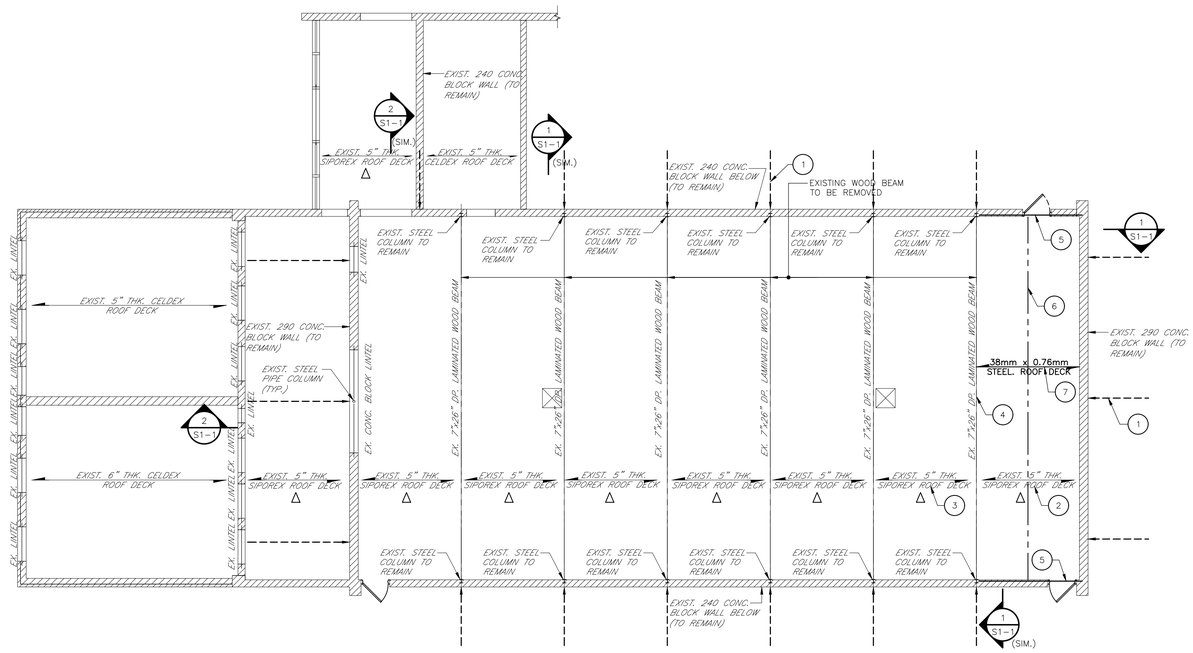
DEMOLITION PROCEDURES

MARK	REMARKS
1	INSTALL SHORING AROUND PERIMETER OF BUILDING. SHORING SHOWN ON PLAN IS SCHEMATIC ONLY. SHORING IS TO BE DESIGNED BY LICENSED ENGINEER, HIRED BY SHORING CONTRACTOR. SUBMIT SHORING DRAWINGS FOR REVIEW PRIOR TO CONSTRUCTION.
2	STARTING FROM ONE END OF BUILDING, REMOVE ONE BAY OF EXISTING SIPOREX ROOF PANELS.
3	REMOVE NEXT BAY OF EXISTING SIPOREX ROOF PANEL.
4	REMOVE EXISTING WOOD BEAM AND INSTALL NEW OWSJ. AS PER ROOF FRAMING PLAN.
5	INSTALL NEW STEEL BEAMS AT EDGE OF BUILDING, AS PER ROOF FRAMING PLAN.
6	INSTALL NEW OWSJ AS PER ROOF FRAMING PLAN.
7	INSTALL NEW METAL ROOF DECK, AS PER ROOF FRAMING PLAN.
8	CONTINUE THIS SEQUENCE OF REMOVING AND REINSTALLING ROOF FRAMING FROM ONE END OF THE BUILDING TO THE OTHER.

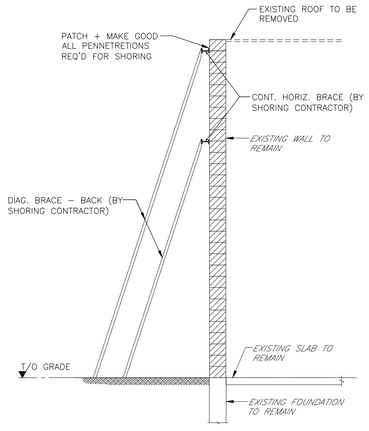
NOTE:
1. UPPER GYMNASIUM ROOF TO BE COMPLETED PRIOR TO DEMOLITION OF LOWER ROOFS
2. CONTRACTOR SHALL MAKE ALL PROPER PROVISIONS NECESSARY TO ENSURE INTERIOR BUILDING FINISHES ARE NOT DAMAGED DURING CONSTRUCTION.
3. SHORING MAY BE REMOVED AS NEW METAL DECK IS INSTALLED
4. CONTRACTOR SHALL PATCH AND MAKE GOOD ALL FINISHES WHERE SHORING TIES INTO THE REMAINING STRUCTURE



EXISTING ROOF FRAMING PLAN - DEMOLITION

SCALE= 1:100

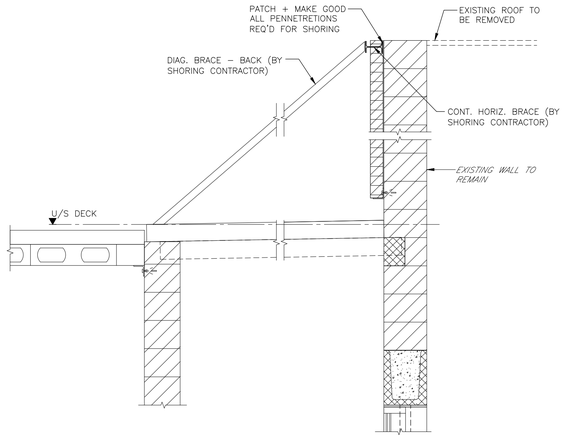
LEGEND	
MARK	REMARKS
△	DENOTES EXISTING SIPOREX ROOF DECK TO BE REMOVED



1 S1-1 TYPICAL SHORING DETAIL AT LOW ROOF

SCALE= N.T.S.

NOTE: SCHEMATIC ONLY. SHORING DESIGN BY SHORING ENGINEER. SUBMIT DRAWINGS FOR REVIEW PRIOR TO CONSTRUCTION.



2 S1-1 TYPICAL SHORING DETAIL AT LOW ROOF

SCALE= N.T.S.

NOTE: SCHEMATIC ONLY. SHORING DESIGN BY SHORING CONTRACTOR

CONSTRUCTION NORTH

Contractor must verify all dimensions on the Project Site and report any discrepancies before proceeding with the Work.

This drawing is a part of the Contract Documents and is to be read in conjunction with all other Contract Documents.

Revision Record

No.	Description	Date (m/d/y)
3	ISSUED FOR TENDER	04/14/26
2	ISSUED FOR BUILDING PERMIT	03/30/26
1	ISSUED FOR CLIENT REVIEW	03/10/26
No.	Description	Date (m/d/y)

Issue Record

General Notes:

LANHACK Steelcon Inc.
Consulting Engineers
1709 Upper James Street
Hamilton, ON L9B 1K7
Tel: (905) 777-1454
Fax: (905) 336-8142

Regina Mundi Catholic Elementary School

ROOF ASSEMBLY REPLACEMENT

675 MOHAWK ROAD WEST
HAMILTON, ONTARIO

Date:	JANUARY 2026
Drawn By:	VE
Chkd By:	MB
Scale:	AS NOTED

Drawing title:

