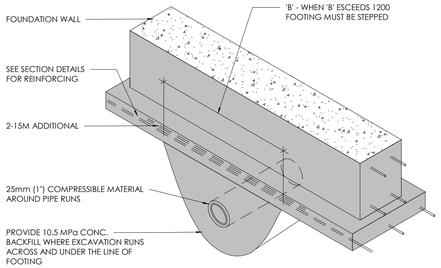
2 S5.0 TYP. FOUNDATION PLAN DETAIL- WALL INTERSECTION
1 : 10



3 S5.0 TYP. COLUMN ISOLATION JOINT
1 : 10



4 S5.0 TYP. SAWCUT CONTROL JOINT IN SLAB ON GRADE
1 : 10



5 S5.0 TYP. PIPE UNDERNEATH FOOTING DETAIL
1 : 10

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

Rev.#	Description	Date
4	ISSUED FOR TENDER	2025/02/25
3	ISSUED FOR TENDER REVIEW	2025/02/21
2	ISSUED FOR COSTING	2024/02/08
1	ISSUED FOR PERMIT	2024/01/31

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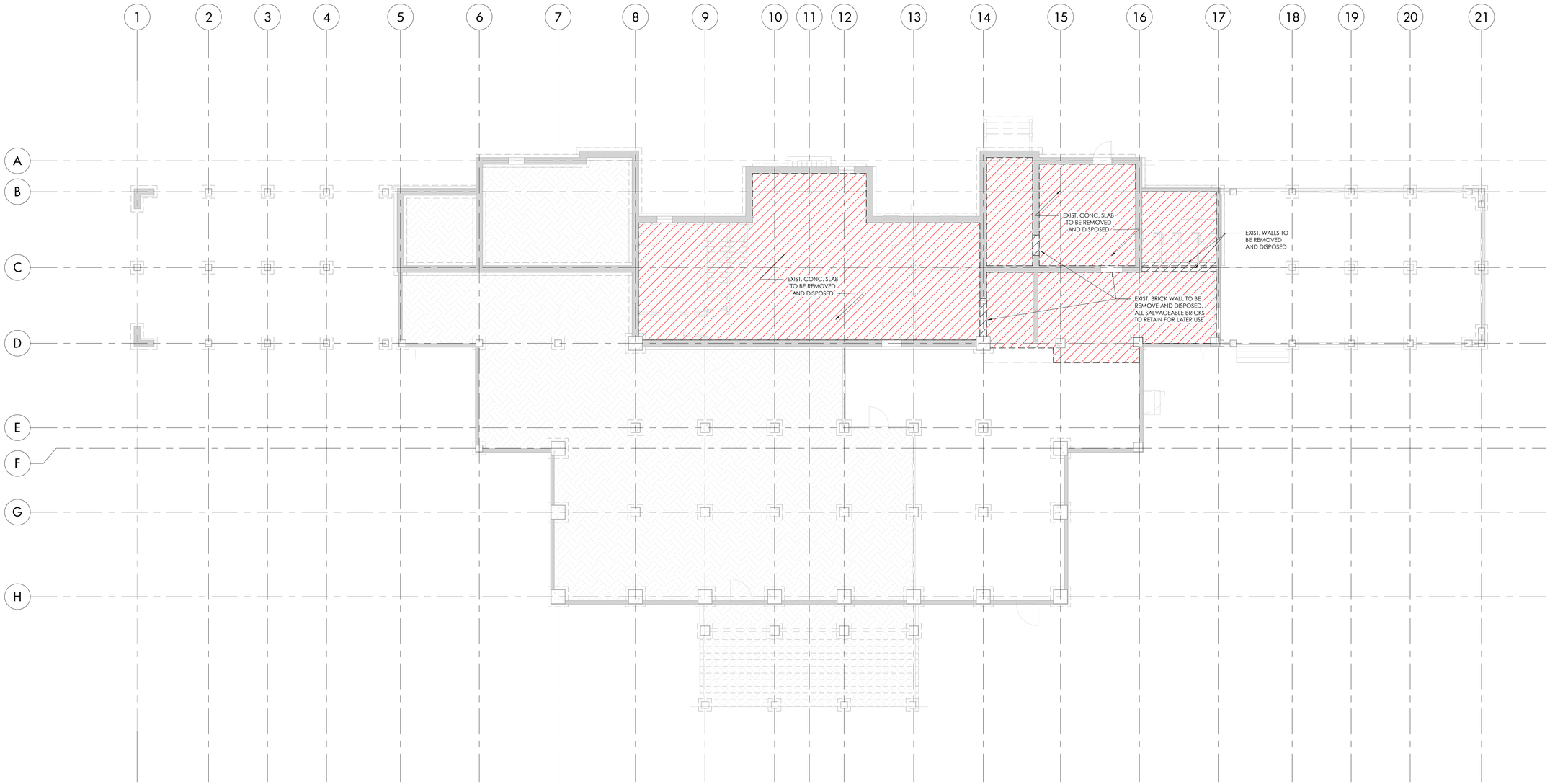
HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
TYPICAL DETAIL