EXISTING ROOF FRAMING PLAN DEMOLITION

Project No.:	Drawing No.:	Rev.:
26032(A)	S1-1	3

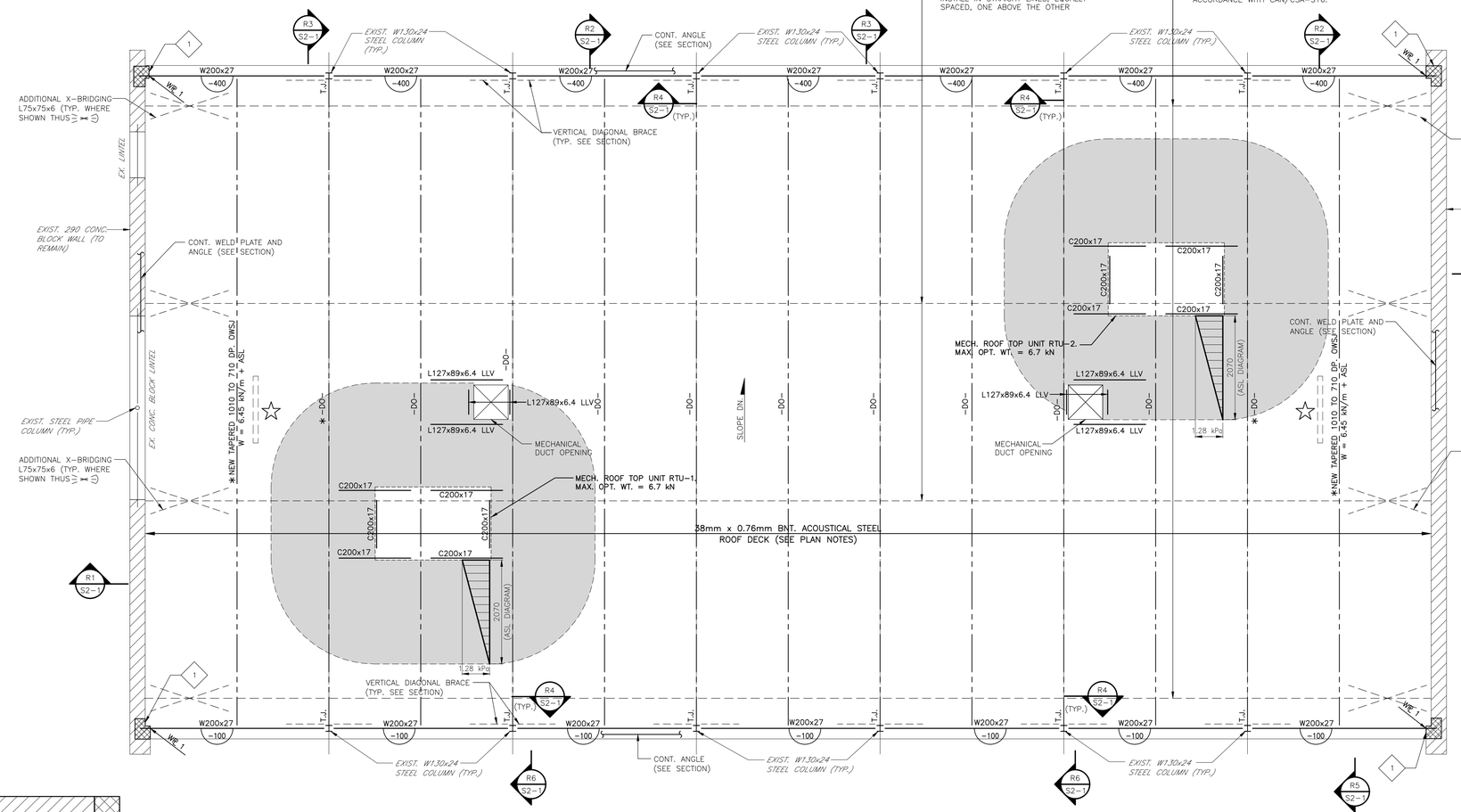
Plot Date:

GYMNASIUM ROOF FRAMING PLAN NOTES

1. U/S OF ROOF DECK AT HIGH POINTS TO BE 0.00m ABOVE ROOF DATUM ELEVATION 7.28m, UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS. TOD = TOP OF DECK. ELEVATION DATUM TAKEN FROM TOP OF FINISHED GYM FLOOR.
2. TOP OF GYM FLOOR TAKEN FROM DATUM AS 0.00m
3. ROOF DECK TO SLOPE TO DRAINS AS SHOWN ON ARCHITECTURAL DRAWINGS.
4. TOP OF STEEL BEAMS TO BE 0.0m BELOW ROOF DATUM ELEVATION, EXCEPT AS SHOWN THUS (xx) ON PLAN.
5. OWSJ SHOES TO BE 100mm DEEP UNLESS NOTED OTHERWISE.
6. TOTAL DEAD LOAD AS FOLLOWS:

   GYMNASIUM ROOF (OWSJ) – 1.0 kPa

7. DEAD LOAD OF FOUR PLY BUILT-UP ROOFING SYSTEM IS ASSUMED TO BE 0.32 kPa UNLESS NOTED OTHERWISE.
8. LIVE LOAD IS A UNIFORM LOAD OF 1.84 kPa PLUS ACCUMULATED SNOW LOAD (ASL) IN ACCORDANCE WITH THE ONTARIO BUILDING CODE REQUIREMENTS AND IN NO CASE LESS THAN AS NOTED ON PLAN.
9. STEEL ROOF DECK SHALL BE DESIGNED TO SUPPORT SPECIFIED TOTAL DEAD AND LIVE LOADS. MINIMUM BASE NOMINAL THICKNESS (BNT) OF STEEL DECK SHALL BE 0.76mm (22 GA.).
10. STEEL ROOF DECK SHALL BE INSTALLED FOR DIAPHRAGM ACTION IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE CANADIAN SHEET STEEL BUILDING INSTITUTE AND SPECIFICATIONS. STEEL DECK TO BE FASTENED AT ALL SUPPORTS W/ MIN. 20mmø PUDDLE WELDS (36/9 SUPPORT PATTERN) AND 150 O/C AT PERIMETER. ALL SIDE LAPS TO BE BUTTON PUNCHED AT 230 O/C.
11. STEEL JOISTS SHALL BE DESIGNED TO SUPPORT SPECIFIED TOTAL DEAD AND LIVE LOADS. IN ADDITION, JOISTS SHALL BE DESIGNED FOR ADDITIONAL LOADS SHOWN ON PLAN, AND FOR POINT LOADS OF BRACING MECHANICAL EQUIPMENT, IN EXCESS OF 0.66 kN PER JOIST.
12. JOISTS AND BEARING ANCHORAGE SHALL BE DESIGNED TO RESIST UPLIFT DUE TO WIND AS REQUIRED BY THE ONTARIO BUILDING CODE AND IN NO CASE LESS THAN THE GREATER OF THOSE INDICATED ON PLAN OR 0.48 kPa NET UPLIFT.
13. JOISTS SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER. SHOP DRAWINGS AND CALCULATIONS BEARING THE STAMP AND SIGNATURE OF THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION AND ERECTION.
14. PROVIDE CONTINUOUS TOP AND BOTTOM CHORD BRIDGING FOR JOISTS IN ACCORDANCE WITH CAN3–S16.1M AND IN NO CASE LESS THAN AS SHOWN ON PLAN.
15. PROVIDE BOTTOM CHORD BRACING FOR ALL JOISTS SUBJECT TO NET UPLIFT IN ACCORDANCE WITH CAN3–CSA–S16.1 AND AT LEAST AT EACH END OF JOISTS NEAR THE FIRST BOTTOM CHORD PANEL POINT. BRIDGING CANNOT BE CONSIDERED AS BRACING.
16. LIVE LOAD DEFLECTION OF ROOF JOIST SHALL NOT EXCEED 1/360 OF SPAN. EXCEPT JOISTS MARKED THUS * ON PLAN SHALL HAVE A MAXIMUM LIVE LOAD DEFLECTION OF 12mm.
17. "W" NOTED AGAINST OWSJ INCLUDES TOTAL SERVICE LIVE AND DEAD LOADS IN KILONEWTONS PER LINEAL METRE; INCLUDING ASSUMED JOIST SELF WEIGHT AND EXCLUDING ALL ACCUMULATED SNOW LOADS.
18. GROUT IN MASONRY WALLS TO BE EITHER "CONCRETE" OR "GROUT" WITH A MIN. COMPRESSIVE STRENGTH OF 20 MPa U.N.O. MORTAR IS UNACCEPTABLE.
19. ALL ROOF TRUSSES SHALL BE DESIGNED FOR THE EFFECTS OF PONDING IN ACCORDANCE WITH THE ONTARIO BUILDING CODE.
20. OWSJ OVER GYMNASIUM TO BE A MODIFIED WARREN CONFIGURATION WITH A VERTICAL WEB MEMBER, SYMMETRICAL ABOUT THE CENTRELINE WITH DIAGONAL AND VERTICAL MEMBERS FABRICATED FROM RECTANGULAR OR SQUARE HSS SECTIONS OF UNIFORM APPEARANCE. ALL WEB MEMBERS ARE TO LINE UP. ALL CONNECTIONS TO BE NEAT AND UNIFORM. ALL WORK TO BE DONE TO THE SATISFACTION OF THE ARCHITECT. SEE ARCH. DWGS. FOR OVERALL CONFIGURATION OF THE TRUSSES. SEE ALSO ELEV. ON THIS DWG.
21. "T.J." NOTED AGAINST OWSJ DENOTES "TIE JOIST" ie, EXTEND BOTTOM CHORD AND CONNECT TO SUPPORT.



GYMNASIUM ROOF FRAMING PLAN

SCALE= 1:50

SUSPENDED BASKETBALL STOP, WT.= 500 kg. ON OWSJ. CO–ORDINATE CONNECTION DETAILS AND LOADS WITH SUPPLIER BEFORE PROCEEDING WITH FABRICATION. TYP. WHERE DENOTED THUS ☆ ON PLAN

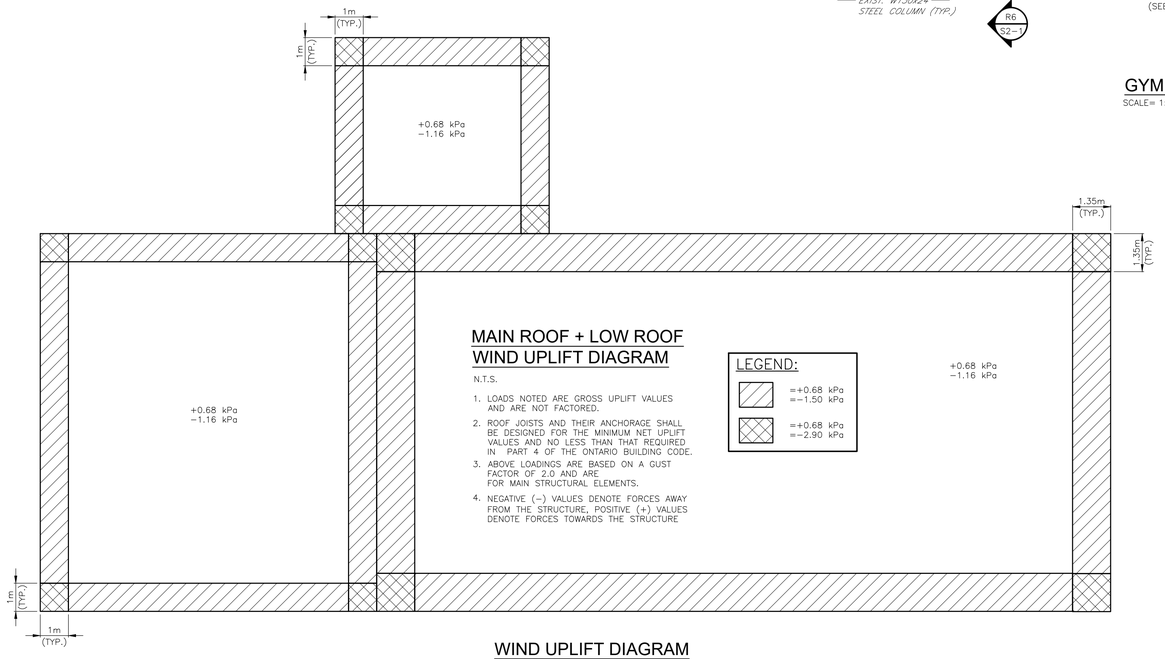
WALL PLATE SCHEDULE

MARK	SIZE	REMARKS
WP. 1	150x16x150	C/W 2–12.7mmø x 300 LG. HOOKED ANCHOR RODS
WP. 2	175x13x102	C/W 2–12.7mmø x 150 LG. HOOKED ANCHOR RODS

LAST DIMENSION PARALLEL TO BEAM WEB

DRAWING LEGEND

1 — LOCALLY POCKET EXIST. WALL A MIN. OF 800 LG. x WIDTH OF WALL. GROUT SOLID VOIDS OF BLOCK DIRECTLY BELOW BEAM. INSTALL NEW WALL PLATE FOR BEARING OF NEW BEAM. GROUT/PACK SOLID AND MAKE GOOD FINISH TO MATCH EXIST., ONCE NEW WORK IS COMPLETE



WIND UPLIFT DIAGRAM

SCALE: N.T.S

CONSTRUCTION NORTH

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Tel: (905) 777-1454
Fax: (905) 336-8142

Regina Mundi Catholic Elementary School

ROOF ASSEMBLY REPLACEMENT

675 MOHAWK ROAD WEST
HAMILTON, ONTARIO

Date:	JANUARY 2026
Drawn By:	VE
Chkd By:	MB
Scale:	1:50

Drawing title:

GYMNASIUM ROOF FRAMING PLAN

Project No.:	Drawing No.:	Rev.:
26032(A)	S1-2	3

Plot Date:

LOBBY ROOF FRAMING PLAN NOTES

1. U/S OF ROOF DECK TO MATCH T/O OF EXIST. SIPOREX PLANK, UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS.
2. ROOF DECK TO BE PLACED FLAT. SLOPE TO DRAINS TO BE ACHIEVED BY ROOFING SYSTEM, AS SHOWN ON ARCHITECTURAL DRAWINGS.
3. TOP OF STEEL BEAMS TO BE 0.0mm BELOW ROOF DATUM ELEVATION, EXCEPT AS SHOWN THUS (xx) ON PLAN.
4. TOTAL DEAD LOAD AS FOLLOWS: LOBBY ROOF – 1.0 kPa
5. DEAD LOAD OF FOUR PLY BUILT-UP ROOFING SYSTEM IS ASSUMED TO BE 0.32 kPa UNLESS NOTED OTHERWISE.
6. LIVE LOAD IS A UNIFORM LOAD OF 1.84 kPa PLUS ACCUMULATED SNOW LOAD (ASL) IN ACCORDANCE WITH THE ONTARIO BUILDING CODE REQUIREMENTS AND IN NO CASE LESS THAN AS NOTED ON PLAN.
7. STEEL ROOF DECK SHALL BE DESIGNED TO SUPPORT SPECIFIED TOTAL DEAD AND LIVE LOADS. MINIMUM BASE NOMINAL THICKNESS (BNT) OF STEEL DECK SHALL BE 0.76mm. (22 GA.).
8. STEEL ROOF DECK SHALL BE INSTALLED FOR DIAPHRAGM ACTION IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE CANADIAN SHEET STEEL BUILDING INSTITUTE AND SPECIFICATIONS. STEEL DECK TO BE FASTENED AT ALL SUPPORTS W/ MIN. 20mmø PUDDLE WELDS (36/7 SUPPORT PATTERN) AND 150 O/C AT PERIMETER. ALL SIDE LAPS TO BE BUTTON PUNCHED AT 300 O/C.
9. LOCATION OF MECHANICAL EQUIPMENT AND MECHANICAL EQUIPMENT LOADS ARE TO BE CONFIRMED BY MECHANICAL CONTRACTOR BEFORE STRUCTURAL STEEL IS FABRICATED. REFER TO MECHANICAL DRAWINGS.
10. FRAME ALL ROOF OPENINGS AS NOTED IN SPECIFICATIONS AND ON TYPICAL DETAIL.
11. SUBMIT DETAILS FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON STRUCTURAL DRAWINGS TO STRUCTURAL CONSULTANT FOR REVIEW.
12. AN INDEPENDENT INSPECTION AND TESTING COMPANY IS TO INSPECT STRUCTURAL STEEL AND STEEL DECK IN THE SHOP AND IN THE FIELD FOR WELDING, CONNECTIONS, BOLT TORQUES, AND GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
13. SEE TYPICAL NOTES, TYPICAL DETAILS, AND ALL OTHER DRAWINGS.

DRAWING LEGEND

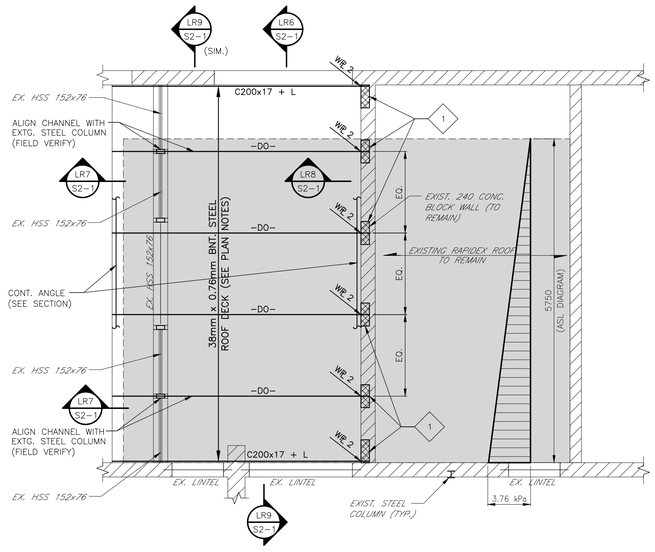
1 — LOCALLY POCKET EXIST. WALL A MIN. OF 800 LG. x WIDTH OF WALL. GROUT SOLID VOIDS OF BLOCK DIRECTLY BELOW BEAM. INSTALL NEW WALL PLATE FOR BEARING OF NEW BEAM. GROUT/PACK SOLID AND MAKE GOOD FINISH TO MATCH EXIST., ONCE NEW WORK IS COMPLETE

WALL PLATE SCHEDULE

MARK	SIZE	REMARKS
WP. 1	150x16x150	C/W 2–12.7mmø x 300 LG. HOOKED ANCHOR RODS
WP. 2	175x13x102	C/W 2–12.7mmø x 150 LG. HOOKED ANCHOR RODS
LAST DIMENSION PARALLEL TO BEAM WEB		