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.
NORTH ARROW

Drawing No.:
S5.0

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION



1 / SD1.0 DEMOLISION BASEMENT FLOOR PLAN

1 : 100

Rev.#	Description	Date
4	ISSUED FOR TENDER	2025/02/25
3	ISSUED FOR TENDER REVIEW	2025/02/21
2	ISSUED FOR COSTING	2024/02/08
1	ISSUED FOR PERMIT	2024/01/31

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HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:

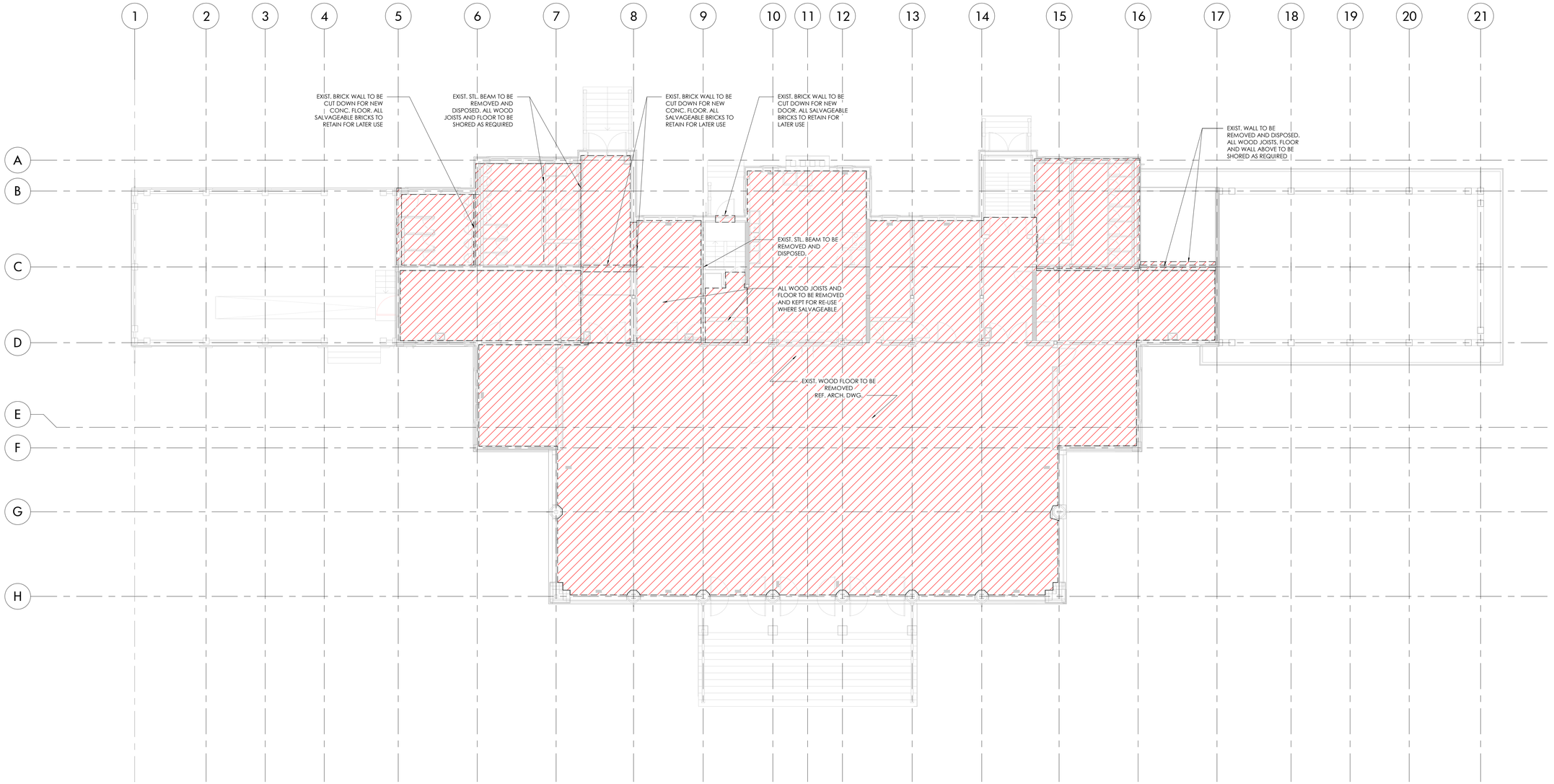
DEMOLITION FLOOR PLAN - BASEMENT - PHASE 1

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:

SD1.0



1 SD2.0 DEMOLISION LEVEL 1 FLOOR PLAN
1 : 100

NOTE: REFER TO ARCHITECTURAL DRAWINGS FOR PHASING INFORMATION

Rev #	Description	Date
4	ISSUED FOR TENDER	2025/02/25
3	ISSUED FOR TENDER REVIEW	2025/02/21
2	ISSUED FOR COSTING	2024/02/08
1	ISSUED FOR PERMIT	2024/01/31

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HIGH PARK NATURE AND VISITOR'S CENTER

Drawing Name:
DEMOLITION FLOOR PLAN - FIRST FLOOR - PHASE 1

Project Number: 22-142
Drawing Scale: As Indicated
Date: 2025/02/25
File Name:
Drawn By: D.N
Reviewed By: D.U.

NORTH ARROW

Drawing No.:
SD2.0