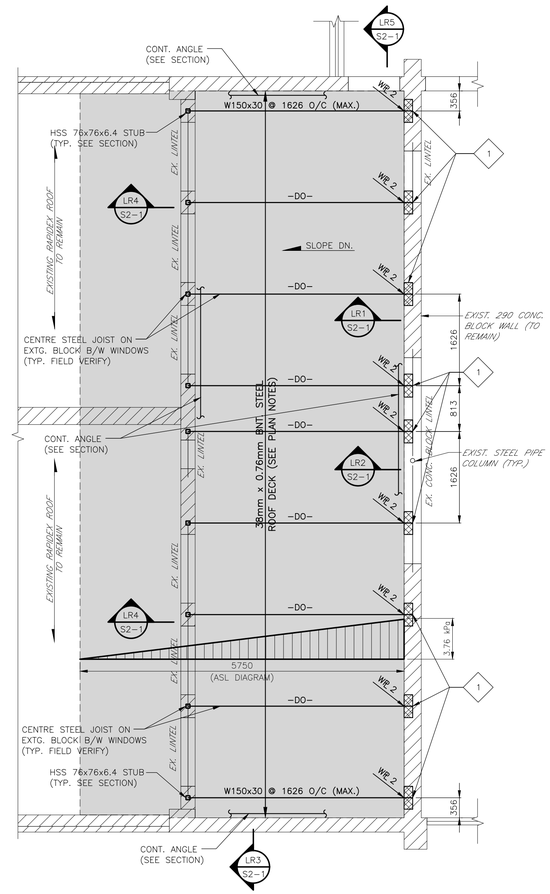
KITCHEN ROOF FRAMING PLAN NOTES

1. U/S OF ROOF DECK AT PERIMETER AND HIGH POINTS TO BE 0.0m ABOVE ROOF DATUM ELEVATION 190.610m, UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS.
2. ROOF DECK TO SLOPE TO DRAINS, AS SHOWN ON ARCHITECTURAL DRAWINGS.
3. TOTAL DEAD LOAD AS FOLLOWS: LOBBY ROOF – 1.0 kPa
4. DEAD LOAD OF FOUR PLY BUILT-UP ROOFING SYSTEM IS ASSUMED TO BE 0.32 kPa UNLESS NOTED OTHERWISE.
5. LIVE LOAD IS A UNIFORM LOAD OF 1.84 kPa PLUS ACCUMULATED SNOW LOAD (ASL) IN ACCORDANCE WITH THE ONTARIO BUILDING CODE REQUIREMENTS AND IN NO CASE LESS THAN AS NOTED ON PLAN.
6. STEEL ROOF DECK SHALL BE DESIGNED TO SUPPORT SPECIFIED TOTAL DEAD AND LIVE LOADS. MINIMUM BASE NOMINAL THICKNESS (BNT) OF STEEL DECK SHALL BE 0.76mm.
7. STEEL ROOF DECK SHALL BE INSTALLED FOR DIAPHRAGM ACTION IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE CANADIAN SHEET STEEL BUILDING INSTITUTE AND SPECIFICATIONS. STEEL DECK TO BE FASTENED AT ALL SUPPORTS W/ MIN. 20mmø PUDDLE WELDS (36/7 SUPPORT PATTERN) AND 150 O/C AT PERIMETER. ALL SIDE LAPS TO BE BUTTON PUNCHED AT 300 O/C.
8. LOCATION OF MECHANICAL EQUIPMENT AND MECHANICAL EQUIPMENT LOADS ARE TO BE CONFIRMED BY MECHANICAL CONTRACTOR BEFORE STRUCTURAL STEEL IS FABRICATED. REFER TO MECHANICAL DRAWINGS.
9. FRAME ALL ROOF OPENINGS AS NOTED IN SPECIFICATIONS AND ON TYPICAL DETAIL.
10. SUBMIT DETAILS FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON STRUCTURAL DRAWINGS TO STRUCTURAL CONSULTANT FOR REVIEW.
11. AN INDEPENDENT INSPECTION AND TESTING COMPANY IS TO INSPECT STRUCTURAL STEEL AND STEEL DECK IN THE SHOP AND IN THE FIELD FOR WELDING, CONNECTIONS, BOLT TORQUES, AND GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
12. SEE TYPICAL NOTES, TYPICAL DETAILS, AND ALL OTHER DRAWINGS.



LOBBY ROOF FRAMING PLAN

SCALE= 1:50



KITCHEN ROOF FRAMING PLAN

SCALE= 1:50

Revision Record

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1709 Upper James Street
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Tel: (905) 777-1454
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Regina Mundi Catholic Elementary School

ROOF ASSEMBLY REPLACEMENT

675 MOHAWK ROAD WEST
HAMILTON, ONTARIO

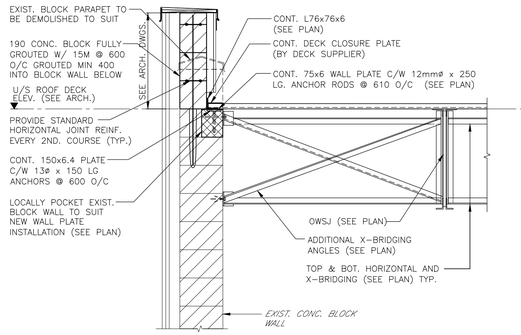
Date:	JANUARY 2026
Drawn By:	VE
Chkd By:	MB
Scale:	1:50

Drawing title:

LOBBY AND KITCHEN ROOF FRAMING PLANS

Project No.:	Drawing No.:	Rev.:
26032(A)	S1-3	3

Plot Date:



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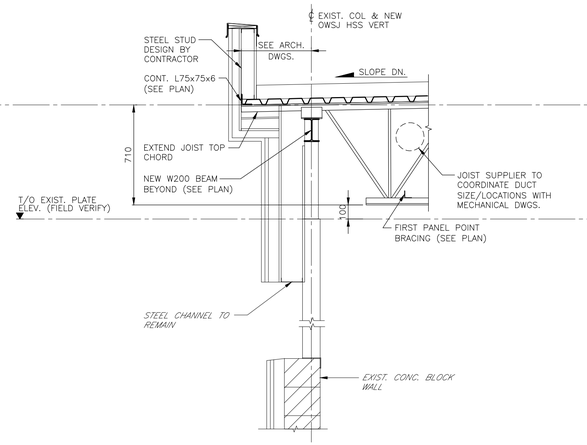

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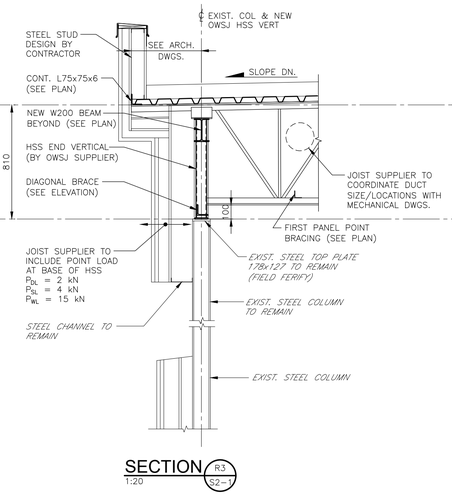

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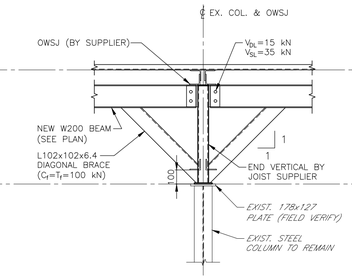

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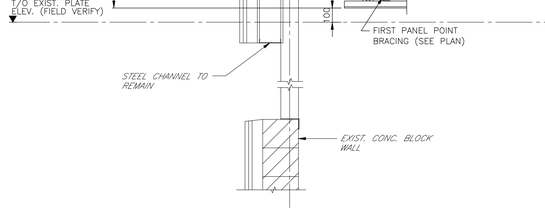

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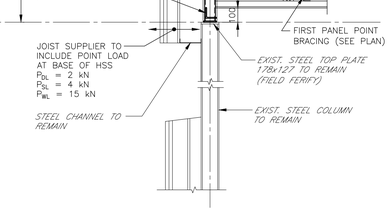

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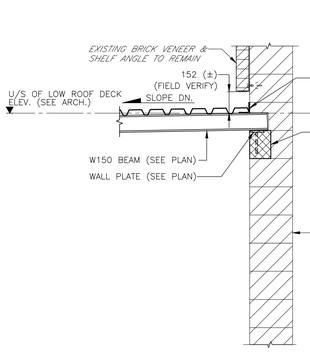

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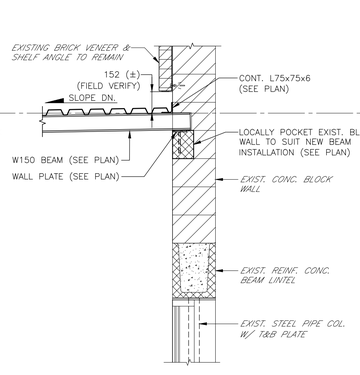

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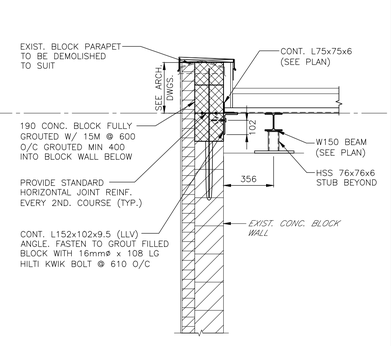

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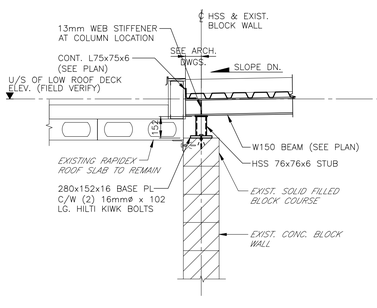

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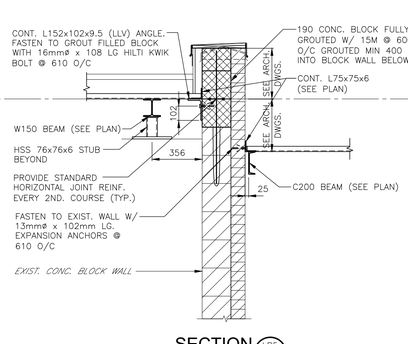

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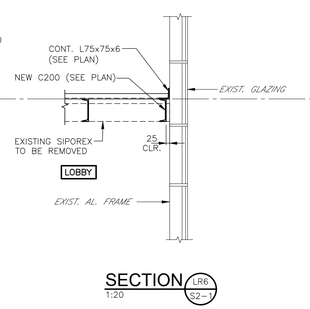

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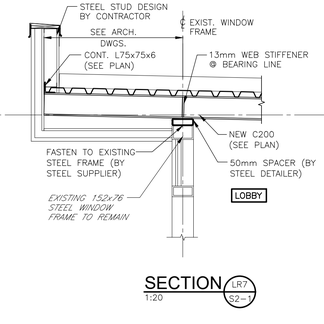

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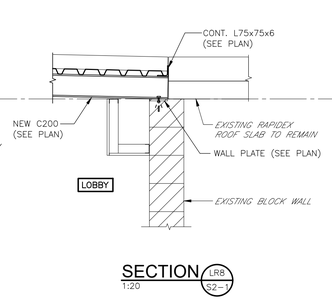

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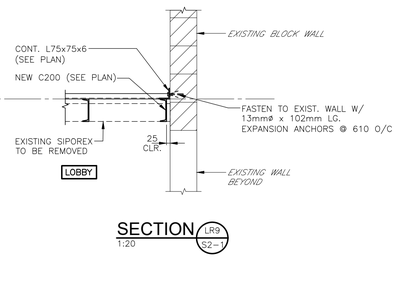

SECTION LR9 / S2-1
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Drawn By:	VE
Chkd By:	MB
Scale:	1:20

Drawing title:

SECTIONS

Project No.:	Drawing No.:	Rev.:
26032(A)	S2-1	3

Plot Date:

LOAD BEARING MASONRY NOTES

1.0 GENERAL

1.1 THE FOLLOWING INDICATES ONLY THE MINIMUM REQUIREMENTS APPLICABLE TO STRUCTURAL LOAD BEARING MASONRY, BASED UPON CSA S304-14 (R2010) DESIGN OF MASONRY STRUCTURES, CLAUSE 10.5.2.

1.2 REFER ALSO TO ARCHITECTURAL DRAWINGS &/OR THE SPECIFICATION FOR REQUIREMENTS OTHER THAN STRUCTURAL, AND FOR NON-LOAD BEARING WALLS & PARTITIONS.

1.3 IF MASONRY CONSTRUCTION IS BASED ON ENGINEERING ANALYSIS "ENGINEERED MASONRY", THEN REFER TO NOTES & DETAILS ON STRUCTURAL DRAWINGS.

1.4 MASONRY CONSTRUCTION TO CONFORM TO CSA STANDARDS CSA-S304-14 & CSA A37-14.

2.0 PRODUCTS

2.1 CONCRETE BLOCKS & BRICKS:- TO CONFORM TO ONE OR MORE OF CSA A165 SERIES-14. BLOCKS TO BE MODULAR UNITS AS SHOWN ON THE ARCHITECTURAL DRAWINGS &/OR SPECIFICATION, AND UNLESS OTHERWISE NOTED SHALL BE:-
1. FOR BELOW GRADE & EXTERIOR EXPOSED WALLS USE NORMAL WEIGHT LOAD BEARING UNITS:-
   STANDARD HOLLOW: TYPE H/15/A/M.
   75% SOLID: TYPE S/15/A/M.
   100% SOLID: TYPE S/15/A/M.
2. FOR INTERIOR ABOVE GRADE WALLS USE LIGHTWEIGHT LOAD BEARING BLOCKS:-
   STANDARD HOLLOW: TYPE H/15/C/M.
   75% SOLID: TYPE S/15/C/M.
   100% SOLID: TYPE S/15/C/M.

2.2 CLAY BRICKS:- TO CONFORM TO ONE OR MORE OF CSA A82-14. SEE ARCHITECTURAL DRAWINGS &/OR SPECIFICATIONS FOR TYPES & STYLES OF BRICKS REQUIRED. UNLESS OTHERWISE NOTED, THE MINIMUM COMPRESSIVE STRENGTH (BRICK FLATWISE) GROSS AREA SHALL BE 20 MPa.

2.3 MORTAR:- TO CONFORM TO CSA A179-14. FOR LAYING CONCRETE BLOCKS...USE TYPE "S" MORTAR UNLESS NOTED. FOR LAYING CLAY BRICKS: USE TYPE "N" MORTAR UNLESS NOTED.

2.4 MASONRY GROUT:- TO CONFORM TO CSA A179-14. THE SLUMP SHALL BE + 200mm (+8") AND THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 12.5 MPa.

2.5 MASONRY CONNECTORS:- (ANCHORS, FASTENERS & TIES) SHALL CONFORM TO CSA A370-14, AND BE INSTALLED TO COMPLY WITH CSA A371-14. SPACING, STRENGTH & GALVANIZING OF STRIP TIES, DOVETAIL ANCHORS, BAR ANCHORS, ROD ANCHORS, STRAP ANCHORS, WALL & PARTITION ANCHORS SHALL COMPLY WITH CSA A371-14.

2.6 BRICK VENEER AND MASONRY TIES TO BE DESIGNED AND CERTIFIED BY MASONRY CONTRACTOR'S ENGINEER. PROVIDE CALCULATIONS AND DETAILS, CERTIFIED BY A PROFESSIONAL ENGINEER, LICENSED IN THE PROVINCE OF ONTARIO. TIES TO CONFORM TO O.B.C. 2012 AND CSA A370-14 & CSA A371-14. TIES TO BE DESIGNED FOR SEISMIC REQUIREMENTS, IN ACCORDANCE WITH O.B.C. 2012 AND CSA S304-14.

2.7 VERTICAL REINFORCING FOR ALL NON-LOAD BEARING WALLS AND PARTITIONS: THE FOLLOWING ARE MINIMUM REQUIREMENTS:-
1. 90 (4") BLOCK = 10M @ 800mm (2'-8") O/C
   140 (6") BLOCK = 15M @ 1000mm (3'-4") O/C
   190 (8") BLOCK = 15M @ 800mm (2'-8") O/C
   240 (10") BLOCK = 15M @ 600mm (2'-0") O/C
   290 (12") BLOCK = 15M @ 400mm (1'-4") O/C

2.8 HORIZONTAL JOINT REINFORCEMENT FOR ALL MASONRY WALLS: THE FOLLOWING ARE MINIMUM REQUIREMENTS:-
1. CONFORM TO CSA A370-14 & A371-14.
2. REINFORCEMENT SHALL BE AN APPROVED CONTINUOUS "LADDER" TYPE, PREFABRICATED WITH 3.66mm DIAMETER (9 GAUGE) LONGITUDINAL & CROSS WIRES.
3. SPACING:- PROVIDE REINFORCING IN THE TOP COURSE IMMEDIATELY BELOW FLOOR & ROOF BEARING LEVELS AND THE FIRST TWO COURSES ABOVE AND BELOW EVERY WALL OPENING. THE REINFORCING SHALL EXTEND 600mm (24") BEYOND SUCH OPENINGS. FOR THE REMAINDER OF WALLS, THE VERTICAL SPACING SHALL NOT EXCEED 400mm (16").
4. OVERLAP SPLICES:- SHALL BE A MIN. OF 150mm (6") FOR KNURLED WIRE & 300mm (12") FOR PLAIN WIRE. LAPS SHALL BE STAGGERED A MINIMUM OF 750mm (30") FROM COURSE TO COURSE. REINFORCING SHALL NOT PASS THROUGH A VERTICAL CONTROL JOINT UNLESS OTHERWISE SHOWN.
5. CORROSION RESISTANCE:- JOINT REINFORCING FOR ALL WALLS IN CONTACT WITH SOIL, EXTERIOR WALLS & WALLS IN A MOIST ENVIRONMENT SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION TO ASTM A153/A153M-09, 458 gm/sq.meter (1.5oz./sq.foot).
6. COMPOSITE & CAVITY WALLS:- WHERE COURSING OF WYTHES DO NOT ALIGN OF WHERE IT IS DESIRABLE & PERMITTED TO BUILD ONE WYTHE BEFORE THE OTHER, REINFORCING SHALL BE AN APPROVED ADJUSTABLE TYPE WITH A BOX OR EYE SECTION WHICH EXTENDS INTO THE COLLAR JOINT OR CAVITY AND RESTRAINS THE TRANSVERSE MOVEMENT OF THE TWO WYTHES. FOR CAVITY WALLS WITH RIGID INSULATION, EXTENSION SHALL BE DESIGNED TO HOLD THE INSULATION IN PLACE BY USE OF PLASTIC WEDGES OR APPROVED EQUAL. GALVANIZED HOOK STYLE "BOX TIES" OR "PIN-TIES" SHALL EXTEND INTO THE FACE WYTHE TO COMPLETE THE ASSEMBLY.
7. PROVIDE ALL PREFABRICATED CORNER AND TEE SECTIONS.

2.9 COMPOSITE WALLS:- SHALL HAVE THE VERTICAL COLLAR JOINTS BETWEEN WYTHES COMPLETELY FILLED WITH MORTAR OR GROUT.

2.10 BOND BEAMS:- MADE FROM LINTEL BLOCKS, OR HALF WEB BLOCKS WHERE SHOWN ON STRUCTURAL DRAWINGS SHALL CONFORM TO CSA A371-14.

2.11 GROUTING:- BY FILLING VOIDS OF HOLLOW UNITS & REINFORCED HOLLOW UNITS SHALL CONFORM TO CSA A371-14 (MORTAR IS NOT ACCEPTABLE).

2.12 EXPANSION & CONTROL JOINTS:- SHALL BE PROVIDED. SEE ARCHITECTURAL DRAWINGS &/OR SPECIFICATION FOR DETAILS.

3.0 EXECUTION

3.1 BEARINGS ON MASONRY:-
1. MINIMUM BEARING ON MASONRY UNLESS OTHERWISE NOTED:-
   BEAMS (STEEL,CONC.,WOOD).....200mm (8") NOMINAL
   LINTELS (STEEL,CONC.,WOOD).....150mm (6") NOMINAL
   JOISTS (STEEL,WOOD).....100mm (4") NOMINAL
   SLABS (CAST-IN-PLACE,PRECAST)...100mm (4") NOMINAL
   STEEL DECKING (ON WELD PLATE)...100mm (4") NOMINAL
2. MASONRY BEARINGS SHALL BE OF SOLID BLOCKS (OR GROUTED SOLID) OR BRICKS LAID IN MORTAR. ALL JOINTS ARE TO BE FULLY FILLED WITH TYPE 'S' MORTAR.
3. MIN. SIZE OF SOLID BEARINGS AT BEAMS AND LINTELS UNLESS NOTED SHALL BE EQUAL TO TWICE THE BEARING/WALL PLATE (WP) LENGTH AND FOR A DEPTH EQUAL TO THE BEARING/WALL PLATE (WP) LENGTH, AND IN NO CASE LESS THAN 400 LONG x 200 DEEP (16" x 8"), SYMMETRICAL UNDER BEARING POINT.
4. PROVIDE A MINIMUM OF ONE CONTINUOUS COURSE 200mm (8") OF SOLID OR GROUTED VOID BLOCKS OR BRICKS LAID IN MORTAR AT THE TOP COURSE IMMEDIATELY BELOW ALL FLOOR AND ROOF BEARING LEVELS.

3.2 TOLERANCES:- UNLESS OTHERWISE NOTED ON THE ARCHITECTURAL DRAWINGS &/OR SPECIFICATION, SHALL CONFORM TO CSA A371-14.

3.3 COLD WEATHER CONSTRUCTION:- REQUIREMENTS & PROTECTION SHALL CONFORM TO CSA A371-14 AND UNDER NO CIRCUMSTANCES SHALL MASONRY CONSTRUCTION BE PERMITTED WHEN THE AIR TEMPERATURE FALLS BELOW - 12°C.

4.0 QUALITY CONTROL

4.1 WHEN REQUESTED SAMPLING AND TESTING SHALL CONFORM TO CSA S304-14. REFER ALSO TO GENERAL NOTES.

STEEL DECK NOTES

1.0 GENERAL

1.1 DESIGN, FABRICATION, HANDLING & ERECTION SHALL CONFORM TO THE FOLLOWING STANDARDS:-
1. CSA S136.
2. CSSBI 10M: STANDARD FOR STEEL ROOF DECK.
3. CSSBI: STANDARD FOR COMPOSITE STEEL DECK.
4. ASTM A525: GENERAL REQUIREMENTS FOR STEEL SHEET, ZINC COATED.
5. WELDING SHALL CONFORM TO CSA STANDARD W59 AND BE PERFORMED BY A FABRICATOR CERTIFIED TO CSA STANDARD W47.

1.2 WHEREVER STRUCTURAL FRAMING PERMITS, STEEL DECK SHALL BE DESIGNED & FABRICATED TO SPAN CONTINUOUSLY OVER AT LEAST 4 SUPPORTS (3 SPANS). PROVIDE AN ADEQUATE INCREASE IN THICKNESS OF METAL TO COMPENSATE FOR CONTINUITY WHEREVER FEWER SUPPORTS MAY OCCUR. END LAPS TO BE A MIN. OF 50mm (2") AND BE LOCATED OVER SUPPORTS.

1.3 1. DEFLECTION OF ROOF DECK UNDER LIVE LOAD ONLY SHALL NOT EXCEED 1/240TH OF SPAN.

2. ROOF DECK SHALL BE FORMED WITH INTEGRAL RIBS IN ORDER TO SAFELY SUPPORT THE LOADS GIVEN ON THE DRAWINGS OVER THE SPANS REQUIRED. DECK THICKNESS GIVEN ON DRAWINGS IS MINIMUM ALLOWED.

1.4 1. FLOOR DECK SHALL BE FORMED WITH INTEGRAL RIBS AND EMBOSSMENTS FOR COMPOSITE ACTION WITH CONCRETE SLAB IN ORDER TO SAFELY SUPPORT THE LOADS GIVEN ON THE DRAWINGS OVER THE SPANS REQUIRED. IN ADDITION, THE DECK SHALL SAFELY SUPPORT ALL CONSTRUCTION LOADS UNTIL CONCRETE IS SET. DECK THICKNESS GIVEN ON DRAWINGS IS MINIMUM ALLOWED.

2. DEFLECTION OF COMPOSITE FLOOR UNDER LIVE LOAD ONLY SHALL NOT EXCEED 1/360TH OF SPAN.

1.5 DESIGN & DETAIL ON SHOP DRAWINGS CONNECTIONS TO REPORTING MEMBERS SO THAT DIAPHRAGM FORCES ARE PROPERLY TRANSMITTED.

1.6 CLEARLY SHOW ON SHOP DRAWINGS POSITION OF TEMPORARY SHORING FOR FLOOR DECK IF REQUIRED.

1.7 STEEL ROOF DECK SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER. SHOP DRAWINGS AND CALCULATIONS BEARING THE STAMP AND SIGNATURE OF THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION AND ERECTION.

2.0 PRODUCTS

2.1 UNLESS OTHERWISE NOTED ROOF DECK AND/OR COMPOSITE DECK SHALL BE FORMED OF METALLIC COATED SHEET STEEL CONFORMING TO CSSBI 101 M, & ASTM A446M, STRUCTURAL QUALITY GRADE 'A' WITH A ZF75 ZINC COATING.

2.2 UNLESS OTHERWISE NOTED DECK SHALL BE SINGLE FLUTED ELEMENT WITH INTEGRAL RIBS OF DEPTH & MIN. BASE NOMINAL THICKNESS (BNT) AS NOTED ON THE DRAWINGS. DECK SHALL HAVE INTERLOCKING SIDE JOINTS BETWEEN PANELS. [MIN. BNT 0.76mm (0.30")]

2.3 COVER PLATES, CELL CLOSURES, FLASHINGS & REINFORCING STIFFENERS FOR UNSUPPORTED EDGES TO BE SUPPLIED OF SIMILAR MATERIAL & ZINC COATING TO THAT FOR DECK, UNLESS NOTED.

2.4 PRIMER PAINT TO BE ZINC RICH, READY MIX TO CGSB 1-GP-181M FOR FIELD "TOUCH-UP" OF WELD BURNS AFTER DECK IS INSTALLED.

2.5 UNLESS OTHERWISE SHOWN FOR OPENINGS THROUGH ROOF DECK FROM 150mm TO 450mm (6" TO 18") ACROSS THE FLUTES PROVIDE NOT LESS THAN A L50x50x6 (L2x2x1/4). REINFORCEMENT TO FRAME ACROSS EACH SIDE OF THE OPENING PERPENDICULAR TO THE FLUTES, WELDED TO AT LEAST TWO FLUTES EACH SIDE OF THE OPENING.

2.6 FOR ROOF OPENINGS OVER 450mm (18") ACROSS THE FLUTES AND FOR AREAS OF CONCENTRATED LOAD, REINFORCE IN ACCORDANCE WITH STRUCTURAL FRAMING DETAILS SHOWN ON PLANS OR TYPICAL DETAILS.

3.0 EXECUTION

3.1 SUPPLY AND PLACE STEEL PACKING AS REQUIRED TO PRODUCE AN EVEN BEARING PRESSURE AT SUPPORTS.

3.2 UNLESS OTHERWISE NOTED ON DRAWINGS OR SPECIFICATION, PERMANENTLY ATTACH THE STEEL DECK TO BEARING SURFACES AS FOLLOWS: THE FIRST, THIRD & FIFTH LOW CORRUGATIONS, 300mm (12") MAX. CENTRES, & EACH SIDE OF EACH SHEET, ARC SPOT WELD WITH 20mm (3/4") NOMINAL TOP DIAMETER; - SIDE LAPS OF ADJACENT UNITS SHALL BE MECHANICALLY FASTENED @ 600mm (24") ON CENTRE MAX., OR WELDED USING 25mm (1") WELDS AT 600mm (24") MAX. SPACING; - SIDE CONDITIONS SHALL BE WELDED WITH 20mm (3/4") WELDS AT 900mm (36") MAX. SPACINGS.

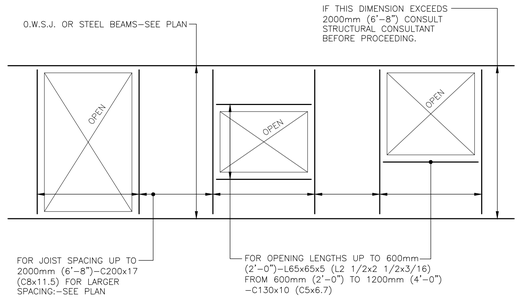
3.3 WELD STUD SHEAR CONNECTORS THROUGH DECK WHERE REQUIRED BY DRAWINGS.

3.4 "TOUCH-UP" GALVANIZED SURFACE WITH SPECIFIED PRIMER AT WELDS AND SCRAPES, ETC., BOTH UPPER AND LOWER SURFACES.

4.0 QUALITY CONTROL

4.1 AN INDEPENDENT INSPECTION & TESTING COMPANY IS TO BE ENGAGED TO CARRY OUT AND REPORT ON THE FOLLOWING INSPECTION SERVICES:-
1. SECTION PROFILE, GAUGE & STEEL GRADE.
2. ZINC COATING.
3. WELDED JOINTS.
4. BEARINGS.
5. SIDE LAP CONNECTIONS.
6. TOUCH-UP PRIMER.
7. FIELD CUTTING AND/OR ALTERATIONS.

4.2 REFER ALSO TO GENERAL NOTES AND SPECIFICATION.



NOTES:

1. TOP OF ALL TRIMMING STEEL AT UNDERSIDE OF STEEL DECK UNLESS OTHERWISE NOTED.
2. LOCATION OF ALL MECHANICAL UNITS AND OPENINGS THROUGH ROOF IS BASED ON INFORMATION SHOWN ON MECHANICAL DRAWINGS. THE STRUCTURAL STEEL SUB-CONTRACTOR MUST CONFIRM ALL THESE DIMENSIONS AND SIZES WITH THE MECHANICAL CONTRACTOR.
3. O.W.S.J. MUST BE DESIGNED FOR ADDITIONAL LOADS FROM MECHANICAL UNITS.
4. IF ACTUAL LOCATIONS OR DETAILS VARY FROM THOSE SHOWN, THE STRUCTURAL CONSULTANT MUST BE INFORMED AND INSTRUCTIONS RECEIVED BEFORE PROCEEDING WITH THE WORK.
5. THE STRUCTURAL STEEL SUB-CONTRACTOR IS TO SUBMIT ERECTION DRAWINGS TO THE MECHANICAL ENGINEER AND/OR CONTRACTOR FOR APPROVAL OF SIZE AND LOCATION OF OPENINGS FOR MECHANICAL UNITS.

TYPICAL DETAILS FOR TRIMMING OPENINGS THROUGH STEEL ROOF DECK

STRUCTURAL STEEL & OWSJ NOTES

1.0 GENERAL

1.1 STRUCTURAL STEEL AND OWSJ DESIGN DETAILS & CONNECTIONS SHALL CONFORM TO CSA STANDARD CAN/CSA S16-14 (LIMIT STATES DESIGN) & SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER EXPERIENCED IN THIS TYPE OF WORK.

1.2 REFER ALSO TO GENERAL NOTES, NOTES UNDER PLANS & TO THE SPECIFICATION.

1.3 WELDING SHALL CONFORM TO CSA STANDARD W59-13 AND BE PERFORMED BY A FABRICATOR CERTIFIED TO CSA W47.1-09 (R2014).

1.4 BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 50% OF THE BEAM SHEAR CAPACITY UNLESS OTHERWISE NOTED, & IN NO CASE BE LESS THAN THE LOADS SHOWN ON OR IMPLIED BY THE DRAWINGS.

2.0 PRODUCTS

2.1 ALL STRUCTURAL STEEL MEMBERS SHALL CONFORM TO CAN/CSA G40.20-13/G40.21-13. ROLLED SECTIONS, PLATES, SAG RODS, STRAP ANCHORS & BARS, EXCEPT WIDE FLANGE BEAMS SHALL BE TYPE 350W AND HOLLOW STRUCTURAL AND WIDE FLANGE BEAMS SECTIONS SHALL BE TYPE 350W, CLASS H FOR SQUARE HSS & CLASS C FOR ROUND HSS.

2.2 OWSJ CHORDS & WEBS SHALL CONFORM TO CLAUSE 16.3 OF CAN/CSA S16.

2.3 BOLTS, NUTS & WASHERS FOR CONNECTIONS TO CONFORM TO ASTM A325-14 UNLESS NOTED.

2.4 ANCHOR BOLTS, NUTS & WASHERS FOR BASE PLATES, BEARING PLATES & WELD PLATES TO CONFORM TO ASTM A307-14 UNLESS NOTED.

2.5 SHEAR STUDS WHERE REQUIRED TO CONFORM TO ASTM A108, WELDING TO CONFORM TO CSA W59.

2.6 WELDING MATERIALS TO CONFORM TO CSA W48-14 (SERIES).

2.7 PRIMER PAINT TO CONFORM TO CGSB 1.40-M89 OR CISC/CPMA 2-75.

2.8 FORMS FOR CONCRETE DECK OVER OWSJ: - SEE NOTES UNDER PLANS & TYPICAL DETAILS.

2.9 BRIDGING FOR OWSJ:- SEE DRAWINGS & TYPICAL DETAILS.

3.0 EXECUTION

3.1 FABRICATION, HANDLING & ERECTION TO CONFORM TO CAN/CSA S16-14.

3.2 PROVIDE A MINIMUM OF 2-12mm (1/2") DIAMETER BY 250 (10") LONG WALL ANCHORS FOR ALL BEAM & OWSJ WALL PLATES ON MASONRY, OR AN APPROVED EQUAL, UNLESS OTHERWISE NOTED. BEAMS & JOIST SHOES TO BE WELDED TO BEARING PLATES.

3.3 PROVIDE ADJUSTABLE ANCHORS TO ALL STEEL TO BE BUILT INTO, ABUTTED BY, OR FACED WITH MASONRY (REFER ALSO TO DETAILS IF SHOWN). SPACING OF ANCHORS TO BE:
1. FOR VERTICAL SPACING: 600 (24") MAX. CENTRES
2. FOR HORIZONTAL SPACING: 10 TIMES WALL THICKNESS*
MAX. 2000 (6'-8") CENTRES
(* NOTE, USE BACK-UP WYTHE ONLY FOR CAVITY WALLS.)
3. WHERE STEEL PROVIDES LATERAL BRACING ONLY TO MASONRY (IE. DOES NOT SUPPORT MASONRY) ANCHORS SHALL PERMIT DIFFERENTIAL VERTICAL MOVEMENT BETWEEN STRUCTURAL MEMBER & MASONRY.

3.4 CLEAN, PREPARE SURFACES AND SHOP PRIME STRUCTURAL STEEL & OWSJ WITH ONE COAT OF SPECIFIED PRIMER PAINT IN ACCORDANCE WITH CSA CAN3-S16-14, EXCEPT WHERE MEMBERS ARE TO BE ENCASED IN CONCRETE. FIELD "TOUCH-UP" BOLTS, WELDS, BURNED OR SCRAPED SURFACES AFTER ERECTION.

3.5 WHEREVER ITEMS ARE TO BE HUNG FROM OWSJ, SECUREMENT SHALL BE FROM THE TOP CHORDS AT PANEL POINTS UNLESS OTHERWISE PERMITTED.

3.6 PROVIDE ALL NECESSARY TEMPORARY BRACING TO KEEP STRUCTURE SAFE AND PLUMB. BRACING SHOWN ON STRUCTURAL DRAWINGS IS PERMANENT FOR FINISHED BUILDING ONLY.

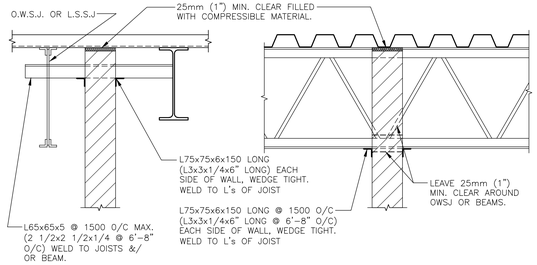
3.7 CO-ORDINATE WITH MECHANICAL & ELECTRICAL CONSULTANTS & SUB-TRADES WHOSE WORK MAY EFFECT DETAILING, FABRICATION & ERECTION OF THE STEEL STRUCTURE.

3.8 TOLERANCES: VARIATION FROM PLUMB & LEVEL EXTERIOR COLUMNS, COLUMNS AT ELEVATOR SHAFTS & SPANDREL BEAMS INCLUDING ANGLES: 1:1000 MAX 25mm (1/8" IN 10'-0" MAX. 1") OTHER PIECES: 1:500 (1/4" IN 10'-0")

3.9 NO HOLES OTHER THAN THOSE SHOWN ON REVIEWED SHOP DRAWINGS SHALL BE MADE IN ANY STEEL MEMBER WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL CONSULTANT.

4.0 QUALITY CONTROL

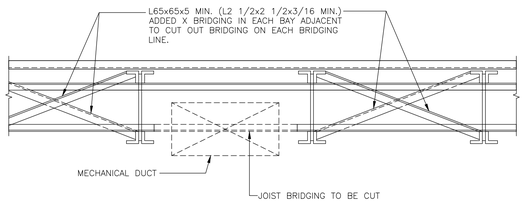
4.1 SEE GENERAL NOTES, NOTES UNDER PLANS, AND/OR SPECIFICATION FOR INSPECTION & TESTING REQUIREMENTS.



WALL PARALLEL TO O.W.S.J. &/OR BEAM

WALL PERPENDICULAR TO O.W.S.J. &/OR BEAM

TYPICAL DETAIL FOR LATERAL SUPPORT TO STEEL FRAMING AT TOPS OF NON-LOAD BEARING MASONRY WALLS



TYPICAL DETAIL WHERE BOTTOM CHORD JOIST BRIDGING AND/OR BRACING IS REQUIRED TO BE CUT DUE TO INTERFERENCE BY MECHANICAL DUCTWORK

GENERAL NOTES

1.0 GENERAL

1.1 DESIGN AND CONSTRUCTION IS TO CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE. REFER ALSO TO TYPICAL DETAILS, NOTES UNDER PLANS & SCHEDULES ON THE STRUCTURAL DRAWINGS, AND TO THE SPECIFICATION. ALL CODES, MANUALS, STANDARDS AND SPECIFICATIONS REFERRED TO SHALL BE THE LATEST EDITIONS INCLUDING ALL REVISIONS AND ADDENDA. ALL DIMENSIONS, OTHER THAN PURELY STRUCTURAL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE CHECKED AGAINST THE ARCHITECTURAL DRAWINGS AND ANY INCONSISTENCIES REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. STRUCTURAL DRAWINGS MUST NOT BE SCALED.

1.2 REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND SIZES OF OPENINGS, TRENCHES, PITS, SUMPS, EQUIPMENT, SLEEVES, DEPRESSIONS, GROOVES AND CHAMFERS NOT INDICATED ON THE STRUCTURAL DRAWINGS. UNLESS SPECIFICALLY NOTED OTHERWISE, THE ABOVE ITEMS WHERE SHOWN ON THE STRUCTURAL DRAWINGS ARE INDICATED ONLY APPROXIMATELY AS TO SIZE AND LOCATION.

1.3 UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS, NO PROVISION HAS BEEN MADE IN THE DESIGN FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE CONTRACTOR IS TO PROVIDE ALL NECESSARY BRACINGS AND SHORING REQUIRED FOR STRESSES AND INSTABILITY OCCURRING FROM ANY CAUSE DURING CONSTRUCTION. THE CONTRACTOR SHALL ACCEPT FULL RESPONSIBILITY FOR ALL SUCH MEASURES. IT SHALL ALSO BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL NECESSARY BRACINGS, SHORINGS, SHEET PILING OR OTHER TEMPORARY SUPPORTS TO SAFEGUARD ALL EXISTING OR ADJACENT STRUCTURES EFFECTED BY THIS WORK.

2.0 SHOP DRAWINGS, PLACING DRAWINGS & BAR LISTS:-

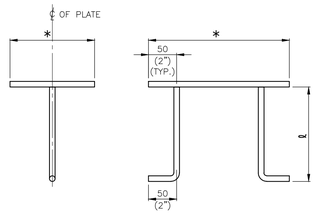
2.1 FOR ALL STRUCTURAL COMPONENTS SHOWN ON THE STRUCTURAL DRAWINGS, SUBMIT COPIES OF SHOP DRAWINGS AS DIRECTED, FOR REVIEW BY THE STRUCTURAL CONSULTANT. SHOP DRAWINGS TO SHOW COMPLETE INFORMATION FOR THE FABRICATION AND ERECTION OF THE STRUCTURAL COMPONENTS.

2.2 REVIEW BY THE STRUCTURAL CONSULTANT SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR SEEING THAT THE WORK IS COMPLETE, ACCURATE AND IN CONFORMITY WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.

3.0 INSPECTION AND TESTING:-

3.1 AN INDEPENDENT INSPECTION AND TESTING COMPANY ARE TO BE ENGAGED TO CARRY OUT THE FOLLOWING SERVICES:-
1. STRUCTURAL STEEL AND OWSJ - ROUTINE SHOP AND FIELD INSPECTION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF CAN/CSA S16-14.
2. STEEL DECK - SEE STEEL DECK NOTES.
3. MASONRY - WHEN REQUIRED OR DIRECTED, CONCRETE BLOCKS SHALL BE TESTED IN ACCORDANCE WITH CSA A165 SERIES-14; BRICKS IN ACCORDANCE WITH CSA A165.2-14; AND MORTAR AND/OR GROUT IN ACCORDANCE WITH CSA A179-14.

3.2 ALL INSPECTION AND TESTING SERVICES ARE TO BE PERFORMED BY COMPANIES CERTIFIED BY THE CANADIAN STANDARDS ASSOCIATION AND FOR WELDING, INSPECTORS ARE TO BE CERTIFIED BY THE CANADIAN WELDING BUREAU.



TYPICAL WALL PLATE DETAIL

1. ALL BENT ANCHORS TO BE FABRICATED FROM BAR STEEL WITH A MIN. YIELD OF 350 MPa, U.N.O.
2. "*": SEE PLANS AND WALL PLATE SCHEDULE
3. a = (SPECIFIED TOTAL LENGTH -50 (-2")). SEE WALL PLATE SCHEDULE FOR LENGTH

Contractor must verify all dimensions on the Project Site and report any discrepancies before proceeding with the Work.

This drawing is a part of the Contract Documents and is to be read in conjunction with all other Contract Documents.

Revision Record

No.	Description	Date (m/d/y)
3	ISSUED FOR TENDER	04/14/26
2	ISSUED FOR BUILDING PERMIT	03/30/26
1	ISSUED FOR CLIENT REVIEW	03/10/26
No.	Description	Date (m/d/y)

Issue Record

General Notes:

LANHACK Steelcon Inc.
Consulting Engineers
1709 Upper James Street
Hamilton, ON L9B 1K7
Tel: (905) 777-1454
Fax: (905) 336-8142

Regina Mundi Catholic Elementary School

ROOF ASSEMBLY REPLACEMENT

675 MOHAWK ROAD WEST
HAMILTON, ONTARIO

Date:	JANUARY 2026
Drawn By:	VE
Chkd By:	MB
Scale:	N.T.S.

Drawing title:

TYPICAL NOTES & DETAILS

Project No.:	Drawing No.:	Rev.:
26032(A)	S3-1	3

Plot Date: